



SOUTH FAYETTE T O W N S H I P

A Community Growing Together

January 22, 2026 - 7 P.M.

Planning Commission Meeting Agenda

1. CALL TO ORDER

2. ROLL CALL

Tom Iagnemma, Chairman
Doug Kaine, Secretary
Joseph Johnson
Jamey Noland
Danny Cerrone Jr.
Jason Paulovich, Gibson-Thomas Engineering
Chris Seymour, Dodaro, Matta & Cambest, P.C.

3. PLEDGE OF ALLEGIANCE

4. OLD BUSINESS

5. NEW BUSINESS

- A. Review And Discuss The Attached Ordinance Amending And Restating Various Sections In Chapter 240, Zoning.

AN ORDINANCE OF THE TOWNSHIP OF SOUTH FAYETTE, ALLEGHENY COUNTY, PENNSYLVANIA, amending and restating Chapter 240, Zoning, Article IV R-1 Rural Residential District, Section 240-16 and Article X, PED Planned Economic Development District, Sections 240-56 through 240-62, amending its Zoning Ordinance, setting forth various regulations to update and modernize the PED District's boundaries and approved uses as well as adjusting the R-1 Rural Residential District's area and bulk regulations; Amending the Township's official Zoning Map to adjust the PED District boundaries and to change an existing residential area in Sturgeon from commercial zoning to residential; Amending Article XVI to add supplemental regulations for Data Centers.

Documents:

[ORDINANCE FINAL DRAFT.PDF](#)
[PROPOSED PED.PDF](#)
[EXISTING PED.PDF](#)
[STURGEON ZONING.PDF](#)

6. ACCEPTANCE OF APPLICATIONS

- A. Review And Consider For Acceptance S-01-2026 Second Revision To The Kosky Plan Of Lots, Preliminary And Final Minor Subdivision Application, Parcels 256-L-2, 256-L-1, Zoned C-2.

Documents:

APPLICATION S-01-2026.PDF
AGENT AUTHORIZATION FORM.PDF
SUBDIVISION.PDF
DEED.PDF
SALES AGREEMENT.PDF

- B. Review And Consider For Acceptance SP-01-2026 Lafayette 180 Planned Shopping Center, Preliminary And Final Land Development Application, Parcels 256-L-2, 256-L-1, Zoned C-2.

Documents:

APPLICATION SP-01-2026.PDF
AGENT AUTHORIZATION FORM.PDF
LAND DEVELOPMENT PLANS (REDUCED SIZE).PDF
ELEVATION RENDERINGS.PDF
TIS ADDENDUM COMPLETE.PDF
TRAFFIC IMPACT STUDY_PROPOSED RETAIL - SOUTH FAYETTE TOWNSHIP TIS.PDF
2022-01-14 GEOTECHNICAL REVIEW MEMO.PDF
2026-01-12 NPDES MAJOR MODIFICATION APPROVAL.PDF
WETLAND DELINEATION.PDF
UTILITY AVAILABILTY LETTERS.PDF
DEED.PDF
SALES AGREEMENT.PDF

7. OPEN DISCUSSION

8. ADJOURNMENT

ORDINANCE NO. ____ OF 2025 OF SOUTH FAYETTE TOWNSHIP

AN ORDINANCE OF THE TOWNSHIP OF SOUTH FAYETTE, ALLEGHENY COUNTY, PENNSYLVANIA, amending and restating Chapter 240, Zoning, Article IV R-1 Rural Residential District, Section 240-16 and Article X, PED Planned Economic Development District, Sections 240-56 through 240-62, amending its Zoning Ordinance, setting forth various regulations to update and modernize the PED District's boundaries and approved uses as well as adjusting the R-1 Rural Residential District's area and bulk regulations; Amending the Township's official Zoning Map to adjust the PED District boundaries and to change an existing residential area in Sturgeon from commercial zoning to residential; Amending Article XVI to add supplemental regulations for Data Centers; Amending Article XVII to add parking and loading berth requirements for Data Centers.

WHEREAS, the Township of South Fayette has enacted a Zoning Ordinance to protect the public health, safety, and welfare of its citizens; and,

WHEREAS, the Township Board of Commissioners wishes to amend Article IV R-1 Rural Residential District, Section 240-16, Article X PED Planned Economic Development District, Sections 240-56 through 240-62, and Article XVII 240-112 and 240-113; and

WHEREAS, the PED Planned Economic Development District was created to promote economic development on large undeveloped tracts in a campus-style atmosphere, allowing for a compatible mix of uses that encourages an integrated living and working environment while preserving adequate buffers between dissimilar uses; and

WHEREAS, the Board of Commissioners of the Township of South Fayette finds that amending such a district will contribute to the general welfare of the community by providing for expanded development access within the Township, and support the vision and goals outlined in the Township's 2023 Comprehensive Plan to promote economic development along the I-79 and I-576 corridors; and

WHEREAS, it is the intention of the Board of Commissioners of the Township of South Fayette by this Ordinance to amend and restate the prior PED District and zoning map to include those areas in the PED that the Board of Commissioners have determined to exist in a limited PED Zoning district as set forth on Exhibit "B"; and

WHEREAS, it is the intention of the Board of Commissioners of the Township of South Fayette by this Ordinance to amend the zoning map in the Sturgeon area to change some parcels from commercial to residential.

NOW, therefore, be it ordained and enacted by the Board of Commissioners of the Township of South Fayette, that Chapter 240 Zoning Article IV and Zoning Article X are hereby amended as follows:

SECTION 1: PED DISTRICT PARCELS AND BOUNDARIES.

Within Section 240-10, this amendment shall repeal and replace the map associated with Exhibit B with a new map highlighting the PED boundaries and parcels, attached to this ordinance, as Exhibit A.

After holding a Public Hearing, the Board of Commissioners of the Township of South Fayette does hereby concur with the updated recommendations made by the Township of South Fayette and now confirms and finds that the areas listed in Exhibit "B" hereto in the Township of South Fayette respectively constitute the PED District for purposes of this Ordinance. The change adds parcel number 0486-G-00002-0000-00 to the existing PED District, effectively amending the Township's official Zoning Map to reflect these same changes.

SECTION 2: NEW TERMS AND DEFINITIONS.

Article II, Section 240-9 shall be amended to include the following definitions, to be inserted in alphabetical order within the existing list

1. Asphalt/Concrete Plant
 - a. A plant where asphalt or concrete is mixed for distribution, typically for use off-site.
2. Backup Generators
 - a. Natural gas, diesel, hydrogen fuel cells, or other non-coal Power Storage System equipment used to generate electricity during a power outage or similar emergency. Backup Generators are only to be used during periods of outages, natural disasters or similar "emergency events" for power generation and for regular reliability testing and exercising.
3. Cryptocurrency and Data Mining
 - a. The operation of specialized computer equipment for the purpose of mining one (1) or more blockchain-based cryptocurrencies, such as Bitcoin. This activity typically involves the solving of algorithms as part of the development and maintenance of a blockchain which is a type of distributed ledger maintained on a peer-to-peer network. Typical physical characteristics of cryptocurrency mining include specialized computer hardware for mining operations as well as

equipment to cool the hardware and operating space. For the purposes of the associated regulations, cryptocurrency mining does not include the exchange of cryptocurrency or any other type of virtual currency nor does it encompass the use, creation, or maintenance of all types of peer-to-peer distributed ledgers.

4. Dormitories

- a. Buildings at a college/university, school, or institution containing a number of private or semiprivate rooms for residents, along with common bathroom facilities and recreation areas.

5. Data Center, Small

- a. A facility less than 50,000 sq. ft. in size, including the primary building(s), support structure(s), Data Center Accessory Uses, and associated appurtenances, used primarily for or intended to be used primarily for the housing, operation, and/or co-location of computer and communications equipment and for handling, storing, and backing up the data necessary for the operation of a business or organizational entity. Data Center may also include data center equipment or DCE and/or Data Center Accessory Uses when located on the same tract or assemblage of adjacent parcels developed as a unified development.

6. Data Center, Medium

- a. A facility between 50,000 sq. ft. and 500,000 sq. ft. in size, including the primary building(s), support structure(s), Data Center Accessory Uses, and associated appurtenances, used primarily for or intended to be used primarily for the housing, operation, and/or co-location of computer and communications equipment and for handling, storing, and backing up the data necessary for the operation of a business or organizational entity. Data Center may also include data center equipment or DCE and/or Data Center Accessory Uses when located on the same tract or assemblage of adjacent parcels developed as a unified development.

7. Data Center, Large

- a. A facility over 500,000 sq. ft. in size, including the primary building(s), support structure(s), Data Center Accessory Uses, and associated appurtenances, used primarily for or intended to be used primarily for the housing, operation, and/or co-location of computer and communications equipment and for handling, storing, and backing up the data necessary for the operation of a business or organizational entity. Data Center may also include data center equipment or DCE

and/or Data Center Accessory Uses when located on the same tract or assemblage of adjacent parcels developed as a unified development.

8. Data Center Accessory Uses

- a. A use or structure that is subordinate and incidental to a Data Center, generally including but not limited to utilities, utility lines, Backup Generators, Power Generation Plans, electrical substations, pump stations, water towers or holding tanks, mechanical equipment, cooling systems, sound control systems, fire suppression systems, and environmental controls (emission controls, noise pollution controls, environmental impact monitoring), redundant/backup power supplies, redundant data communications connections, security operations including fencing and security guard buildings, supporting office and/or employee support space, Power Storage Systems, equipment storage yards, outdoor storage areas, and fuel storage when located on the same parcel or assemblage of adjacent parcels developed as a unified development for a Data Center.

9. Flex Space

- a. An establishment which can include office, light manufacturing, pilot manufacturing, and research and development, in addition to warehousing and associated administrative space.

10. Landscaping Service Center, Warehouse/Wholesale

- a. A business primarily engaged in processing, selling, and distributing indoor or outdoor grown plants and landscaping materials to industrial, commercial, institutional, or professional users or to other wholesalers.

11. Micro-brewery

- a. A small, usually independent brewery that produces limited quantities of malt or brewed beverages. A microbrewery provides for the retail sales of the beer at the location where it is produced. A microbrewery may also include a tasting room and restaurant in conjunction with the use, as regulated by Title 47, the Pennsylvania Liquor Code, as amended.

12. Pet Boarding

- a. Taking custody or possession of more than four dogs and/or more than six cats for the keeping, accommodation, care, training, or feeding for fee or reward at a property other than the animal's normal place of residence. The domesticated

animal can be left in the care of said establishment for a variable period of time. The establishment must be able to accommodate domesticated pets for extended periods of time, including but not limited to overnight stays within indoor facilities.

13. Power Generation Plant

- a. A plant fueled by natural gas, combined or single cycle power plant, or natural gas linear generation, fuel cell, or similar non-oil or non-coal based power generation systems and equipment, which creates electricity to be used by the Data Center, sold to or by a utility company, or in some combination of sale or purchase.

14. Power Storage System

- a. A power storage system and the infrastructure systems and processes used to store, distribute and manage the power generated within the Data Center site. Power Storage System may also include a BESS (Battery Energy Storage System), which provides support to the local, regional or overall power grid by storing energy during off-peak hours and releasing it back into the grid as demand warrants.

15. Salt Storage Facility

- a. A structure utilized to house salt used to treat public and/or private roadways.

SECTION 3: UPDATED R-1 RURAL RESIDENTIAL AREA AND BULK REGULATIONS.

Article IV, R-1 Rural Residential District, Section 240-16 B. Minimum lot width., shall be amended in its to read as follows, thus increasing the required minimum lot width:

§ 240-16 B. Minimum lot width.

- (1) Single-family dwelling.
 - a. Without public sewers: 150 feet.
 - b. With public sewers: 125 feet.
- (2) All other principal uses: 200 feet.

SECTION 4: UPDATED PED PLANNED ECONOMIC DEVELOPMENT DISTRICT ARTICLE.

Article X, PED Planned Economic Development District, shall be repealed and amended to read as follows:

§ 240-57. Authorized uses.

In the PED Planned Economic Development District, only the following uses are authorized:

1. Permitted uses.
 - a. Principal uses.
 - i. Agriculture, subject to § [240-103](#).
 - ii. Essential services.
 - iii. Data center, small, subject to the supplemental regulations in Article XVI, § 240-109.2.
 - iv. Data center, medium, subject to the supplemental regulations in Article XVI, § 240-109.2.
 - v. Data center, large, subject to the supplemental regulations in Article XVI, § 240-109.2.
 - vi. Data center accessory uses where a data center has been approved.
 - vii. Forestry, subject to § [240-105](#).
 - viii. Incidental mineral removal.
 - ix. Water towers and water storage facilities.
 - x. Once conditional use approval has been granted to a planned economic development, one or more of the following uses may be established on a lot within the planned economic development, subject to land development plan approval as required by the Township Subdivision and Land Development Ordinance.
 1. Within an approved open space design component: (Note: The maximum percentage of the total tract of land that can be used for development of any residential component equals 20%.)
 - a. Single-family dwellings.
 - b. Two-family dwellings.
 - c. Triplexes or fourplexes.
 - d. Townhouses.
 - e. Garden apartments.
 - f. Mid-rise apartments.
 - g. Retirement community.
 - h. Nursing home.
 - i. Common open space.
 - j. Public recreation; noncommercial recreation.
 - k. Community club.
 2. Within an approved planned commercial development component:
 - a. Any permitted principal use in the C-2 District specifically listed in § 240-50A(1).
 - b. Any conditional use in the C-2 District specifically listed in § 240-50B, except billboards, subject to compliance with the applicable express standards and criteria in Article XV.
 3. Within an approved planned educational campus component:

- a. Public, private or commercial school.
 - b. Campus housing.
 - c. Administrative offices.
 - d. Restaurants.
 - e. Indoor entertainment.
 - f. Motel or hotel.
 - g. Public recreation; noncommercial recreation.
 - h. Places of assembly.
 - i. Museums, libraries.
 - j. Research and development.
 - k. Financial institution.
 - l. Hospital.
 - m. Dormitories.
4. Within an approved planned research or technology park component:
 - a. Business and professional offices; medical offices; medical clinic.
 - b. Business services.
 - c. Financial institutions.
 - d. High technology industries.
 - e. Research and development.
 - f. Health club.
 - g. Motel or hotel.
 - h. Public recreation; noncommercial recreation.
 - i. Restaurant.
 - j. Hospital.
 - k. Cryptocurrency and data mining.
5. Within an approved planned industrial park component:
 - a. Business or professional offices.
 - b. Business services.
 - c. Contracting business.
 - d. Equipment storage yard.
 - e. Financial institutions.
 - f. High technology industries.
 - g. Light manufacturing.
 - h. Medical offices; medical clinic.
 - i. Printing establishment.
 - j. Research and development, including pilot manufacturing.
 - k. Hotel or motel.
 - l. Health club.
 - m. Public recreation; noncommercial recreation.
 - n. Repair shop.
 - o. Restaurant.
 - p. Truck and heavy equipment rental, sales and service.

- q. Vehicle repair.
 - r. Warehousing and distribution.
 - s. Wholesale business.
 - t. Asphalt/concrete plant.
 - u. Distribution center.
 - v. Flex space.
 - w. Truck terminal.
 - x. Landscaping Service Center, Warehouse/Wholesale.
 - y. Self-storage facility.
 - z. Medical marijuana grower/processor.
 - aa. Natural gas compressor station.
 - bb. Natural gas processing station.
6. Within an approved mixed-use development:
- a. Residential uses including:
 - i. Single-family dwellings.
 - ii. Two-family dwellings.
 - iii. Triplexes or fourplexes.
 - iv. Townhouses.
 - v. Garden apartments.
 - vi. Mid-rise apartments.
 - vii. Age-restricted housing community.
 - b. Nonresidential uses including:
 - i. All permitted uses in § [240-57A\(1\)\(f\)\[2\]](#) through [\[5\]](#) and § [240-57B\(1\)\(d\)\[2\]](#) through [\[5\]](#).
 - ii. Assisted living facility.
 - iii. Retirement community.
 - iv. Nursing homes.
 - v. Common open space.
 - vi. Public recreation; noncommercial recreation.
 - vii. Community club.
 - viii. Micro-brewery.
 - ix. Salt storage facility.
- b. Accessory uses.
- i. Signs, subject to Article XVIII.
 - ii. Off-street parking and loading, subject to Article XVII.
 - iii. Fences, subject to § 240-99.
 - iv. Other accessory uses customarily incidental to and on the same lot with any permitted use, conditional use or use by special exception authorized in this District.
 - v. Warehousing of products produced on site, comprising no more than 25% of the gross floor area of the building.
 - vi. Water towers and water storage facilities.
 - vii. Community center in connection with an age-restricted housing community.

2. Conditional uses.
 - a. Principal uses.
 - i. Airport, subject to § 240-95A(50).
 - ii. Communications tower, subject to § 240-95A(11).
 - iii. Mineral removal, subject to § 240-95A(28).
 - iv. Planned economic development, subject to § 240-95A(34), comprised of one or more of the following components:
 1. Open space design option.
 2. Planned commercial development.
 3. Planned educational campus.
 4. Planned research or technology park.
 5. Planned industrial park.
 6. Mixed-use development.
 - v. Shooting range, subject to § 240-95A(47).
 - vi. Oil and gas development, subject to § 240-95A(54).
 - vii. Within an approved planned commercial development component:
 1. Pet services only in combination with a kennel, subject to § 240-95A(57).
 - b. Accessory uses.
 - i. Supporting commercial uses in a planned industrial park or planned research or technology park, subject to § 240-95A(41).
 - ii. Communications antenna mounted on an existing building or on an existing public utility storage or transmission structure, subject to § 240-95A(12).
 - iii. Temporary construction trailer or sales office, subject to § 240-95A(49).
 - iv. Data center accessory uses where a data center has been approved.
3. Uses by special exception.
 - a. Principal uses.
 - i. Temporary use or structure, other than a construction trailer or sales office, subject to § 240-95A(42).
 - ii. Comparable uses not specifically listed, subject to § 240-95A(13).
 - b. Accessory uses.
 - i. Agricultural sales, subject to § 240-95A(48).

SECTION 5: SUPPLEMENTAL REGULATIONS ASSOCIATED WITH DATA CENTERS.

Article XVI Supplemental Regulations shall be amended to add a new section, § 240-109.2, Data Centers, to read as follows:

§ 240-109.2. Data centers.

Data centers may be conducted with the PED District provided all of the following criteria are met:

- A. A Data Center Master Plan shall be submitted to the Township for review and shall include the following information:
 - a. Noise Study.
 - b. Water Usage Calculations.
 - c. Emergency Response Plan.
 - d. Environmental Impact Statement.
 - e. Traffic Impact Study.
- B. Minimum lot size requirements, as follows:
 - a. Data center, small: 10 acres.
 - b. Data center, medium: 50 acres.
 - c. Data center, large: 300 acres.
- C. Data Center land development plans may be submitted in multiple phases, provided an overall Master Plan is submitted for review by the Township.
- D. The submission of a Bulk Earthwork or Mass Grading Permit Package may be submitted in advance of any land development approval, provided that an overall Master Plan is submitted for review by the Township, and provided that all other regulatory approvals required for land disturbance are obtained. Copies of any other approvals shall be provided to the Township for its records.
- E. Maximum impervious surface coverage
 - a. 70% (Stormwater detention facilities shall not be included in calculation)
- F. Maximum height.
 - a. All Data Centers may be no more than 100 feet in height.
 - b. All accessory structures may be six stories but not more than 100 feet in height.
 - c. The height limitations of this chapter shall not apply to the following structures: church spires, chimneys, elevator bulkheads and other mechanical equipment which is part of the principal structure, conveyors, flagpoles, silos, standpipes, elevated water tanks, derricks, public utility structures, sound walls when placed on top of a building, other structures not intended for human habitation which do not exceed the height limitations of the zoning district by more than 15 feet, and elevated water tanks for Data Centers, which may be no more than 130 feet in height.
- G. Noise.
 - a. The following uses or activities shall be exempted from the noise regulations:
 - i. Noises emanating from construction or maintenance activities between 7:00 a.m. and 9:00 p.m. and backup generation equipment during a power outage or similar emergency at all times of day.
- H. Buffer areas.
 - a. No structures or uses shall be permitted in the required buffer area, other than fences, sound walls or other noise mitigation measures, active or passive recreation facilities and stormwater management facilities, provided the structures or uses do not interfere with the required plantings in the buffer area and provided all plantings are located outside any stormwater management structure. Structures or uses not permitted within the required buffer area include, but are not limited to, buildings, accessory structures, parking spaces,

access drives and lighting devices. Notwithstanding the requirements of Article XVI, § 240-98 J. (4), the deciduous tree planting requirement for a Data Center with a building footprint exceeding 100,000 square feet shall be 50 trees per building.

- I. Landscaping specifications.
 - a. Parking areas shall be landscaped in accordance with the following requirements:
 - i. Placement of required trees may be relocated outside of parking areas so long as they are located within the site development boundary overall. Reductions in the required number of trees may be approved at the discretion of the Township, when such utility infrastructure can be demonstrated to impede the installation and/or survival of required trees.
- J. Accessory Structures.
 - a. Fences and walls.
 - i. The maximum height of a fence for a Data Center is ten (10) feet.
 - ii. The maximum height of a wall used for the purposes of retaining earth shall not be limited if a retaining wall design is provided by a licensed professional engineer. Sound walls are excluded from this maximum height limitation, and the height of sound walls shall be in accordance with the findings of an approved noise study.
 - b. Structures accessory to nonresidential structures and buildings.
 - i. No structure accessory to a nonresidential building or structure, other than a sign or off-street parking area, shall be located in the front yard setback.
- K. Storage.
 - a. Outdoor storage in commercial and industrial districts.
 - i. Storage and display of materials outside a completely enclosed structure shall be permitted for Data Centers when outdoor storage is a Data Center Accessory Use.

SECTION 6: OFF-STREET PARKING AND LOADING REQUIREMENTS FOR DATA CENTERS.

Article XVII shall be amended to include off-street parking and loading requirements for Data Centers. The Table of Parking Requirements found in § 240-112. C. shall be amended to add the Use Data Center and its requirement, to be inserted in alphabetical order, as follows:

§ 240-112. C. Table of Parking Requirements

Use	Parking Spaces Required
Data Center	One space for each 1,500 square feet of net floor area that is not occupied by data halls, or, one space per employee during peak shift, whichever is greater.

Off-street loading requirements found in § 240-113 shall be amended to add a requirement for Data Centers, to be inserted at the end of this section, as follows:

§ 240-113. D. Off-street loading requirements for Data Centers. Data Centers shall provide 1 loading berth per building.

SECTION 7: STURGEON ZONING MAP CHANGE.

The Township's official Zoning Map shall be amended in the Sturgeon neighborhood to better reflect the existing uses of properties along Magnolia Street. The following parcels shall be changed from C-1 Limited Commercial to R-1 Rural Residential: 0583-B-00001-0000-00, 0583-B-00005-0000-00, 0583-B-00004-0000-00, 0583-B-00003-0000-00, 0583-F-00050-0000-00, 0583-F-00049-0000-00, 0583-F-00041-0000-00, 0583-F-00044-0000-00, 0583-F-00043-0000-00, 0583-F-00045-0000-00, 0583-F-00046-0000-00, 0583-F-00047-0000-00, 0583-F-00048-0000-00, 0583-K-00024-0000-00, 0583-K-00025-0000-00, 0583-F-00059-0000-00, 0583-F-00039-0000-00, 0683-G-00100-0000-00, and 0583-F-00040-0000-00.

A map showing these changes is attached in Exhibit B.

SECTION 8: REPEALER.

That any and all previous Ordinance(s) which are inconsistent with the terms and provisions of this Ordinance are hereby repealed.

SECTION 9: SEVERABILITY.

If any sentence, clause, section, or part of this Ordinance is for any reason found to be unconstitutional, illegal or invalid, such unconstitutionality, illegality or invalidity shall not affect or impair any of the remaining provisions, sentences, clauses, sections or parts of this Ordinance. It is hereby declared as the intent of the Board of Supervisors that this Ordinance would have been adopted had such unconstitutional, illegal or invalid sentence, clause, section or part thereof not been included herein.

SECTION 10: EFFECTIVE DATE.

That this Ordinance shall take effect immediately upon enactment as provided by law.

ORDAINED AND ENACTED into law this ___ day of January 2026.

ATTEST:

SOUTH FAYETTE TOWNSHIP

Township Secretary

BY: _____
President , Board of Commissioners

APPROVED AS TO FORM:

Solicitor

DRAFT

PROPOSED PED DISTRICTS

SOUTH FAYETTE TOWNSHIP
PED ORDINANCE UPDATE

Legend

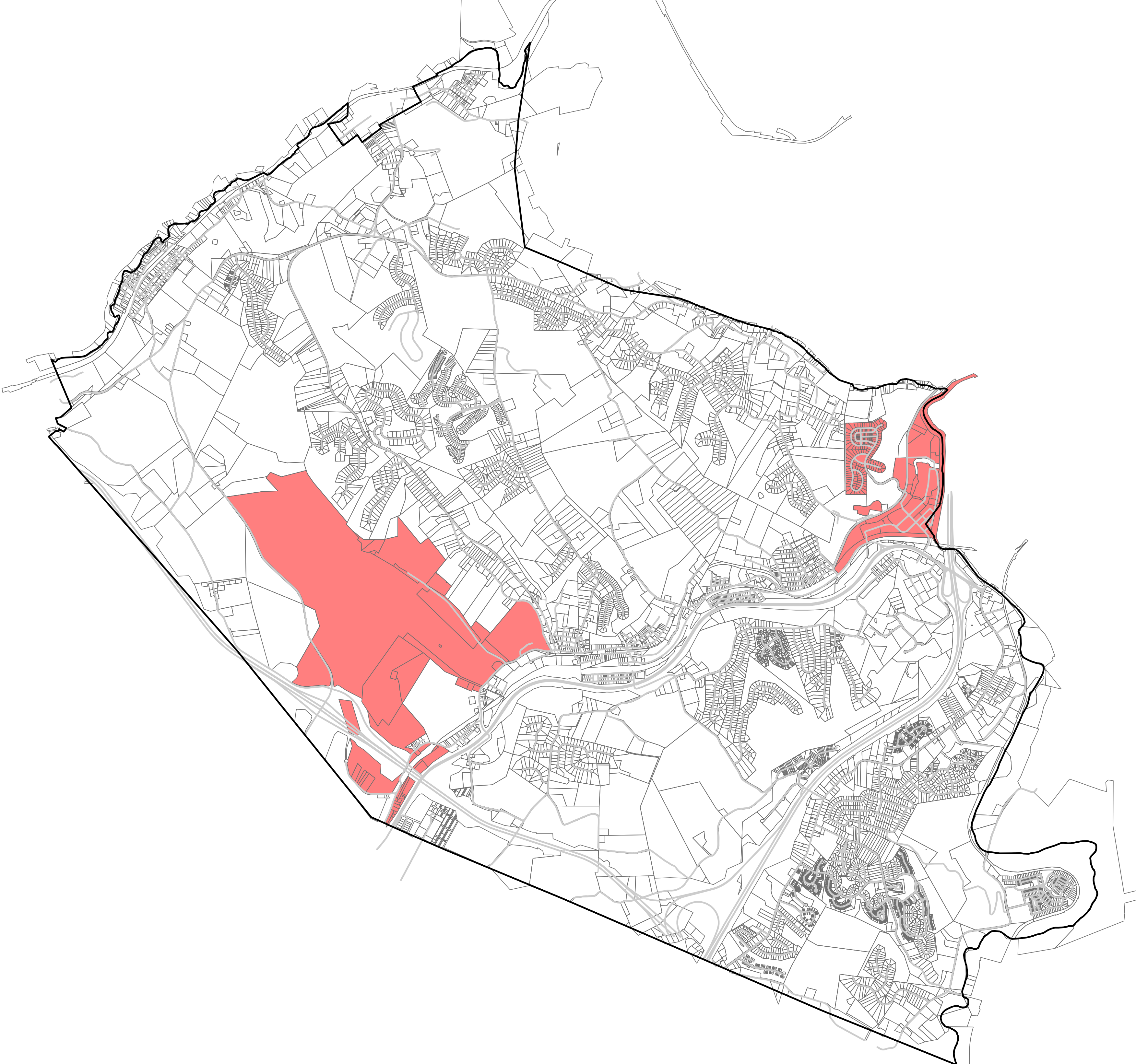
□ South Fayette

— Roads

Proposed PED Parcels

■ Proposed PED

□ All other parcels



10/30/2025 P:\0025\002517_0449\GIS\Projects\SouthFayette\

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HRG

EXISTING PED DISTRICTS

SOUTH FAYETTE TOWNSHIP
PED ORDINANCE UPDATE

Legend

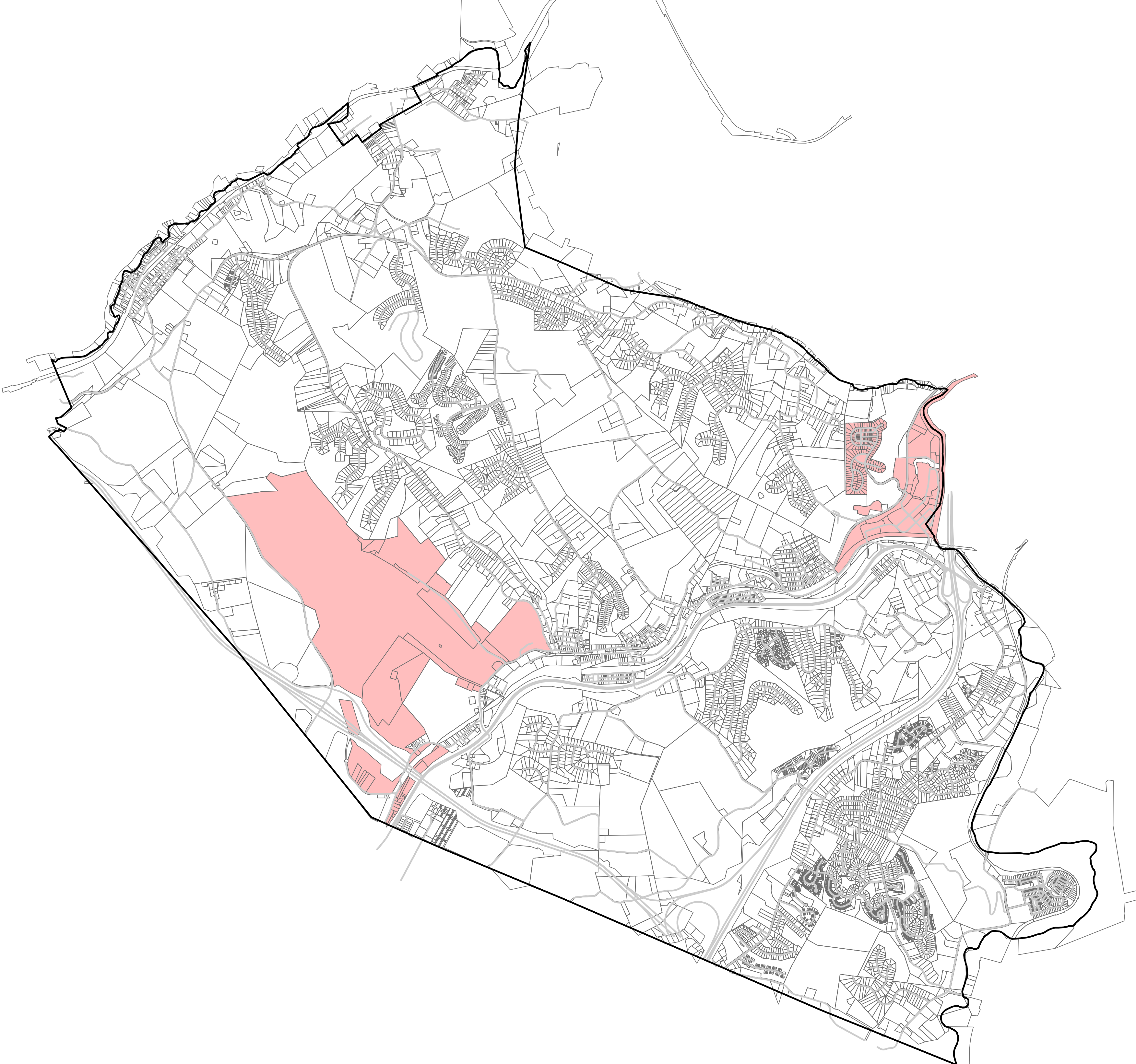
□ South Fayette

— Roads

Parcel Status

■ Existing PED

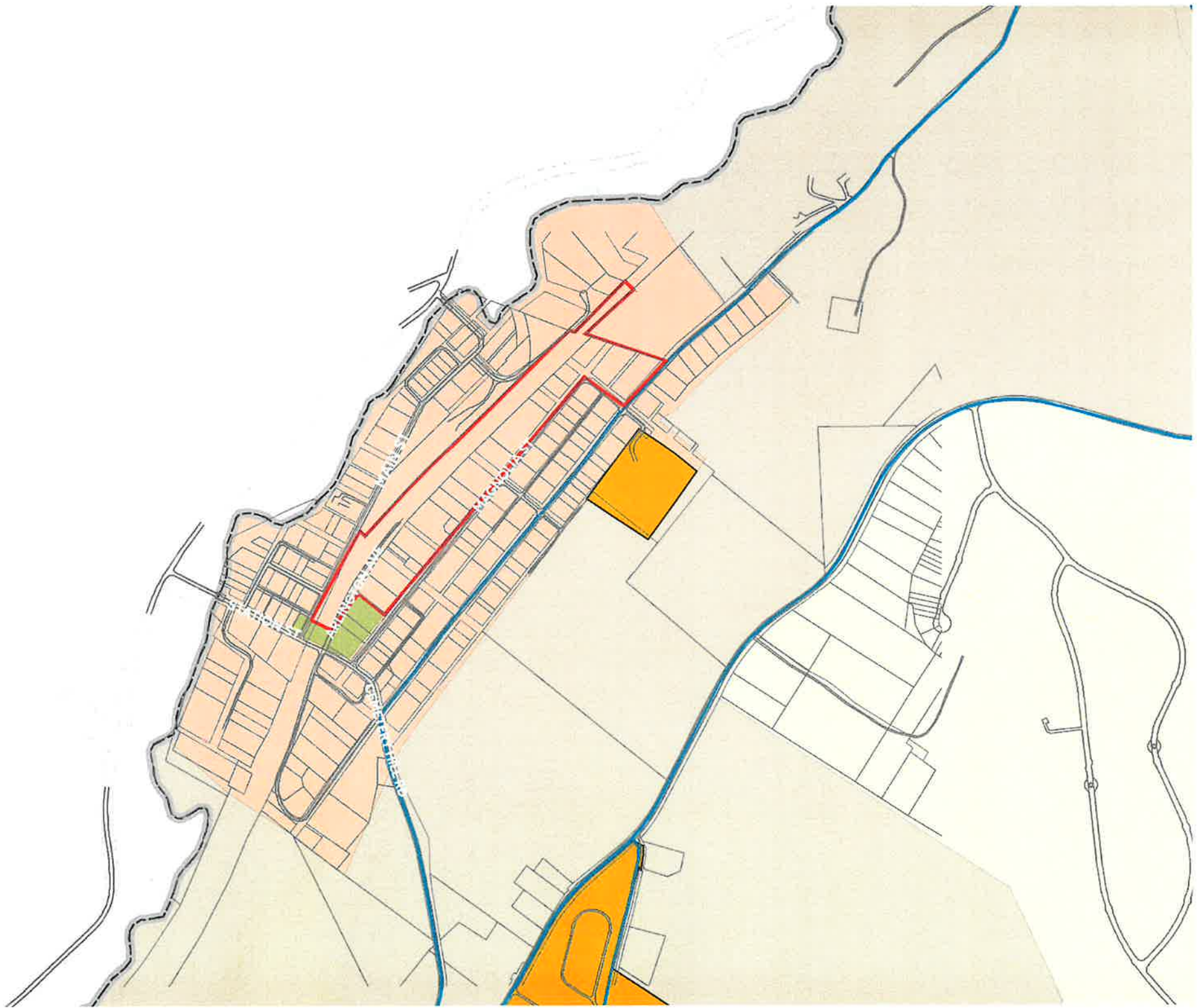
□ All other parcels



10/30/2025 P:\0025\002517_0449\GIS\Projects\SouthFayette\

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HRG





Plan Name:	Lafayette 180	File No.	S-01-2026
Plan Location:	Newbury Drive	Tax I.D. #	0256-L-00002, 0256-L-00001, 0256-L-00009
Project Description	Proposed planned shopping center with parking lot and associated utilities.		

Check Appropriate Box(es)

Land Development Plan	<input type="checkbox"/>	Subdivision Plan	<input type="checkbox"/>	Conditional Use Plan	<input type="checkbox"/>
Minor Subdivision	<input checked="" type="checkbox"/>	Major Subdivision	<input type="checkbox"/>	Open Space Plan	<input type="checkbox"/>
Preliminary Plan Submission	<input type="checkbox"/>	Final Plan Submission	<input checked="" type="checkbox"/>		

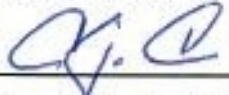
Zoning District(s)	C-2	Property Acreage	5.48 acres	No. Lots/Units	
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Applicant's Name:	Craig Cozza	Phone No.	
Applicant's Address:	295 Myoma Road, Mars, PA 16046	Fax No.	
Applicant's E-Mail:			
Engineer Firm/Name	The Gateway Engineers, Inc.	Phone No.	
Engineer's Address:	100 McMorris Road, Pittsburgh, PA	Fax No.	
Contact Person:	Joseph Galbraith	E-Mail Address	

The following items are reviewed as part of the South Fayette Township application process. Applications submitted WITHOUT these elements will NOT be reviewed by the township.

REQUIRED SUBMISSION ITEMS	Copies	✓ Yes	✓ No	✓ N/A
1) Completed Application Form	1			
2) ACED Subdivision/Land Development Application	1			
3) Maps and Plans				
• Plus PDF of drawings	1			
• Sets Full-Size Plans (24" x 36")	5			
• Sets Half-Size Plans (11" x 17")	5			
4) Agent Authorization Form	1			
5) Application Fee	1			
6) Escrow – Engineer, Solicitor, Inspection	1			
7) Stormwater Management Plan & Calculations (plus PDF)	2			
8) Erosion & Sedimentation Control Plan (plus PDF)	2			
9) Deed, Sales Agreement or Other Ownership	1			

I have familiarized myself with and hereby agree to comply with the subdivision and zoning ordinance of the township as well as to all township rules, regulations and resolutions. I agree to pay the applicable fee(s).

Signature of Applicant:  Date: 01/14/2026

If applicant is not the property owner, an Agent Authorization Form must be attached.



SOUTH FAYETTE T O W N S H I P

A Community Growing Together

Agent Authorization

Form

www.southfayettepa.com

Name of Property Owners: Craig Cozza

Property Tax Map Number: 0256-L-00002, 0256-L-000001, 026-L-00009

Property/Project(s): Lafayette 180 - Planned Shopping Center

This application/phase only

All related applications and phases

The above named property owner hereby appoints: The Gateway Engineers, Inc.

as its agent and authorizes said agent to apply for and process the above mentioned development plan/variance on his/her behalf. Agent is further authorized to sign all necessary documentations for such purposes, including acceptance of conditions imposed by the Board of Commissioners upon arrival of the plan. This authorization shall remain in full force and effect until written notice of revocation is delivered to the South Fayette Township Manager.

SIGNED AND SEALED, intending to be legally bound on this date of:

Craig J. Cozza

Owner(s) Signature:  (SEAL)

ALL SIGNATURES MUST BE MADE WITH A BLUE INK PEN

THE CE-SF, LP, OWNER OF THE LAND SHOWN ON THE SECOND REVISION TO THE KOSKY PLAN OF LOTS HEREBY ADOPTS THIS PLAN AS ITS PLAN OF LOTS AND IRREVOCABLY DEDICATES ALL STREETS AND OTHER PROPERTY IDENTIFIED FOR IDENTIFICATION ON THE PLAN TO THE TOWNSHIP OF SOUTH FAYETTE. THIS ADOPTION AND DEDICATION SHALL BE BINDING UPON THE PARTNERSHIP AND UPON ITS HEIRS, EXECUTORS, AND ASSIGNS.

IN WITNESS OF WHICH, TO THIS I SET MY HAND AND SEAL THIS ____ DAY OF _____, 20 ____

ATTEST:

NOTARY PUBLIC CRAIG J. COZZA

BEFORE ME, THE UNDERSIGNED NOTARY PUBLIC IN AND FOR THE COMMONWEALTH OF PENNSYLVANIA AND COUNTY OF ALLEGHENY, PERSONALLY APPEARED THE ABOVE NAMED CRAIG J. COZZA, A PARTNER IN THE FIRM OF CE-SF, LP, AND ACKNOWLEDGED THE FOREGOING ADOPTION AND DEDICATION TO BE THE ACT OF THE PARTNERSHIP.

WITNESS MY HAND AND NOTARIAL SEAL THIS ____ DAY OF _____, 20 ____.

MY COMMISSION EXPIRES THE ____ DAY OF _____, 20 ____.

(SEAL)

NOTARY PUBLIC

I HEREBY CERTIFY THAT THE TITLE TO THE PROPERTY CONTAINED IN THE SECOND REVISION TO THE KOSKY PLAN OF LOTS IS IN THE NAME OF CE-SF, LP AND IS RECORDED IN DEED BOOK VOLUME _____, PAGE _____, DEED BOOK VOLUME _____, PAGE _____, AND DEED BOOK VOLUME _____, PAGE _____.

WITNESS CRAIG J. COZZA

S&T BANK, MORTGAGEE OF THE PROPERTY CONTAINED IN THE SECOND REVISION TO THE KOSKY PLAN OF LOTS CONSENTS TO THE RECORDING OF SAID PLAN AND TO THE DEDICATIONS AND ALL OTHER MATTERS APPEARING ON THE PLAN.

WITNESS NAME, TITLE, AND MORTGAGEE

I, THE UNDERSIGNED, HEREBY CERTIFY THAT WE UNDERSTAND THE FOLLOWING:

1. THAT RECORDING A PLAN DOES NOT TRANSFER TITLE OF PROPERTY BETWEEN LANDOWNERS.
2. THAT A DEED MUST BE RECORDED IN ORDER TO TRANSFER THE TITLE OF PROPERTY FROM ONE LANDOWNER TO ANOTHER LANDOWNER.
3. THAT THE PLAN AND DEED MUST BE RECORDED IN THE SAME YEAR IN ORDER FOR THE REVALUATION OF THE PROPERTY TO BE COMPLETED AND TAX BILLS TO BE ADJUSTED ACCORDINGLY BY THE FOLLOWING YEAR.

WITNESS CRAIG J. COZZA

I CERTIFY THAT, TO THE BEST OF MY INFORMATION, KNOWLEDGE AND BELIEF, THE SURVEY AND PLANS SHOWN HEREON ARE CORRECT AND ACCURATE TO THE STANDARDS REQUIRED.

DRAFT

DATE SCOTT A. WELLS, PLS
REG. NO. SU-075231

(SEAL)

I CERTIFY THAT THIS PLAN MEETS ALL ENGINEERING AND DESIGN REQUIREMENTS OF THE APPLICABLE ORDINANCES OF THE TOWNSHIP OF SOUTH FAYETTE, EXCEPT AS DEPARTURES HAVE BEEN AUTHORIZED BY THE APPROPRIATE OFFICIALS OF THE MUNICIPALITY.

DATE NAME
(SEAL) REGISTRATION NUMBER

THE BOARD OF COMMISSIONERS OF THE TOWNSHIP OF SOUTH FAYETTE GIVES NOTICE THAT, IN APPROVING THIS PLAN FOR RECORDING, THE TOWNSHIP OF SOUTH FAYETTE ASSUMES NO OBLIGATION TO ACCEPT THE DEDICATION OF ANY STREETS, LAND, OR PUBLIC FACILITIES AND HAS NO OBLIGATION TO IMPROVE OR MAINTAIN SUCH STREETS, LAND OR FACILITIES.

TOWNSHIP MANAGER PRESIDENT, BOARD OF COMMISSIONERS

THE TOWNSHIP OF SOUTH FAYETTE AGREES NOT TO ISSUE BUILDING PERMITS UNTIL THE 'PLANNING MODULE FOR LAND DEVELOPMENT' HAS BEEN APPROVED IN ACCORDANCE WITH THE REQUIREMENTS OF THE PENNSYLVANIA DEPARTMENT OF ENVIRONMENTAL PROTECTION.

DATE AUTHORIZED MUNICIPAL OFFICIAL

REVIEWED BY THE PLANNING COMMISSION OF THE TOWNSHIP OF SOUTH FAYETTE, THIS ____ DAY OF _____, 20 ____.

TOWNSHIP MANAGER CHAIRPERSON, PLANNING COMMISSION

APPROVED BY THE BOARD OF COMMISSIONERS OF THE TOWNSHIP OF SOUTH FAYETTE, BY RESOLUTION, THIS ____ DAY OF _____, 20 ____.

TOWNSHIP MANAGER PRESIDENT, BOARD OF COMMISSIONERS

(SEAL)

REVIEWED BY THE ALLEGHENY COUNTY DEPARTMENT OF ECONOMIC DEVELOPMENT ON THIS ____ DAY OF _____, 20 ____.

(SEAL) DIRECTOR

AS OF THE DATE OF THIS PLAN'S APPROVAL BY THE APPROVING AUTHORITY, NO DEVELOPMENT OF ANY LAND CONTAINED IN THIS SUBDIVISION OR LAND DEVELOPMENT FOR ANY PURPOSE REQUIRING SANITARY SEWAGE FACILITIES IS PLANNED. NO PORTION OF THIS PROPERTY HAS BEEN APPROVED BY THE MUNICIPALITY OR THE DEPARTMENT OF ENVIRONMENTAL PROTECTION FOR THE INSTALLATION OF SEWAGE DISPOSAL FACILITIES. NO SEWAGE PERMIT WILL BE ISSUED FOR THE INSTALLATION, CONSTRUCTION, CONNECTION TO OR USE OF ANY SEWAGE COLLECTION, CONVEYANCE, TREATMENT OR DISPOSAL SYSTEM UNLESS THE MUNICIPALITY AND THE DEP HAVE BOTH APPROVED SEWAGE FACILITIES PLANNING FOR THE PROPERTY INCLUDED IN THIS PLAN IN ACCORDANCE WITH THE PENNSYLVANIA SEWAGE FACILITIES ACT (35 P.S. § 750.1 ET SEQ.) AND REGULATIONS PROMULGATED THEREUNDER. PRIOR TO THE TRANSFER OF ANY LOT OR PROPERTY INCLUDED IN THIS PLAN, ANY PURCHASER SHOULD CONTACT APPROPRIATE OFFICIALS OF THE MUNICIPALITY, WHICH IS CHARGED WITH ADMINISTERING THE SEWAGE FACILITIES ACT, TO DETERMINE WHAT SEWAGE FACILITIES PLANNING IS REQUIRED AND THE PROCEDURE AND REQUIREMENTS FOR OBTAINING APPROPRIATE PERMITS OR APPROVALS.

A HIGHWAY OCCUPANCY PERMIT IS REQUIRED PURSUANT TO SECTION 420 OF THE ACT OF JUNE 1, 1945 (P.L. 1242, NO. 428), KNOWN AS THE "STATE HIGHWAY LAW" BEFORE DRIVEWAY ACCESS TO A STATE HIGHWAY IS PERMITTED.

REVISION RECORD		
NO	DATE	DESCRIPTION
1	10-17-2023	REVISED PER SOUTH FAYETTE TOWNSHIP AND ALLEGHENY COUNTY COMMENTS

BEING A SUBDIVISION AND CONSOLIDATION PLAN OF REVISED PARCEL A-1 OF THE FIRST REVISION TO THE KOSKY PLAN OF LOTS RECORDED IN P.B.V. 274, PG. 44; LOT 2 OF THE SCHNEIDER PLAN RECORDED IN P.B.V. 130, PG. 133; AND TAX PARCEL 256-L-1 RECORDED IN D.B.V. 10562, PG. 390.

CEC
Civil & Environmental Consultants, Inc.
700 Cherrington Parkway · Moon Township, PA 15108
Ph: 412.429.2324 · 800.365.2324 · Fax: 412.429.2114
www.cecinc.com

**SECOND REVISION TO THE
KOSKY PLAN OF LOTS
TOWNSHIP OF SOUTH FAYETTE
ALLEGHENY COUNTY, PENNSYLVANIA**

DRAWN BY: RWO CHECKED BY: CMM APPROVED BY: SAW
DATE: 07-14-2023 DWG SCALE: N/A PROJECT NO: 333-642

PREPARED FOR:
CE-SF, LP

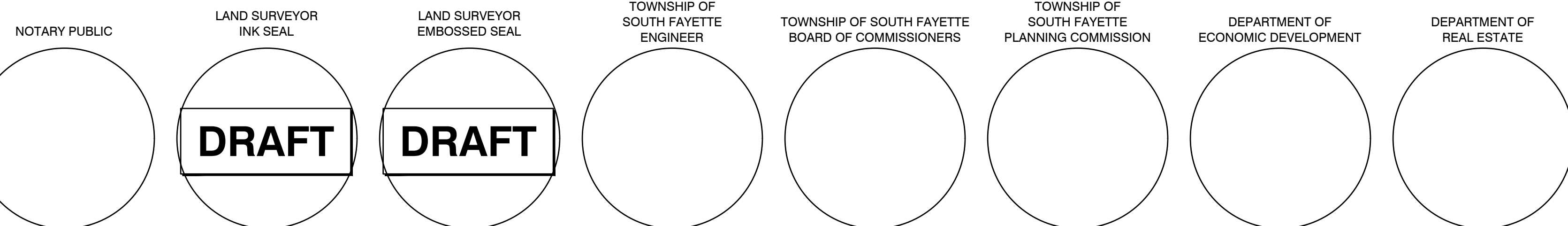
DRAWING NO.:
SUB
SHEET 1 OF 3

RECORDED IN THE OFFICE OF THE DEPARTMENT OF REAL ESTATE OF THE COUNTY OF ALLEGHENY, COMMONWEALTH OF PENNSYLVANIA, IN PLAN BOOK VOLUME _____, PAGE(S) _____.

GIVEN UNDER MY HAND AND SEAL THIS ____ DAY OF _____, 20 ____.

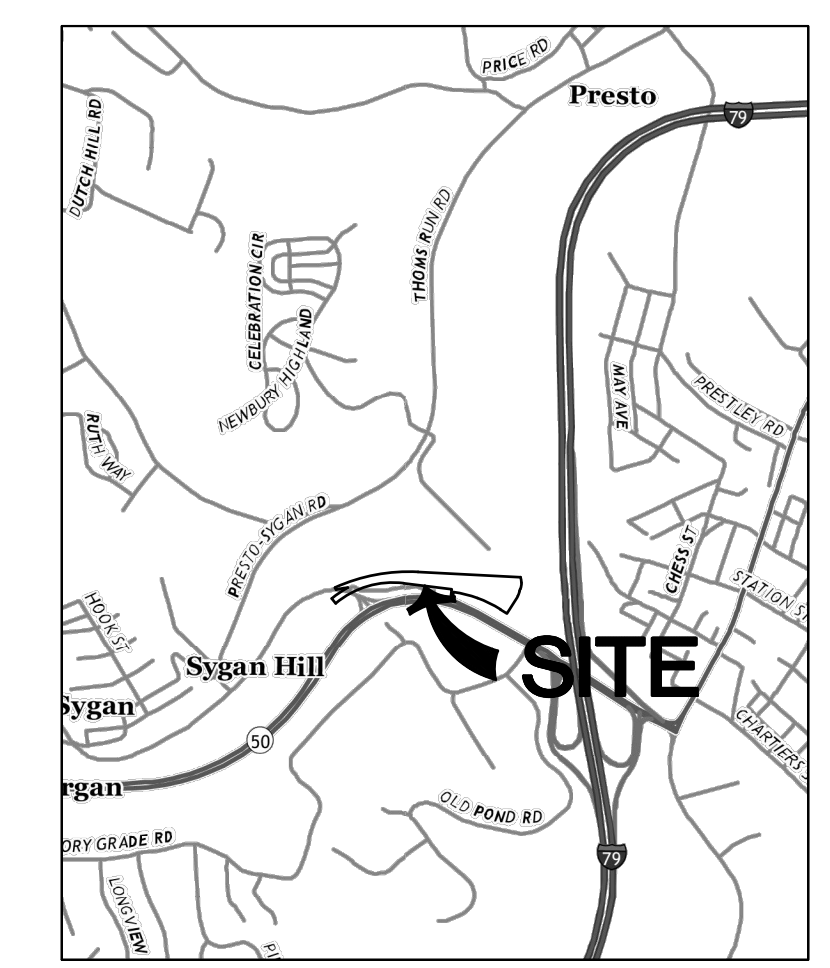
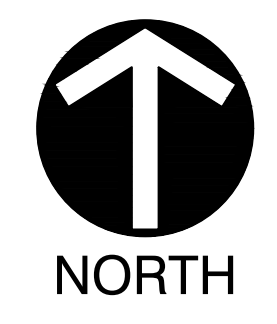
(SEAL)

DEPARTMENT OF REAL ESTATE



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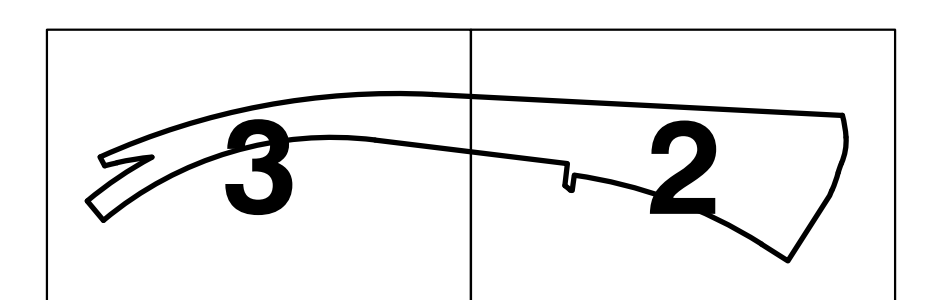
REVISION RECORD		
NO	DATE	DESCRIPTION
1	10-17-2023	REVISED PER SOUTH FAYETTE TOWNSHIP AND ALLEGHENY COUNTY COMMENTS
2		
3		
4		



VICINITY MAP
SCALE: 1" = 200'

SURVEYORS NOTES:

- PLAN NORTH IS BASED UPON PENNSYLVANIA STATE PLANE NAD83-2011, SOUTH ZONE, AS DETERMINED BY CIVIL & ENVIRONMENTAL CONSULTANTS, INC., USING SURVEY GRADE GPS MEASUREMENTS AND OPUS POST-PROCESSING.
- PROPERTY IS LOCATED IN FLOOD ZONE AE (REGULATORY FLOODWAY); ZONE AE (SPECIAL FLOOD HAZARD AREAS WITH BASE FLOOD ELEVATIONS DETERMINED); ZONE X (OTHER AREAS OF FLOOD HAZARD; 0.2% ANNUAL CHANCE FLOOD HAZARD, AREAS OF 1% ANNUAL CHANCE FLOOD WITH AVERAGE DEPTH LESS THAN ONE FOOT OR WITH DRAINAGE AREAS OF LESS THAN ONE SQUARE MILE); AND ZONE X (AREA OF MINIMAL FLOOD HAZARD) AS DEPICTED ON FLOOD INSURANCE RATE MAP 4999030435H AND 42003C0451H, EFFECTIVE DATE 09-26-2014, ALLEGHENY COUNTY, PENNSYLVANIA. THE FLOOD ZONE SHOWN ARE SCALED FROM THE F.I.R.M. COMMUNITY PANEL AND ARE APPROXIMATE.
- SEE COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF TRANSPORTATION DRAWINGS ESTABLISHING AND RE-ESTABLISHING LIMITED ACCESS HIGHWAY AND AUTHORIZING AND RE-AUTHORIZING CONDEMNATION OF RIGHT OF WAY OF LEG. ROUTE 1198, SECTION 1 RW DATED SEPTEMBER 1974.
- THE APPROXIMATE LOCATION OF SANITARY SEWER EASEMENTS ON LOT 2 ARE SHOWN BASED ON D.B.V. 17543 PG. 349



KEY MAP
1" = 500'

DRAFT



BEING A SUBDIVISION AND CONSOLIDATION PLAN OF REVISED PARCEL A-1 OF THE FIRST REVISION TO THE KOSKY PLAN OF LOTS RECORDED IN P.B.V. 274, PG. 44; LOT 2 OF THE SCHNEIDER PLAN RECORDED IN P.B.V. 130, PG. 133; AND TAX PARCEL 256-L-1 RECORDED IN D.B.V. 10562, PG. 390.



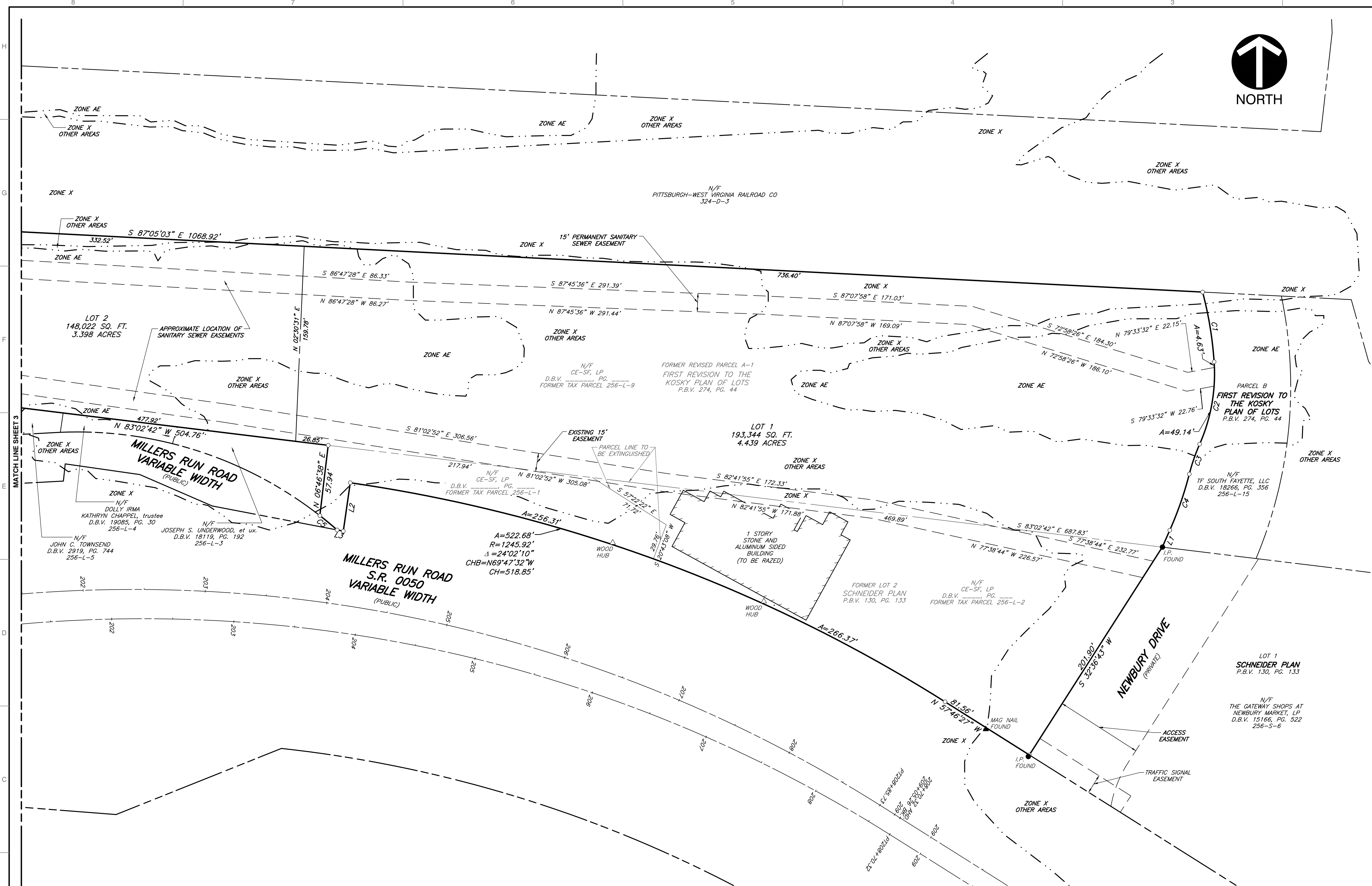
Civil & Environmental Consultants, Inc.
700 Cherrington Parkway · Moon Township, PA 15108
Ph: 412.429.2324 · 800.365.2324 · Fax: 412.429.2114
www.cecinc.com

**SECOND REVISION TO THE
TOWNSHIP OF SOUTH FAYETTE
ALLEGHENY COUNTY, PENNSYLVANIA**

DRAWN BY: **RWO** CHECKED BY: **CMM** APPROVED BY: **SAW**
DATE: **07-14-2023** DWG SCALE: **1" = 40'** PROJECT NO: **333-642**

PREPARED FOR:
CE-SF, LP

DRAWING NO.: **SUB**
SHEET **2** OF **3**



**ZONING INFORMATION-TOWNSHIP OF SOUTH FAYETTE
ZONED C-2 - HIGHWAY COMMERCIAL DISTRICT
PLANNED SHOPPING DISTRICT**

	C-2	C-2 (PLANNED SHOPPING CENTER)
MINIMUM LOT AREA	20,000 SQ. FT.	5 ACRES
MINIMUM LOT WIDTH	60 FT.	200 FT.
MINIMUM FRONT YARD	25 FT.	50 FT.
MINIMUM SIDE YARD	20 FT.	20 FT.
MINIMUM REAR YARD	40 FT.	50 FT.
MAXIMUM BUILDING HEIGHT	60 FT.	60 FT.
MAXIMUM LOT COVERAGE	70%	70%

ORIGINAL AREA TABULATION

	SQ. FT.	ACRES
FORMER TAX PARCEL 256-L-1	10,757	0.247
FORMER LOT 2 FORMER TAX PARCEL 256-L-2	45,527	1.045
FORMER REVISED PARCEL A-1 FORMER TAX PARCEL 256-L-9 (INCLUDES RIGHT OF WAY)	285,082	6.545
TOTAL	341,366	7.837

AREA TABULATION

	SQ. FT.	ACRES
LOT 1	193,344	4.439
LOT 2	148,022	3.398
TOTAL	341,366	7.837

CURVE TABLE

CURVE #	RADIUS	DELTA	LENGTH	CHL	CHB
C1	301.00'	11°01'48"	57.95'	57.86'	S 09°22'41" E
C2	143.00'	27°40'42"	69.08'	68.41'	S 09°58'35" W
C3	157.00'	9°28'17"	25.95'	25.92'	S 19°04'47" W
C4	289.00'	9°39'42"	48.73'	48.68'	S 19°10'29" W
C5	322.68'	3°15'47"	18.38'	18.37'	N 50°20'38" W

LINE TABLE

LINE #	DIRECTION	LENGTH
L1	S 24°00'20" W	15.07'
L2	S 08°11'23" W	40.00'
L3	N 79°08'22" W	5.14'

SURVEYOR:

CIVIL & ENVIRONMENTAL CONSULTANTS, INC.
700 CHERRINGTON PARKWAY
MOON TOWNSHIP, PA 15108
CONTACT: SCOTT A. WELLS, P.L.S.
PHONE NUMBER: 412-429-2324
EMAIL: SWELLS@CECINC.COM

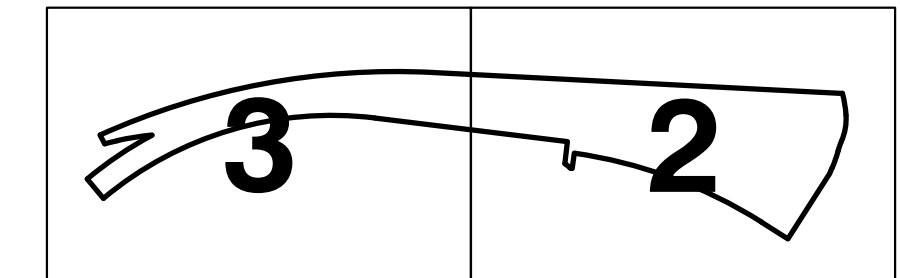
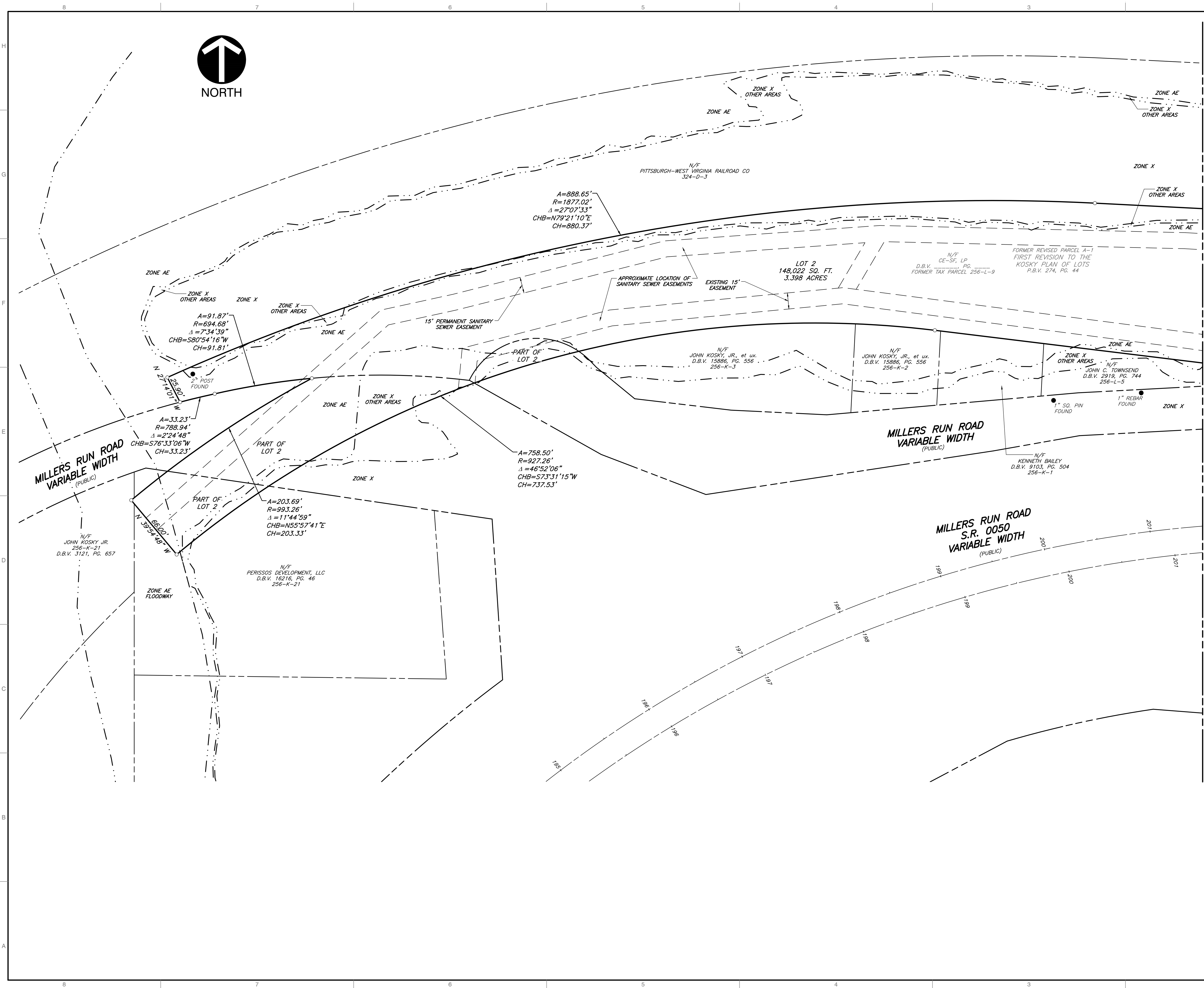
PROPERTY OWNER:

CE-SF, LP
295 MYOMA ROAD
MARS, PA 16046
CONTACT: CRAIG J. COZZA
PHONE NUMBER: (412)-381-7002

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REVISION RECORD		
NO	DATE	DESCRIPTION
1	10-17-2023	REVISED PER SOUTH FAYETTE TOWNSHIP AND ALLEGHENY COUNTY COMMENTS



KEY MAP
1" = 500'

DRAFT



BEING A SUBDIVISION AND CONSOLIDATION PLAN OF REVISED PARCEL A-1 OF THE FIRST REVISION TO THE KOSKY PLAN OF LOTS RECORDED IN P.B.V. 274, PG. 44; LOT 2 OF THE SCHNEIDER PLAN RECORDED IN P.B.V. 130, PG. 133; AND TAX PARCEL 256-L-1 RECORDED IN D.B.V. 10562, PG. 390.



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**SECOND REVISION TO THE
KOSKY PLAN OF LOTS
TOWNSHIP OF SOUTH FAYETTE
ALLEGHENY COUNTY, PENNSYLVANIA**

DRAWN BY:	RWO	CHECKED BY:	CMM	APPROVED BY:	SAW
DATE:	07-14-2023	DWG SCALE:	1" = 40'	PROJECT NO.:	333-642

PREPARED FOR:
CE-SF, LP

DRAWING NO.:
SUB

SHEET 3 OF 3

P:\130-001\133-642\1-Survey\DWG\133642-SF01-02.dwg (10/17/2023 4:14 PM)

THIS DEED

MADE the 14th day of January, 2022

BETWEEN

C. Hackett Holdings, LLC, a Pennsylvania limited liability company
(hereinafter called "Grantor")

AND

CE-S.F. One, LP, a Pennsylvania limited partnership
(hereinafter called "Grantee")

WITNESSETH, that the said Grantor in consideration of One Million Five Hundred Sixty-Nine Thousand Two Hundred Fifty and no/100 Dollars (\$1,569,250.00), paid to the Grantor by the Grantee, receipt of which is hereby acknowledged, does grant, bargain, sell, and convey to the said Grantee, its successors and assigns, all of the Grantor's right, title, and interest in and to the following property:

ALL THAT CERTAIN lot or tract of land situate in the Township of South Fayette, County of Allegheny and Commonwealth of Pennsylvania, being known as Lot No. 2, as shown on a certain plan entitled Schneider Plan, recorded in the Department of Real Estate Office of Allegheny County, Pennsylvania in Plan Book Volume 130, Page 133.

AND

ALL THAT CERTAIN lot or piece of ground situate in the Township of South Fayette, County of Allegheny and Commonwealth of Pennsylvania, bounded and described as follows:

BEGINNING at a point on the Northeasterly side of State Highway L.R. 545 (also known as Traffic Route 28) at the corner of land now or late of Humble Oil and Refining Co.; thence along the Northeasterly side of said State Highway, Northwestwardly by the arc of a circle curving to the left, having a radius of 3367.10 feet, an arc distance of 297.75 feet to a point on line of land now or late of Anelita Ferri and Angelina Mals; thence by said land North 19 degrees, 05 minutes, 35 seconds East, 102.41 feet to a point on line of land now or late of Humble Oil and Refining Company; thence by said land the following two courses and distances; South 58 degrees, 59 minutes, 55 seconds East 310.50 feet to a point; thence South 25 degrees, 57 minutes, 05 seconds West 55.77 feet to the Northeasterly side of said State Highway at the place of beginning.

SUBJECT TO the condemnation of a portion of the subject property by the Commonwealth of Pennsylvania, Department of Transportation of right of way for Legislative Route 1138 of the Court of Common Pleas of Allegheny County, Pennsylvania, at No. 1717 October Term, 1971.

ALSO DESCRIBED AS all that certain lot or parcel of land situate in the Township of South Fayette, County of Allegheny, Commonwealth of Pennsylvania, being a portion of Block and Lot No. 256-L-2, more particularly bounded and described as follows:

Beginning at a point on the northerly right of way line of Miller Run Road, S.R. 0050, variable width, said point being at the southeast corner of property now or formerly Anthoni Mals Peterson (Tax Parcel 256-L-1); thence along the dividing line of property now or formerly Anthoni Mals Peterson and property herein described, North 20°43'08" East, 29.76' to a point on the former southerly line of Lot 2 of the Schneider Plan, recorded in Plan Book Volume 130, Page 133; thence along the former southerly line of Lot 2 of the Schneider Plan and through property now or formerly C. Hackett Holdings, LLC, (Tax Parcel 256-L-2), South 57°22'22" East, 225.98' to a point on the northerly right of way line of Miller Run Road, S.R. 0050, variable width; thence along the northerly right of way line of Miller Run Road, S.R. 0050, by an arc of a circle deflecting to the left in a northwestwardly direction, having a radius of 1245.92', an arc distance of 222.05' (chord bearing and distance, North 64°55'04" West, 221.76') to a point at the place of beginning.

Bearings based on First Revision to the Kosky Plan of Lots, recorded in Plan Book Volume 274, Page 44.

Contains 2,559 Sq. Ft. or 0.0587 Acres

THE ABOVE DESCRIBED PROPERTY TOGETHER BEING BLOCK AND LOT 256-L-2.

TOGETHER with and subject to all rights, duties and obligations set forth in the certain Easement Agreement, dated December 14, 2015, and recorded January 13, 2016, in Plan Book Volume 16256, page 110, being an Access Drive Easement.

BEING the same property which Richard Schneider and Jane Schneider, husband and wife, by Corrective Deed dated January 10, 2022 and recorded on January __, 2022 in the Department of Real Estate of Allegheny County, Pennsylvania, in Deed Book Volume _____, Page _____ granted and conveyed C. Hackett Holdings, LLC.

UNDER AND SUBJECT TO coal and mining rights and all rights and privileges incident to the mining of coal heretofore conveyed, excepted, or reserved by instruments of record; the right of surface, lateral, or subjacent support; or any surface subsidence; oil and gas and minerals and all rights incident to the extraction or development of oil and gas or minerals heretofore conveyed, leased, excepted, or reserved by instruments of record; and all easements, rights of way, and restrictions as contained in prior instruments of record and/or as installed or located on the premises and all other matters of record appearing prior hereto.

With the appurtenances thereto: **TO HAVE AND TO HOLD** the same to and for the use of the said Grantee, its successors and assigns forever, and the Grantor for its successors and assigns hereby covenants and agrees that it will **SPECIALLY** warrant title to the property hereby conveyed.

NOTICE: THIS DOCUMENT MAY NOT/DOES NOT SELL, CONVEY, TRANSFER, INCLUDE, OR INSURE THE TITLE TO THE COAL AND RIGHT OF SUPPORT UNDERNEATH THE SURFACE LAND DESCRIBED OR REFERRED TO HEREIN, AND THE OWNER OR OWNERS OF SUCH COAL MAY HAVE/HAVE THE COMPLETE LEGAL RIGHT TO REMOVE ALL OF SUCH COAL AND, IN THAT CONNECTION, DAMAGE MAY RESULT TO THE SURFACE OF THE LAND AND ANY HOUSE, BUILDING, OR OTHER STRUCTURE ON OR IN SUCH LAND. THE INCLUSION OF THIS NOTICE DOES NOT ENLARGE, RESTRICT, OR MODIFY ANY LEGAL RIGHTS OR ESTATES OTHERWISE CREATED, TRANSFERRED, EXCEPTED, OR RESERVED BY THIS INSTRUMENT. [This notice is set forth in the manner provided in Section 1 of the Act of July 17, 1957, P.L. 984, as amended, and is not intended as notice of unrecorded instruments, if any.]

WITNESS the hand and seal of the said Grantor.

WITNESS:

C. Hackett Holdings, LLC

[Signature]

By [Signature]
Charles Hackett, Member

Commonwealth of Pennsylvania)
County of Allegheny) ss:

On this, the 14th day of January, 2022, before me, a Notary Public, the undersigned officer, personally appeared Charles Hackett, who acknowledged himself to be the Member of C. Hackett Holdings, LLC, a Pennsylvania limited liability company, and that he as such Member, being authorized to do so, executed the foregoing deed for the purposes therein contained by signing the name of the corporation by himself as such Member.

IN WITNESS WHEREOF, I have hereunto set my hand and official seal.

[Signature]
Notary Public

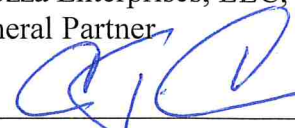
My commission expires:

Commonwealth of Pennsylvania - Notary Seal
Thomas H. Ayoub III, Notary Public
Allegheny County
My commission expires February 21, 2025
Commission number 1004840
Member, Pennsylvania Association of Notaries

NOTICE: THE UNDERSIGNED, AS EVIDENCED BY THE SIGNATURE[S] TO THIS NOTICE AND THE ACCEPTANCE AND RECORDING OF THIS DEED, IS/ARE FULLY COGNIZANT OF THE FACT THAT THE UNDERSIGNED MAY NOT BE OBTAINING THE RIGHT OF PROTECTION AGAINST SUBSIDENCE, AS TO THE PROPERTY HEREIN CONVEYED, RESULTING FROM COAL MINING OPERATIONS AND THAT THE PURCHASED PROPERTY, HEREIN CONVEYED, MAY BE PROTECTED FROM DAMAGE DUE TO MINE SUBSIDENCE BY A PRIVATE CONTRACT WITH THE OWNERS OF THE ECONOMIC INTEREST IN THE COAL. THIS NOTICE IS INSERTED HEREIN TO COMPLY WITH THE BITUMINOUS MINE SUBSIDENCE AND LAND CONSERVATION ACT OF 1966, AS AMENDED 1980, OCT. 10, P.L. 874, NO. 156, § 1.

WITNESS:



CE-S.F. One, LP
By: Cozza Enterprises, LLC,
its General Partner
By: 
Craig J. Cozza, Managing Member

CERTIFICATE OF RESIDENCE

I hereby certify that (1) FOR THE PURPOSE OF DELIVERY OF TAX STATEMENTS ONLY, the precise residence of the Grantee is P.O. Box 453, Carnegie, PA 15106,

and (2) FOR ALL OTHER PURPOSES (including delivery of assessment change notices) the precise residence of Grantee is P.O. Box 453, Carnegie, PA 15106.

Witness the due execution hereof this 14th day of January, 2022



Grantee/Agent for Grantee

AFTER RECORDING, PLEASE RETURN TO:

Pioneer Land Settlement, Inc.
710 Fifth Ave. – Suite 2000
Pittsburgh, PA 15219

PURCHASE AND SALE AGREEMENT

THIS PURCHASE AND SALE AGREEMENT ("Agreement") by and between PETERSON ANTHONI MALS ("Seller") and CE-South Fayette, LP or related assigns, a limited liability company ("Purchaser") is made and entered into as of the last date this Agreement is executed by Seller or Purchaser (the "Effective Date").

Recitals:

WHEREAS, Seller is the owner of a certain property identified as Millers Run Rd in the City of Bridgeville, County of 946 South Fayette, Commonwealth of Pennsylvania identified as Block and Lot Parcel ID 0256-L-00001-0000-00, as such property is more fully described at Deed Book Volume 10562, page 390 (the "Property"); and

WHEREAS, Seller desires to sell and Purchaser desires to purchase the Property, pursuant to the terms, provisions, and conditions herein.

NOW, THEREFORE, intending to be legally bound the parties hereto agree as follows:

1. PURCHASE AND SALE OF PROPERTY.

Seller agrees to sell to Purchaser, and Purchaser agrees to purchase from Seller, subject to the terms and conditions of this Agreement, the Property. The Property shall be conveyed together with all privileges, rights, easements and appurtenances belonging to such land, and all right, title and interest (if any) of Seller in and to any streets, alleys, passages, and other rights-of-way or appurtenances included in, adjacent to or used in connection with such land, and all right, title and interest (if any) of Seller in all mineral and development rights appurtenant to such land and with all of the rights and privileges attributable to ownership of the Property.

2. PURCHASE PRICE AND DEPOSIT.

2.1 The purchase price for the Property shall be [REDACTED] (the "Purchase Price").

2.2 Within three (5) days of the Effective Date, Purchaser shall deposit with Pioneer Land Settlement, Inc. (hereinafter "Title Company") as escrow agent, a deposit in the amount of [REDACTED] (the "Deposit") to be held in a non-interest bearing account.

2.3 At Closing, the Deposit shall be applied to the Purchase Price.

3. TITLE.

3.1 Title to the Property shall be conveyed to Purchaser at Closing in fee simple by general Warranty Deed, in a form and substance satisfactory to Purchaser's counsel.

3.2 Purchaser shall obtain a Commitment for Title Insurance from the Title Company, committing to insure upon the payment of a requisite premium at standard rates that Purchaser shall own good and indefeasible fee simple title to the Property, subject only to the

Permitted Exceptions, as defined herein.

3.3 The term "Permitted Exceptions", as used herein, shall mean (i) the lien of real estate taxes not yet due and payable, (ii) all matters revealed in the Title Commitment obtained by Purchaser and approved by Purchaser, (iii) all existing building, zoning and other city, state, county or federal laws, codes and regulations affecting the Property, (iv) any existing general utility easements serving the Property, provided such existing utility easements would not materially interfere with Purchaser's intended use of the Property as determined by Purchaser in its sole discretion, and (v) any title exception created directly by any act or omission of Purchaser or its representatives, agents, employees or invitees.

3.4 Notwithstanding anything to the contrary in this agreement, Seller shall pay all costs of clearing title.

4. DUE DILIGENCE PERIOD.

4.1 Purchaser, at Purchaser's sole expense, shall have the right for a period of [REDACTED] days from the Effective Date (the "Due Diligence Period"), at any time, to perform any due diligence at its sole cost and expense that it deems proper, including but not limited to, survey and title review, environmental review, structural review, roof evaluation, electrical and plumbing review, and zoning review. Purchaser may elect, at its sole discretion, during the Due Diligence Period, to terminate this Agreement for any reason (or for no reason whatsoever) and receive the prompt refund of the Deposit. Purchaser shall elect to terminate this Agreement by providing written notice delivered to Seller prior to the expiration of the Due Diligence Period notifying Seller that Purchaser is terminating this Agreement. In the absence of such notice, this Agreement shall remain in full force and effect.

4.2 Seller shall provide to Purchaser, within five (5) days after the Effective date of this Agreement, to the extent such are available to Seller, a copy of all plans, drawings, and blueprints pertaining to the Property, any existing title insurance policies covering the Property, a copy of any site plans and/or surveys for the Property, and a copy of any environmental reports.

4.3 The Purchaser shall have one (1) successive option to extend the term of the Due Diligence Period for periods of thirty (30) days each. To exercise an option to extend the term of the Due Diligence Period, Purchaser must notify Seller in writing to be received by Seller on or before the end of the Due Diligence Period, as extended, pursuant to the notice provisions set forth below in this Agreement.

5. REPRESENTATIONS AND WARRANTIES OF SELLER.

Seller hereby represents and warrants the following to the Purchaser as of the date Seller signs this Agreement and as of the Closing:

5.1 Seller is the record owner in fee simple of the Property, and the Property will be on the Closing date free and clear of all liens and encumbrances except for Permitted Exceptions, as defined herein.

5.2 Seller possesses all requisite power and authority to enter into and perform this Agreement and to carry out the transactions contemplated herein. The execution and delivery by Seller of this Agreement and the performance and consummation by Seller of the transaction

contemplated by this Agreement have been duly and validly authorized by all requisite and necessary company and other internal action on the part of Seller.

5.3 No suit, action, arbitration, or legal, administrative, or other proceedings, including but not limited to condemnation proceeding, is pending or has been threatened against the Property or against the Seller with respect to the Property.

5.4 No bankruptcy, insolvency, rearrangement, or similar action or proceedings, whether voluntary or involuntary, is pending or threatened against Seller, or any partner of Seller and Seller has no intention of filing or commencing any such action or proceeding.

5.5 There are no existing or pending contracts of sale, leases, options to purchase, or rights of first refusal (or the like) with respect to the Property.

5.6 Seller is not a "foreign person" as defined in the Foreign Investment in Real Property Tax Act of 1980, as amended.

5.7 The Property is not subject to any protest or appeal proceedings related to real property taxes.

5.8 Seller has not received any written notice indicating that the Property is in violation, or that with the giving of notice or the passage of time would be in violation, of any applicable law, enactment, statute, code, ordinance, rule, regulation, judgment, writ, injunction, authorization, covenant, condition, restriction or agreement, or other direction or requirement of any governmental authority.

5.9 Neither Seller nor any affiliate or agent or contractor of Seller has disposed of or otherwise released any Hazardous Substances on the Property. To the best of Seller's knowledge, there are no Hazardous Substances present on the Property. Seller further warrants that until termination of this Agreement or delivery of possession of the Property to Purchaser, neither Seller nor any agent of Seller will cause or permit any Hazardous Substance to be disposed of or released or present on, over, beneath, in or upon the Property or to exist on or within any portion of the Property. "Hazardous Substances" shall mean asbestos (including asbestos in friable form), polychlorinated biphenyls, petroleum products, any flammable explosives, radioactive materials, hazardous materials, hazardous wastes, hazardous or toxic substances or related materials as defined in the Comprehensive Environmental Response, Compensation and Liability Act of 1980, as amended (42 U.S.C. §9601, et seq.), the Hazardous Materials Transportation Act, as amended, (49 U.S.C. §1801, et seq.), the Resource Conservation and Recovery Act, as amended (42 U.S.C. §6901, et seq.), the Toxic Substances Control Act, as amended (15 U.S.C. §2601, et seq.), any Environmental Laws. "Environmental Laws" means any federal, state or local statutes, laws, regulations, rules, decrees, orders, judgments, stipulations, ordinances, policies or common law related to the protection of human health and the environment or the use, handling, treatment, storage, disposal, release, remediation or transportation, or exposure of persons to, Hazardous Substances.

5.10 The representations and warranties of this Section 5 shall survive Closing.

6. CLOSING.

6.1 The consummation of the contemplated transaction (the "Closing") shall be held at the offices of Pioneer Land Settlement, Inc., in Pittsburgh, Pennsylvania, not later than thirty (30) days after expiration of the Due Diligence Period, including any extensions, time being of the essence. The exact date and time of the Closing shall be designated by mutual agreement of the Seller and Purchaser upon notice to Seller of not less than five (5) days. The Title Company shall be responsible at the Closing for preparing the settlement statement, causing all documents to be recorded, disbursing all closing proceeds, and otherwise conducting settlement.

6.2 The following apportionments shall be made between the parties at the Closing:

(a) Real estate taxes, personal property taxes, special assessments, if any, on the basis of the fiscal or calendar period for which assessed.

(b) Water and sewer service charges and charges for gas, electricity, telephone and all other public utilities. If there are meters measuring the consumption of water, gas or electric current, Seller shall, not more than one day prior to the Closing date, if possible, cause such meters to be read, and shall pay all utility bills for which Seller is liable upon receipt of statements therefor. Purchaser shall be responsible for causing such utilities and services to be changed to its name and shall be liable for and shall pay all utility bills for services rendered after the Closing.

(c) All other charges and fees customarily prorated and adjusted in similar transactions in Pennsylvania.

6.3 At the Closing, Seller shall deliver to Purchaser, the following:

(a) A recordable Warranty Deed as required by Section 3.1 of this Agreement, conveying the Property in fee simple to Purchaser.

(b) A certificate, dated as of the Closing date, to establish that Seller is not a foreign person for the purposes of the Foreign Investment in Real Property Tax Act.

(c) Exclusive physical possession of the Property in its "AS IS" condition with all personal property removed, together with all books and records in Seller's possession or control and all keys.

(d) Such customary owner's title affidavits and gap indemnities as may be required by the Title Company in order to issue the title policy subject only to the Permitted Exceptions and without exception for parties in possession, mechanics' or materialmen's liens, unrecorded easements or matters first appearing of record after the effective date of the most recent Title Commitment but prior to the conveyance of the Property to Purchaser.

(e) If Seller is a business entity other than an individual, a Pennsylvania Good Standing Certificate, copies of the organizational documents for the Seller, and resolutions of Seller approving this Agreement and the transaction contemplated hereby and authorizing the execution and delivery of this Agreement, the completion of the transaction contemplated hereby and the execution and delivery of all documents required to be executed and delivered by Seller.

(f) Such other documents, instruments and affidavits as may be reasonably requested by Purchaser or the Title Company to effectuate the transaction contemplated by this Agreement and to induce the Title Company to insure title to the Property as described herein.

6.4 At the Closing, Purchaser shall deliver to Seller, the following:

(a) The balance of the Purchase Price, less the Deposit and subject to the prorations and credits set forth herein, payable in certified funds or by Federal Reserve Bank wire transfer to the Title Company on or prior to Closing.

(b) Such other documents, instruments and affidavits as may be reasonably requested by Seller or the Title Company to effectuate the transaction contemplated by this Agreement and to induce the Title Company to insure title to the Property as described herein.

6.5 Purchaser shall pay the costs and expenses associated with the following: (i) all costs of Purchaser's due diligence, including fees due its consultants and attorneys, (ii) all lenders' fees related to any financing to be obtained by Purchaser, (iii) all recording and filing charges in connection with the instruments by which Seller conveys the Property, (iv) all premiums and charges of the Title Company for the Title Commitment and the Owner's (and any mortgagee's) Title Policy (including endorsements), (v) the cost of the Survey, (vi) one-half of the transfer taxes, documentary stamp taxes and similar charges, if any, applicable to the transfer of the Property to Purchaser. The obligations of the Purchaser under this Section 6.5 shall survive the Closing (and not be merged therein) or any earlier termination of this Agreement.

6.6 Seller shall pay the costs and expenses associated with the following: (i) all fees due its attorneys and consultants, (ii) all reasonable costs incurred in connection with causing the Title Company to remove any title objections required to be removed or otherwise cured by Seller, (iii) all costs incurred in connection with the satisfaction of monetary liens on the Property, including any costs related to recording of any satisfaction or termination documents, and (iv) one-half of the transfer taxes, documentary stamp taxes and similar charges, if any, applicable to the transfer of the Property to Purchaser, and (v) a customary and reasonable settlement fee. The obligations of the Seller under this Section 6.6 shall survive the Closing (and not be merged therein) or any earlier termination of this Agreement.

7. RISKS OF LOSS; MAINTENANCE OF PROPERTY.

Risk of loss of the Property shall remain upon the Seller until Closing and delivery of possession to Purchaser. Seller shall maintain the Property in as good condition as it is now, except for ordinary wear and tear, until delivery of the same to Purchaser. Seller shall maintain such fire and casualty insurance as it has in force at this time. Purchaser understands that Purchaser may have an insurable interest in the Property upon the signing of this Agreement and, in order to protect Purchaser's own interest in the Property, Purchaser may retain or place in force adequate fire and casualty insurance with extended coverage on the Property as of the Effective Date of this Agreement.

8. EMINENT DOMAIN; CASUALTY.

After the Effective Date, in the event Seller receives any notice of any condemnation proceedings, or other proceedings in the nature of eminent domain, or if any part

of the Property is damaged or destroyed by casualty, Seller will forthwith notify Purchaser of same, and Purchaser shall have the option to: (i) proceed under this Agreement and obtain by assignment or otherwise all damages to which the owner of the Property may be entitled pursuant to the Pennsylvania Eminent Domain Code, or under any insurance policy of Seller, as applicable; or (ii) void this Agreement whereupon no party shall have any further duty or liability to the other. Notwithstanding the foregoing, if the Property is damaged by fire or casualty, and such damage can be repaired or reconstructed prior to the Closing in a good and workmanlike manner to the reasonable satisfaction of Purchaser, the Purchaser shall not have the right to terminate the Agreement.

9. REMEDIES.

9.1 In the event Seller materially fails to perform or breaches any of its representations, warranties or covenants to be performed by Seller under this Agreement, or Seller materially misrepresents any fact or circumstance, Purchaser shall be entitled (a) to enforce specific performance of this Agreement; (b) to bring suit for all damages suffered by reason of such failure and all of Purchaser's costs and expenses, including reasonable attorneys' fees; or (c) to terminate this agreement and have the Deposit and any Additional Deposit returned to Purchaser. Each remedy under this Section 9.1 may be cumulative and not exclusive.

9.2 If Purchaser defaults in its performance of any term, covenant, condition, or obligation under this Agreement, including the obligation of Purchaser to purchase the Property if all conditions precedent to such obligations have been satisfied, Seller shall be entitled to receive as complete liquidated damages the Deposit and any Additional Deposit as liquidated damages. The parties acknowledge that the Deposit and any Additional Deposit represents a reasonable effort to ascertain the damages to Seller in the event of a Purchaser default, which damages are difficult or impossible to quantify. Seller waives all other remedies.

9.3 A failure by either party to perform any act required by it under this Agreement, other than the requirement to close if all conditions have been met, shall not be deemed a default under this Agreement until such party has received written notice from the other party setting forth the alleged failure, and such failure has not been cured within five (5) days of receipt of such notice.

10. BROKERAGE COMMISSION.

Purchaser and Seller acknowledge that no brokerage commission is payable in connection with this transaction. Each of the parties hereto agrees to indemnify and hold the other harmless from claims made by any other broker, attorney or finder claiming through such party for a commission, fee or compensation in connection with this Agreement or the sale of the Property hereunder. The provisions of this Section 10 shall survive Closing.

11. ASSIGNMENT.

11.1 Neither party shall assign or transfer or permit the assignment or transfer of its rights or obligations under this Agreement without the prior written consent of the other, any such assignment or transfer without such prior consent being hereby declared to be null and void; provided, however, that Purchaser shall have the right to assign this Agreement to an Affiliate, whose direct or indirect ownership is at least 51% of the ownership of the Purchaser,

upon written notice to Seller no later than two (2) days prior to the Closing date, and such assignee(s) shall assume Purchaser's rights and obligations under this Agreement.

11.2 In the event either party consents to an assignment of this Agreement by the other for which consent is required, no further assignment shall be made without another written consent from the consenting party, unless the assignment may otherwise be made without consent under this Agreement. An assignment by either Seller or Purchaser of its interest in this Agreement shall not relieve Seller or Purchaser, as the case may be, from its obligations, but this Agreement shall then inure to the benefit of, and be binding on, the assignee's successors, heirs, legal representatives and assigns.

11.3 If Seller or Purchaser reasonably determine that an assignment of this Agreement may be subject to the imposition of realty transfer tax or other applicable taxes, then the parties shall terminate this Agreement effective prior to Closing. In the event of such termination, the parties hereby agree that Seller and Purchaser (or Purchaser's assignee) shall enter into a new purchase agreement immediately following the termination of this Agreement, which shall contain the same terms and conditions as this Agreement, except as otherwise agreed by the parties in advance. In addition to the foregoing, the parties hereby acknowledge and agree that any termination of this Agreement as contemplated by this Section 11.3 shall not constitute a default under this Agreement or result in the disbursement of any portion of the Deposit and any Additional Deposit, and, upon such termination, the Deposit and Any Additional Deposit shall be treated as if they were delivered to Purchaser and repaid to the Title Company. The parties shall execute and deliver such additional documents, instruments and certificates as may be reasonably requested by either party to evidence the transactions described in this Section 11.3.

12. GENERAL PROVISIONS.

12.1 The terms and conditions of this Agreement shall be binding upon and inure to the benefit of the parties hereto and their respective heirs, successors, assigns, and legal representatives.

12.2 Notices and other communications required by this Agreement shall be in writing and (i) delivered by hand with receipt; (ii) sent by recognized overnight delivery service; (iii) sent by certified or registered mail, postage prepaid, with return receipt requested; or (iv) by electronic mail with a confirmation copy sent by another method permitted under this Section. All notices shall be addressed as follows:

If to the Seller:	PETERSON ANTHONI MALS 754 Windows Road, Smicksburg, PA 16256
If to the Purchaser:	Cozza Enterprises LLC 295 Myoma Rd Mars, PA 16046
With Copy To:	Thomas H. Ayoob III, Esquire Thomas H. Ayoob III & Associates, LLC 710 Fifth Avenue, Suite 2000

Pittsburgh, PA 15219
e-mail: tom@pioneerls.com

Notices shall be deemed to be effective upon receipt or refusal of the addressee to accept delivery.

12.3. Whenever used herein, unless expressly provided otherwise, the term "days" shall mean consecutive calendar days, except that if the expiration of any time period measured in days occurs on a Saturday, Sunday, legal holiday, such expiration shall automatically be extended to the next business day.

12.4 This Agreement constitutes the entire agreement between the parties concerning the Property and supersedes all prior agreements or undertakings.

12.5 This Agreement may not be modified except by the written agreement of the parties.

12.6 In the event any one or more of the provisions contained in this Agreement are held to be invalid, illegal, or unenforceable in any respect, such invalidity, illegality, or unenforceability will not affect any other provisions hereof, and this Agreement shall be construed as if such invalid, illegal, or unenforceable provision had not been contained herein.

12.7 The parties acknowledge that each party and its counsel of choice if so desired has had an opportunity to review and revise this Agreement and that the normal rule of construction to the effect that any ambiguities are to be resolved against the drafting party shall not be employed in the interpretation of this Agreement or any amendment or modification hereof or any of the closing documents delivered by Seller or Purchaser hereunder.

12.8 Any paragraph headings or captions contained in this Agreement shall be for convenience of reference only and shall not affect the construction or interpretation of any provisions of this Agreement.

12.9 This Agreement shall be governed by and construed in accordance with the laws of the Commonwealth of Pennsylvania.

12.10 The parties hereby agree to indemnify and defend the Title Company in its role as escrow agent from any and all suits, actions or claims if the Title Company in its role as escrow agent acts in good faith on the written notice and direction of the parties delivered in accordance with the terms hereof.

12.11 The individuals executing this Agreement represent and warrant that they have full authority and/or have been duly authorized by their respective parties to do so on behalf of such parties.

12.12 This Agreement may be executed in separate counterparts, none of which need contain the signatures of all parties, each of which shall be deemed to be an original, and all of which taken together constitute one and the same instrument. It shall not be necessary in making proof of this Agreement to produce or account for more than the number of counterparts containing the respective signatures of, or on behalf of, all of the parties hereto. The exchange of executed copies of this Agreement by electronic mail, portable document format (.pdf) or other

electronic transmission method will constitute effective execution and delivery of this Agreement as to the parties for all purposes, and electronic signatures of the parties shall be deemed to be their original signatures for all purposes.

12.13 NOTICE--THIS DOCUMENT MAY NOT SELL, CONVEY, TRANSFER, INCLUDE OR INSURE THE TITLE TO THE COAL AND RIGHT OF SUPPORT UNDERNEATH THE SURFACE LAND DESCRIBED OR REFERRED TO HEREIN, AND THE OWNER OR OWNERS OF SUCH COAL MAY HAVE THE COMPLETE LEGAL RIGHT TO REMOVE ALL OF SUCH COAL, AND, IN THAT CONNECTION, DAMAGE MAY RESULT TO THE SURFACE OF THE LAND AND ANY HOUSE, BUILDING OR OTHER STRUCTURE ON OR IN SUCH LAND, THE INCLUSION OF THIS NOTICE DOES NOT ENLARGE OR RESTRICT OR MODIFY ANY LEGAL RIGHTS OR ESTATES OTHERWISE CREATED, TRANSFERRED, EXCEPTED OR RESERVED BY THIS INSTRUMENT. (This notice is set forth in the manner provided in Section 1 of the Act of July 17, 1957, P.L. 984, as amended, and is not intended as notice of unrecorded instruments, if any.

IN WITNESS WHEREOF, the parties hereto have executed this Agreement on the dates written below.

Date: 6/23/23

SELLER:

Anthony Mels Peterson

Date: 6/20/23

PURCHASER:

LJC QJ@



Plan Name:	Lafayette 180	File No.	SP-01-2026
Plan Location:	Newbury Drive	Tax I.D. #	0256-L-00002, 0256-L-00001, 0256-L-00009
Project Description	Proposed planned shopping center with parking lot and associated utilities.		

Check Appropriate Box(es)

Land Development Plan	<input checked="" type="checkbox"/>	Subdivision Plan	<input type="checkbox"/>	Conditional Use Plan	<input type="checkbox"/>
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Minor Subdivision	<input type="checkbox"/>	Major Subdivision	<input type="checkbox"/>	Open Space Plan	<input type="checkbox"/>
Preliminary Plan Submission	<input type="checkbox"/>	Final Plan Submission	<input checked="" type="checkbox"/>		

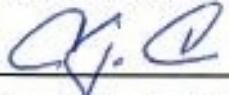
Zoning District(s)	C-2	Property Acreage	5.48 acres	No. Lots/Units	
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Applicant's Name:	Craig Cozza	Phone No.	
Applicant's Address:	295 Myoma Road, Mars, PA 16046	Fax No.	
Applicant's E-Mail:			
Engineer Firm/Name	The Gateway Engineers, Inc.	Phone No.	
Engineer's Address:	100 McMorris Road, Pittsburgh, PA	Fax No.	
Contact Person:	Joseph Galbraith	E-Mail Address	

The following items are reviewed as part of the South Fayette Township application process. Applications submitted WITHOUT these elements will NOT be reviewed by the township.

REQUIRED SUBMISSION ITEMS	Copies	✓ Yes	✓ No	✓ N/A
1) Completed Application Form	1			
2) ACED Subdivision/Land Development Application	1			
3) Maps and Plans				
• Plus PDF of drawings	1			
• Sets Full-Size Plans (24" x 36")	5			
• Sets Half-Size Plans (11" x 17")	5			
4) Agent Authorization Form	1			
5) Application Fee	1			
6) Escrow – Engineer, Solicitor, Inspection	1			
7) Stormwater Management Plan & Calculations (plus PDF)	2			
8) Erosion & Sedimentation Control Plan (plus PDF)	2			
9) Deed, Sales Agreement or Other Ownership	1			

I have familiarized myself with and hereby agree to comply with the subdivision and zoning ordinance of the township as well as to all township rules, regulations and resolutions. I agree to pay the applicable fee(s).

Signature of Applicant:  Date: 01/14/2026

If applicant is not the property owner, an Agent Authorization Form must be attached.



SOUTH FAYETTE T O W N S H I P

A Community Growing Together

Agent Authorization Form

www.southfayettepa.com

Name of Property Owners: Craig Cozza

Property Tax Map Number: 0256-L-00002, 0256-L-000001, 026-L-00009

Property/Project(s): Lafayette 180 - Planned Shopping Center

This application/phase only

All related applications and phases

The above named property owner hereby appoints: The Gateway Engineers, Inc.

as its agent and authorizes said agent to apply for and process the above mentioned development plan/variance on his/her behalf. Agent is further authorized to sign all necessary documentations for such purposes, including acceptance of conditions imposed by the Board of Commissioners upon arrival of the plan. This authorization shall remain in full force and effect until written notice of revocation is delivered to the South Fayette Township Manager.

SIGNED AND SEALED, intending to be legally bound on this date of:

Craig J. Cozza

Owner(s) Signature:  (SEAL)

NOTE:

1. THESE PLANS AS SUBMITTED ARE INTENDED TO COMPLY WITH ALL REGULATIONS, STANDARDS, AND ORDINANCES OF THE SOUTH FAYETTE TOWNSHIP. ANY DEVIATION FROM THOSE REGULATIONS, STANDARDS AND ORDINANCES IS UNINTENTIONAL AND AS SUCH, IN THE EVENT THAT A CONFLICT IS DISCOVERED, IT IS UNDERSTOOD AND AGREED THAT THE SOUTH FAYETTE TOWNSHIP'S REGULATIONS, STANDARDS AND ORDINANCES WILL APPLY.

LAFAYETTE 180 - VARIANCES GRANTED:

SIGNATURE OF PROJECT ENGINEER

ON JULY 26, 2023 A VARIANCE HAS BEEN GRANTED BY THE SOUTH FAYETTE TOWNSHIP ZONING HEARING BOARD TO SECTION 240-51(A)(1) TO ALLOW THE LOT AREA OF 4.4 ACRES DEPARTURE FROM THE REQUIRED 5.0 ACRES FOR A PLANNED SHOPPING CENTER.

ON JULY 26, 2023 A VARIANCE HAS BEEN GRANTED BY THE SOUTH FAYETTE TOWNSHIP ZONING HEARING BOARD TO SECTION 240-111(J)(2) REDUCING THE SETBACK ON MILLERS RUN ROAD SIDE PARKING SETBACK FROM 20-FEET TO 1-FOOT.

ON JULY 26, 2023 A VARIANCE HAS BEEN GRANTED BY THE SOUTH FAYETTE TOWNSHIP ZONING HEARING BOARD TO SECTION 240-111(J)(3) REDUCING THE FRONT YARD PARKING SETBACK FROM 20-FEET TO 8-FEET, AS IT RELATED TO NEWBURY DRIVE.

ON JULY 26, 2023 A VARIANCE HAS BEEN GRANTED BY THE SOUTH FAYETTE TOWNSHIP ZONING HEARING BOARD TO SECTION 240-51(C) TO ALLOW EXCEEDANCE OF MAXIMUM IMPERVIOUS SURFACE AREA FROM THE REQUIRED TO PERCENT TO 84 PERCENT.

LAFAYETTE 180

NEWBURY DRIVE
CUDDY, PA 15031

PREPARED FOR:

CE - SF, LP
295 MYOMA ROAD
MARS, PA 16046

I CERTIFY THAT, TO THE BEST OF MY INFORMATION, KNOWLEDGE AND BELIEF THE SURVEY AND PLAN SHOWN HEREON ARE CORRECT AND ACCURATE TO THE STANDARDS REQUIRED.

DATE _____ NAME _____ REGISTRATION NO. _____

_____, A REGISTERED PROFESSIONAL ENGINEER FOR THE TOWNSHIP OF SOUTH FAYETTE DO HEREBY CERTIFY THAT THIS SUBDIVISION PLAN MEETS ALL THE ENGINEERING REQUIREMENTS OF THE TOWNSHIP SUBDIVISION AND ZONING ORDINANCE, EXCEPT AS DEPARTURES HAVE BEEN AUTHORIZED BY THE APPROVAL AUTHORITY.

DATE _____ NAME _____ REGISTRATION NO. _____

REVIEWED BY THE TOWNSHIP OF SOUTH FAYETTE PLANNING COMMISSION, THIS _____ DAY OF _____, 2023.

SECRETARY _____ (SEAL) CHAIRPERSON _____

THE BOARD OF COMMISSIONERS OF THE TOWNSHIP OF SOUTH FAYETTE HEREBY GIVES PUBLIC NOTICE IN APPROVING THIS PLAN FOR RECORDING PURPOSES ONLY, THE TOWNSHIP OF SOUTH FAYETTE ASSUMES NO OBLIGATIONS, LEGAL OR OTHERWISE, EXPRESSED OR IMPLIED EITHER TO ACCEPT SAID STREETS AS TOWNSHIP STREETS OR ROADS OR GRADES, PAVE AND CURB THE STREETS IN SAID PLAN OR TO CONSTRUCT SEWERS THEREIN OR TO INSTALL ANY OTHER SUCH SERVICE ORDINARILY INSTALLED IN TOWNSHIP STREETS OR ROADS.

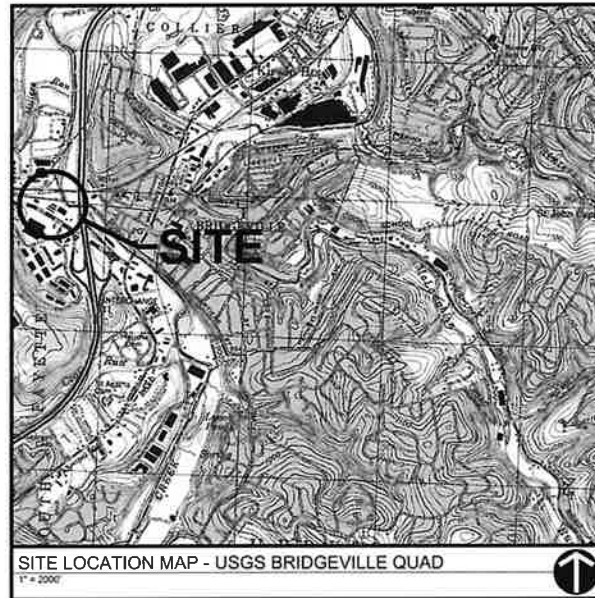
THE TOWNSHIP OF SOUTH FAYETTE AGREES NOT TO ISSUE BUILDING PERMITS UNTIL THE "PLANNING MODULE FOR LAND DEVELOPMENT" HAS BEEN APPROVED IN ACCORDANCE WITH THE REGULATIONS OF THE PENNSYLVANIA DEPARTMENT OF ENVIRONMENTAL PROTECTION.

APPROVED BY THE BOARD OF COMMISSIONERS OF THE TOWNSHIP OF SOUTH FAYETTE THIS _____ DAY OF _____, 2023.

ATTEST:

SECRETARY _____ (SEAL) PRESIDENT OF THE BOARD OF COMMISSIONERS _____

LIST OF UTILITIES	
PEOPLES GAS COMPANY LLC 338 E CUMMINGHAM STREET BUTLER, PA 16001 CONTACT: MICHAEL DENNY 724-431-1498 michael.denny@peoples-gas.com	
VERIZON PENNSYLVANIA LLC 1026 HAY STREET PITTSBURGH, PA 15221 CONTACT: DEBORAH BARUM 412-344-4390 deborah.d.delia@verizon.com	
COLUMBIA GAS OF PA INC 1600 DUBLIN ROAD COLUMBUS, OH 43215 CONTACT: LISA COLLINS 614-325-5586 ldugan@nsource.com	
SOUTH FAYETTE TWP MUNICIPAL AUTHORITY 900 PRESTO SYGAN ROAD BRIDGEVILLE, PA 15017 CONTACT: NICK GOETTMAN 412-257-7510 EXT. 5 ngoettman@sftwp.com	
FIRSTENERGY CORPORATION 21 S MAIN STREET AKRON, OH 44308 CONTACT: MELLYSSA ADAMS 330-604-4407 madams@firstenergycorp.com	
COMCAST 1241 BUSINESS RT 66 GREENSBURG, PA 15601 CONTACT: LLYOD CRAGO 878-295-5899 llyod_crago@cable.comcast.com	
SOUTH FAYETTE TOWNSHIP 515 MILLERS RUN ROAD MORGAN, PA 15064 CONTACT: PEGGY PATTERSON 412-221-187 EXT. 210 PPATTERSON@SFTWP.COM	
PENNSYLVANIA AMERICAN WATER 500 NOBLESTOWN ROAD CARNEGIE, PA 15106 CONTACT: LUCIAN CAPPETTA 412-401-1187 lucian.cappetta@amwater.com	



SITE LOCATION MAP - USGS BRIDGEVILLE QUAD
1" = 200'

ZONING REQUIREMENTS		
SITE ZONING: C-2 HIGHWAY COMMERCIAL		
USE: _____		
TOWNSHIP STANDARD	REQUIRED	PROVIDED
USE	PLANNED SHOPPING CENTER	PLANNED SHOPPING CENTER
BUILDING HEIGHT	NO MORE THAN 60 FT	
LOT AREA	5 Acres	4.4 Acres (Variance Granted)
SETBACKS		
FRONT YARD	50 Ft	58 Ft
REAR YARD	50 Ft	52 Ft
SIDE YARD	20 Ft	53 Ft
PARKING		
PLANNED SHOPPING CENTER TOTAL AREA: 54,420 S.F.		
TOTAL	1 SPACE PER 200 S.F. (272)	273
ADA ACCESSIBLE	7	7
LAND COVERAGES		
OPEN SPACE	20%	16%
IMPERVIOUS	80%	84% (Variance Requested)
PERMIT REQUIREMENTS		
REVIEWING AGENCY		
N.P.D.E.S PERMIT	ALLEGHENY COUNTY CONSERVATION DISTRICT	
PLANNING FACILITIES PLANNING MODULE	PA DEPT. OF ENVIRONMENTAL PROTECTION	
HIGHWAY OCCUPANCY PERMIT (HOP)	PA DEPT. OF TRANSPORTATION	
STORMWATER MANAGEMENT MAINTENANCE PROGRAM		
THE OWNERS SHALL BE RESPONSIBLE FOR INSPECTING THE STORMWATER DETENTION FACILITIES ON A SEMI-ANNUAL BASIS (JANUARY 2 AND JULY 2 OF EACH YEAR), PLUS AFTER EACH SIGNIFICANT RAINFALL. ANY DEBRIS WHICH MIGHT IMPEDE FLOW AT OR THROUGH THE OUTLET STRUCTURE SHALL BE REMOVED. ANY SEDIMENT WHICH ACCUMULATES WITHIN THE FACILITIES SHALL BE REMOVED.		

SHEET INDEX	
NO.	TITLE
C000	COVER SHEET
C050	EXISTING CONDITIONS AND DEMOLITION PLAN
C100	SITE PLAN
C101	FIRE TRUCK TEMPLATE
C200	GRADING PLAN
C300	LUTILITY PLAN
C400	EROSION AND SEDIMENTATION CONTROL PLAN
C401	EROSION AND SEDIMENTATION CONTROL DETAILS
C402	EROSION AND SEDIMENTATION CONTROL DETAILS
C403	EROSION AND SEDIMENTATION CONTROL DETAILS
C404	EROSION AND SEDIMENTATION CONTROL DETAILS
C500	STORM PROFILES
C501	SANITARY PROFILES
C600	CONSTRUCTION DETAILS
C601	CONSTRUCTION DETAILS
C602	CONSTRUCTION DETAILS
C603	CONSTRUCTION DETAILS
C700	POST CONSTRUCTION STORMWATER MANAGEMENT PLAN
C701	POST CONSTRUCTION STORMWATER MANAGEMENT NOTES
C702	POST CONSTRUCTION STORMWATER MANAGEMENT DETAILS
C703	POST CONSTRUCTION STORMWATER MANAGEMENT DETAILS
C704	POST CONSTRUCTION STORMWATER MANAGEMENT DETAILS
L100	LANDSCAPE PLAN
L101	LANDSCAPE PLAN DETAIL
L200	LIGHTING PLAN
L201	LIGHTING DETAIL

I, JOSEPH M. CALBRAITH, P.E. DO HEREBY CERTIFY PURSUANT TO THE PENALTIES OF 18 P.A.C.S.A. SEC. 4904 TO THE BEST OF MY KNOWLEDGE, INFORMATION AND BELIEF, THAT THE INFORMATION CONTAINED IN THE ACCOMPANYING PLANS, SPECIFICATIONS AND REPORTS HAS BEEN PREPARED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE, IS TRUE AND CORRECT, AND IS IN CONFORMANCE WITH CHAPTER 105 OF THE RULES AND REGULATIONS OF THE DEPARTMENT OF ENVIRONMENTAL PROTECTION.



Know what's below.
Call before you dig.
Serial No. 20231760058



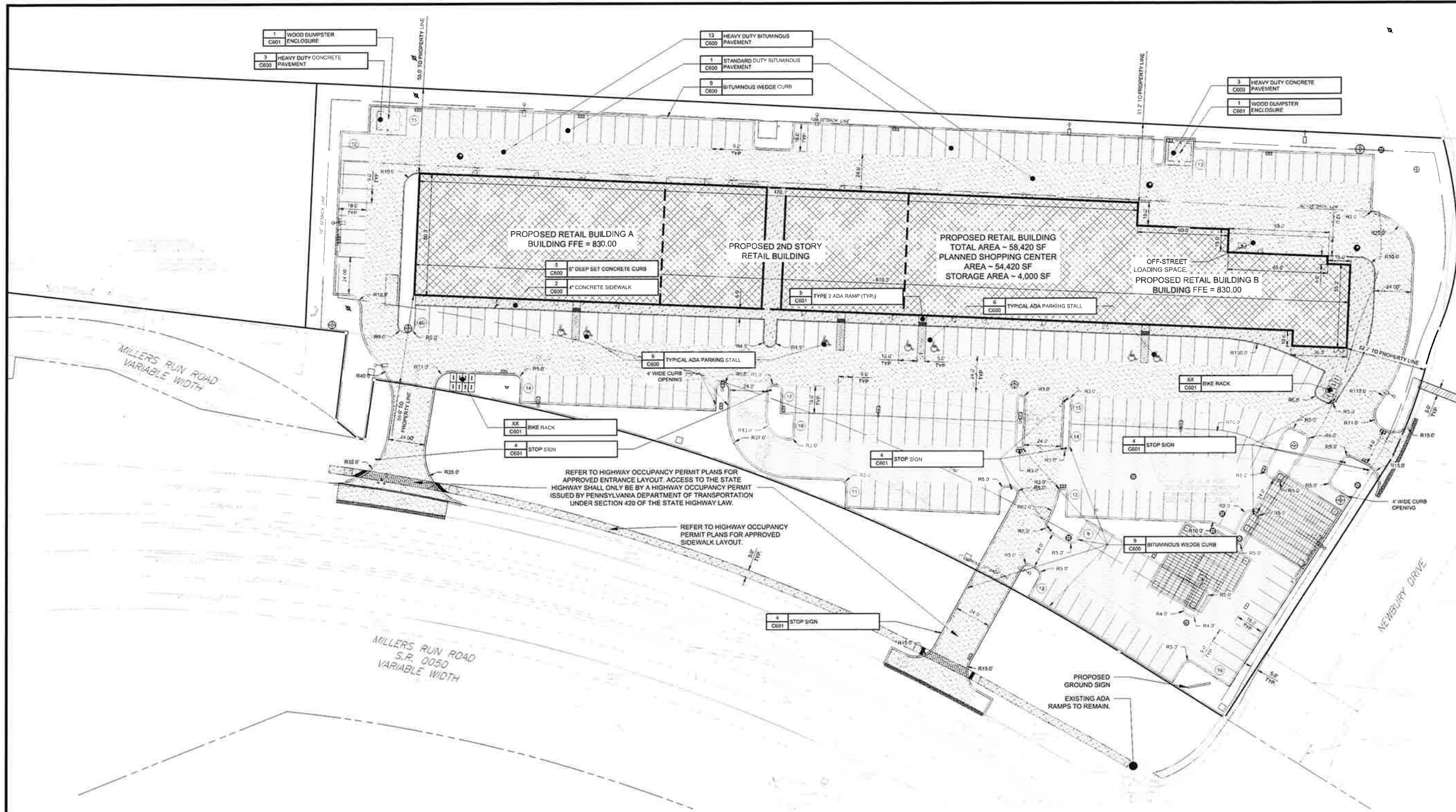
Date	No.	Revision
2025-11-11	01	WPP Updates
2025-12-16	02	ACCD TECHNICAL INFOS RESPONSE
2026-01-14	03	TOWNSHIP SUBMISSION
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARS, PA 16046

COVER SHEET

Project Number: C-12199-0029
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

C000



1 WOOD DUMPSTER ENCLOSURE
C601

3 HEAVY DUTY CONCRETE PAVEMENT
C600

13 HEAVY DUTY BITUMINOUS PAVEMENT
C600

1 STANDARD DUTY BITUMINOUS PAVEMENT
C600

9 BITUMINOUS WEDGE CURB
C600

3 HEAVY DUTY CONCRETE PAVEMENT
C600

1 WOOD DUMPSTER ENCLOSURE
C601

5 6" DEEP SET CONCRETE CURB
C600

2 4" CONCRETE SIDEWALK
C601

5 TYPE 2 ADA RAMP (TYP.)
C601

8 TYPICAL ADA PARKING STALL
C600

XX BIKE RACK
C601

4 STOP SIGN
C601

REFER TO HIGHWAY OCCUPANCY PERMIT PLANS FOR APPROVED ENTRANCE LAYOUT. ACCESS TO THE HIGHWAY SHALL ONLY BE BY A HIGHWAY OCCUPANCY PERMIT ISSUED BY PENNSYLVANIA DEPARTMENT OF TRANSPORTATION UNDER SECTION 420 OF THE STATE HIGHWAY LAW.

REFER TO HIGHWAY OCCUPANCY PERMIT PLANS FOR APPROVED SIDEWALK LAYOUT.

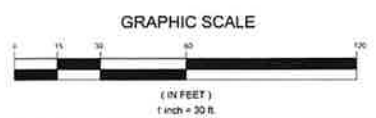
4 STOP SIGN
C601

PROPOSED GROUND SIGN
EXISTING ADA RAMP TO REMAIN.

SITE LEGEND

●	5	PROPOSED STORM MANHOLE	●	2	PROPOSED CONCRETE SIDEWALK
●	SD-010	PROPOSED 48" SANITARY MANHOLE	■	3	PROPOSED HEAVY DUTY CONCRETE PAVEMENT
+	2	PROPOSED CLEANOUT	■	1	PROPOSED STANDARD DUTY BITUMINOUS PAVEMENT
+	C764	PROPOSED CLEANOUT	■	13	PROPOSED HEAVY DUTY BITUMINOUS PAVEMENT
⊖	1	PROPOSED TYPE 'M' INLET	■	XX	PROPOSED PAVEMENT PATCH
⊖	C601	PROPOSED HYDRANT	⊖	C600	PROPOSED UTILITY POLE
⊖	XX	PROPOSED LIGHT STANDARD	⊖	10	PROPOSED ADA SYMBOL
⊖	C600	PROPOSED ADA SYMBOL	⊖	7	PROPOSED CONCRETE WHEEL STOP
⊖	C600	PROPOSED CONCRETE WHEEL STOP	⊖	2	PROPOSED BOLLARD
⊖	C601	PROPOSED BOLLARD	⊖	2	PROPOSED DEPRESSED CURB
⊖	C601	PROPOSED DEPRESSED CURB			

■	2	PROPOSED CONCRETE SIDEWALK
■	3	PROPOSED HEAVY DUTY CONCRETE PAVEMENT
■	1	PROPOSED STANDARD DUTY BITUMINOUS PAVEMENT
■	13	PROPOSED HEAVY DUTY BITUMINOUS PAVEMENT
■	XX	PROPOSED PAVEMENT PATCH
⊖	C600	PROPOSED UTILITY POLE
⊖	10	PROPOSED ADA SYMBOL
⊖	7	PROPOSED CONCRETE WHEEL STOP
⊖	2	PROPOSED BOLLARD
⊖	2	PROPOSED DEPRESSED CURB



REVISION RECORD

No.	Date	Description
01	2025-11-11	WPP Updates
02	2025-12-16	ACCD TECHNICAL NPDES RESPONSE
03	2025-01-14	TOWNSHIP SUBMISSION
04		
05		
06		
07		
08		

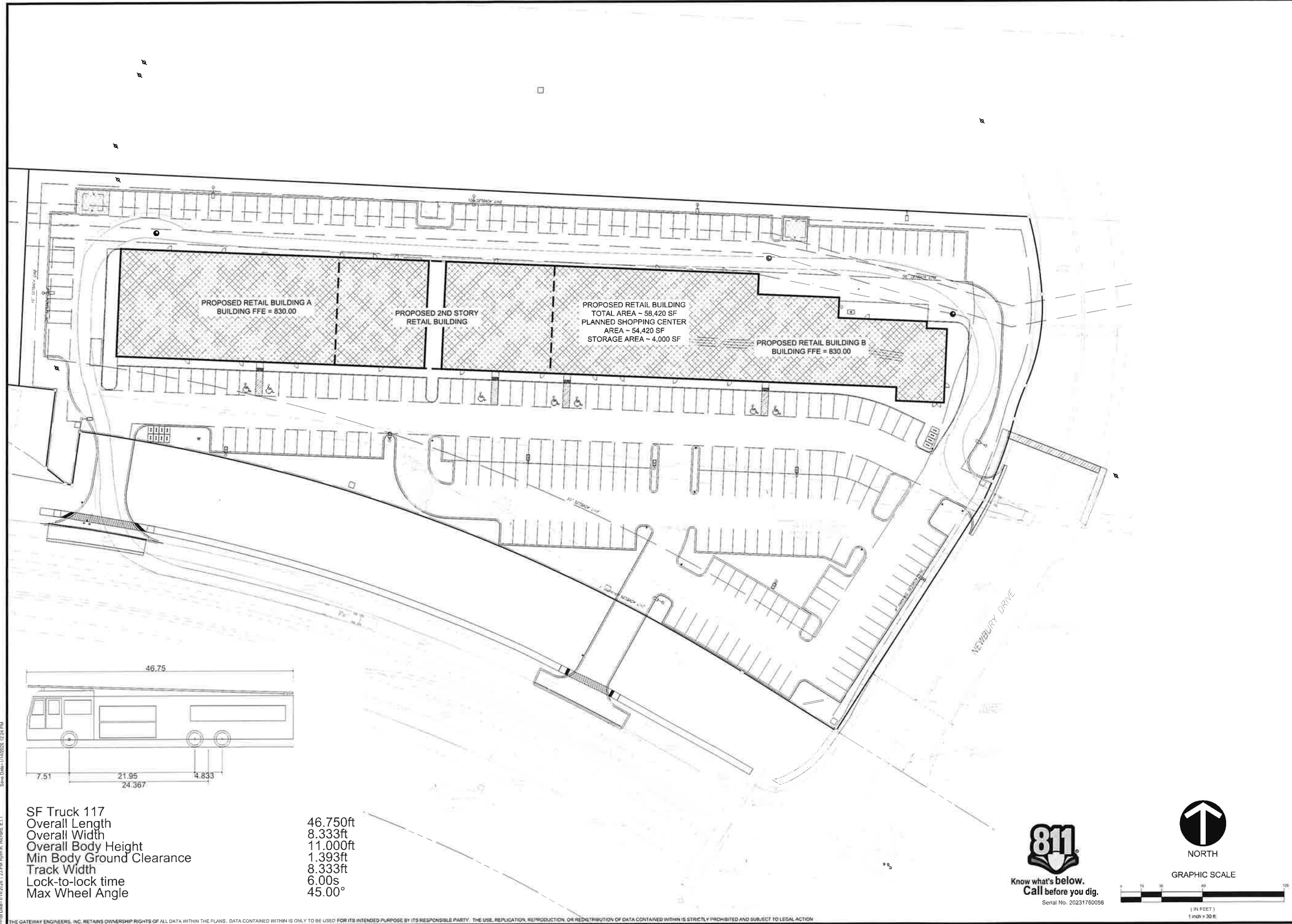
LAFAYETTE 180
NEWBURY DRIVE
CLUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARRS, PA 16046

SITE PLAN

Project Number: C-12199-0028
Drawing Scale: 1/32"
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

C100

Plot & File Name: C:\Projects\12080\12199\KOB\12199-0028\Drawings\SitePlan.dwg
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Plotter: HP DesignJet T120
Scale: 1/32"
Author: JMG
Date: 11/14/2025 1:27 PM

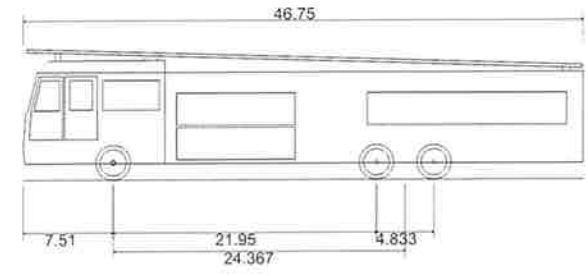


PROPOSED RETAIL BUILDING A
BUILDING FFE = 830.00

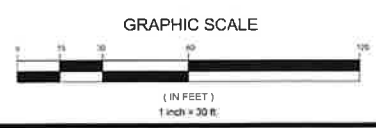
PROPOSED 2ND STORY
RETAIL BUILDING

PROPOSED RETAIL BUILDING
TOTAL AREA ~ 58,420 SF
PLANNED SHOPPING CENTER
AREA ~ 54,420 SF
STORAGE AREA ~ 4,000 SF

PROPOSED RETAIL BUILDING B
BUILDING FFE = 830.00



SF Truck 117
Overall Length 46.750ft
Overall Width 8.333ft
Overall Body Height 11.000ft
Min Body Ground Clearance 1.393ft
Track Width 8.333ft
Lock-to-lock time 6.00s
Max Wheel Angle 45.00°



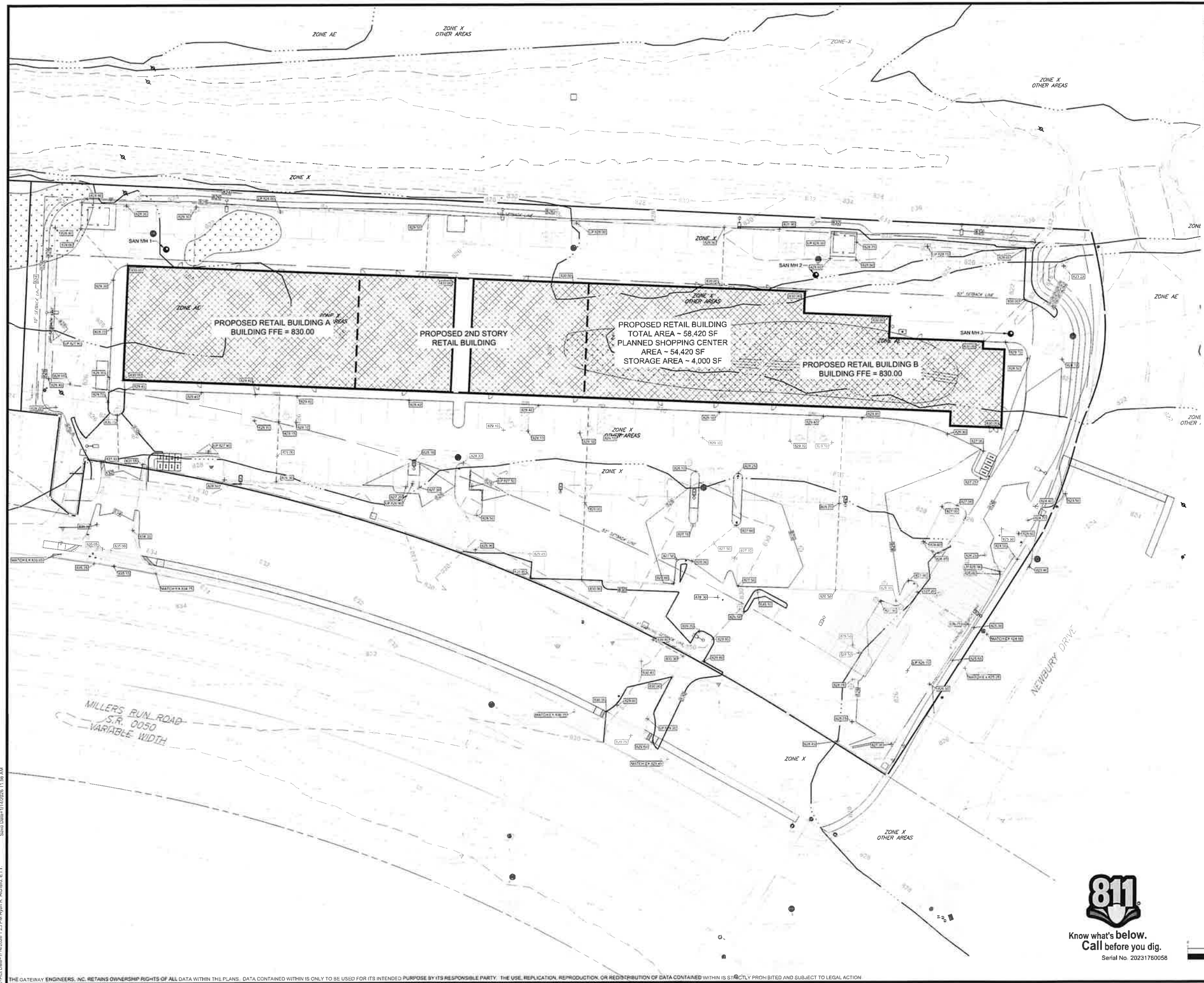
REVISION RECORD

Date	No.	Description
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2025-12-16	02	ACCO TECHNICAL RESPONSE
2026-01-14	03	TOWNSHIP SUBMISSION
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARKS, PA 16046

FIRE TRUCK
TEMPLATE
Project Number: C-12199-0025
Drawing Scale: 1"=30'
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG
C101

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GRADING LEGEND

	PROPOSED SPOT GRADE
	FEMA FLOODPLAIN LINE

SPOT GRADE ABBREVIATIONS

TC	TOP CURB
BC	BOTTOM CURB
TC/BC	TOP CURB/ BOTTOM CURB
TW	TOP WALL
BW	BOTTOM WALL
HP	HIGH POINT
LP	LOW POINT

- GRADING PLAN NOTES:**
- 1 ALL SITE WORK SHALL BE DONE IN ACCORDANCE WITH THE PLANS PREPARED BY THE GATEWAY ENGINEERS, INC. THE CURRENT REQUIREMENTS OF THE MUNICIPALITY AND ALL OTHER PERTINENT FEDERAL AND STATE LAWS.
 - 2 CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT PREPARED BY HSH DATED MAY 3, 2023 PRIOR TO INITIATION OF ANY EARTHWORK ACTIVITY.
 - 3 THE CONTRACTOR SHALL COMPLY AT ALL TIMES WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, PROVISIONS AND POLICIES GOVERNING SAFETY AND HEALTH, INCLUDING THE FEDERAL CONSTRUCTION SAFETY ACT (PUBLIC LAW 91-54) FEDERAL REGISTER, CHAPTER XVII, PART 1926 OF TITLE 29 REGULATIONS, OCCUPATIONAL SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION, AND SUBSEQUENT PUBLICATIONS UPDATING THESE REGULATIONS.
 - 4 THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXAMINING THE AREAS AND CONDITIONS UNDER WHICH THE PROJECT IS TO BE CONSTRUCTED PRIOR TO THE SUBMISSION OF A BID. SUBMISSION OF A BID SHALL BE CONSTRUED TO MEAN THE CONTRACTOR HAS REVIEWED THE SITE AND IS FAMILIAR WITH CONDITIONS AND CONSTRAINTS OF THE SITE.
 - 5 BEFORE EXCAVATIONS, ALL UNDERGROUND UTILITIES SHALL BE LOCATED IN THE FIELD BY THE PROPER AUTHORITIES. THE CONTRACTOR SHALL NOTIFY PENNSYLVANIA ONE CALL SYSTEMS, INC. AT 811. THE LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES ARE APPROXIMATE AND MAY NOT ALL BE SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE EXISTENCE AND EXACT LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES.
 - 6 ALL SLOPES SHALL BE 2:1 (HORIZONTAL: VERTICAL) MAXIMUM UNLESS NOTED OTHERWISE.
 - 7 ALL AREAS NOT PAVED SHALL BE TOP SOLED, SEEDED, MULCHED OR LANDSCAPED UNLESS OTHERWISE NOTED IN THE CONSTRUCTION DRAWINGS, SITE SPECIFICATIONS OR INSTRUCTED BY THE OWNER.
 - 8 SPOT ELEVATIONS ARE LOCATED AT THE TOP OF PAVEMENT.
 - 9 THE CONTRACTOR SHALL REMOVE ANY EXCESS TOPSOIL OFFSITE TO A PA DEP APPROVED WASTE SITE, OR SPREAD UNIFORMLY ON SITE. THE ACQUISITION OF AN APPROVED WASTE SITE IS THE CONTRACTOR'S RESPONSIBILITY.
 - 10 THE CONTRACTOR SHALL PROTECT ALL CORNER PINS, MONUMENTS, PROPERTY CORNERS, AND BENCHMARKS DURING DEMOLITION ACTIVITIES. IF DISTURBED CONTRACTOR SHALL HAVE DISTURBED ITEMS RESET BY A LICENSED SURVEYOR AT NO ADDITIONAL COST TO THE OWNER.
 - 11 AN AS-BUILT DRAWING OF THE STORMWATER DETENTION FACILITY PREPARED AND SEALED BY A PROFESSIONAL LAND SURVEYOR IS REQUIRED TO BE SUBMITTED TO THE PROJECT ENGINEER.



REVISION RECORD

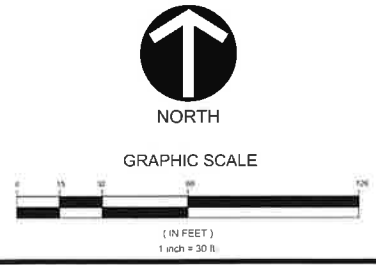
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2025-12-16	02	ACCD TECHNICAL AIDES RESPONSE
2026-01-14	03	TOWNSHIP SUBMISSION
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
 PREPARED FOR:
CE - SF, LP
 295 MYOMA ROAD
 MARS, PA 16046

GRADING PLAN

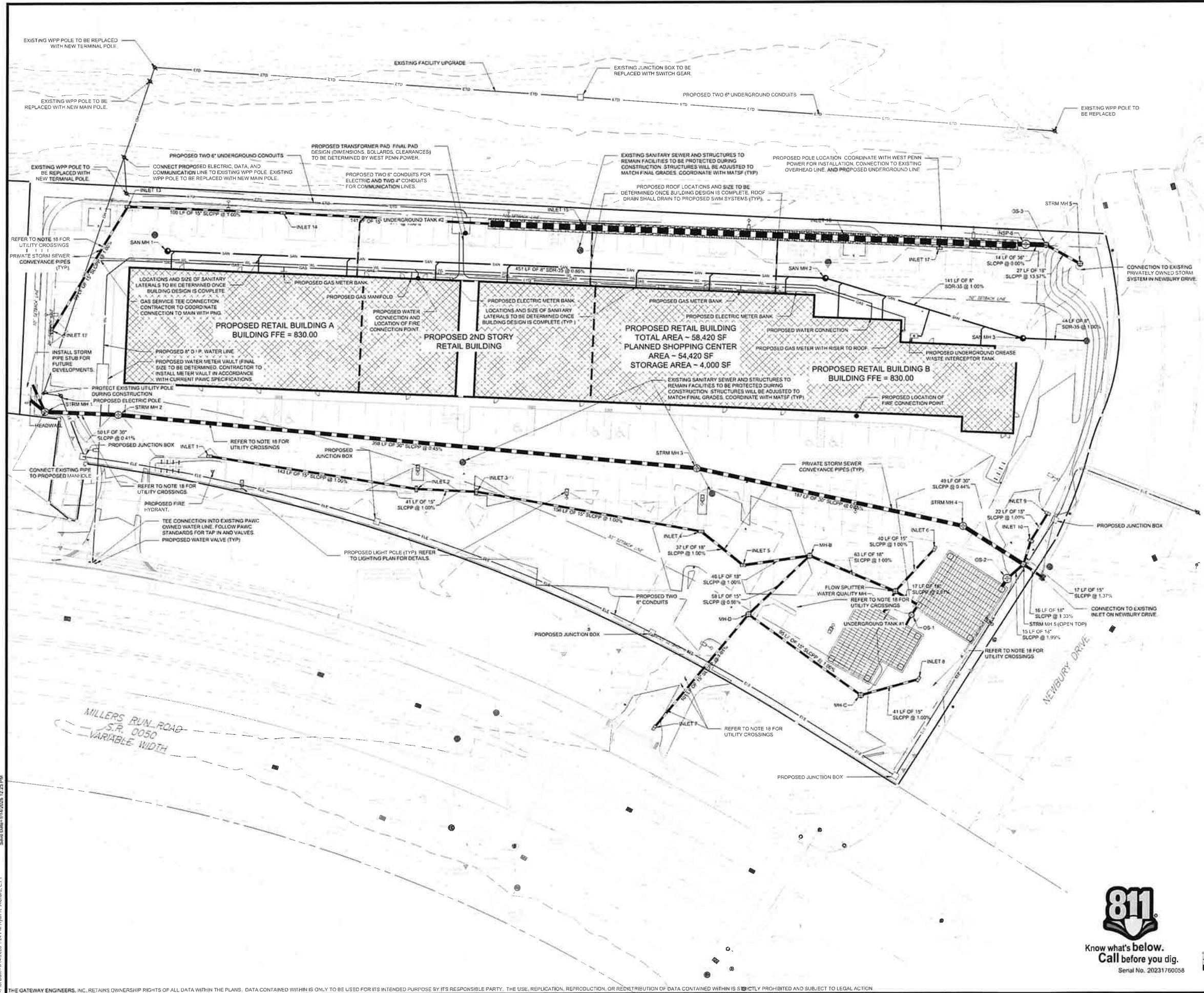
Project Number: C-12199-0025
 Drawing Scale: 1" = 30'
 Date Issued: AUG 2025
 Index Number: _____
 Drawn By: MCL/RRR
 Checked By: JMG
 Project Manager: JMG

C200



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 Save Date: 11/16/2025 11:56 AM

THE GATEWAY ENGINEERS, INC. RETAINS OWNERSHIP RIGHTS OF ALL DATA WITHIN THE PLANS. DATA CONTAINED WITHIN IS ONLY TO BE USED FOR ITS INTENDED PURPOSE BY ITS RESPONSIBLE PARTY. THE USE, REPLICATION, REPRODUCTION, OR REDISTRIBUTION OF DATA CONTAINED WITHIN IS STRICTLY PROHIBITED AND SUBJECT TO LEGAL ACTION.



UTILITY LEGEND

— SAN —	PROPOSED SANITARY LINE
— SAN — SAN —	PROPOSED SANITARY LATERAL
— S — S —	PROPOSED STORM LINE
— WL —	PROPOSED WATER LINE
— WL — WL —	PROPOSED WATER LATERAL
— DAT —	PROPOSED DATA LINE
— EL —	PROPOSED ELECTRIC LINE
— ETD —	PROPOSED ELECTRIC, TELEPHONE, & DATA LINE
— TEL —	PROPOSED TELEPHONE LINE
— OHE —	PROPOSED OVERHEAD ELECTRIC
— OH —	PROPOSED OVERHEAD LINE
— GAS —	PROPOSED GAS LINE
— U —	PROPOSED UTILITY POLE
— FH —	PROPOSED FIRE HYDRANT
— GV —	PROPOSED GAS VALVE
— WV —	PROPOSED WATER VALVE
— T —	PROPOSED TEE

UTILITY ABBREVIATIONS

STM	STORM
SAN	SANITARY
MH	MANHOLE
OS	OUTLET STRUCTURE
CD	CLEANOUT
RD	RDDF DRAIN
HW	HEADWALL
EW	ENDWALL

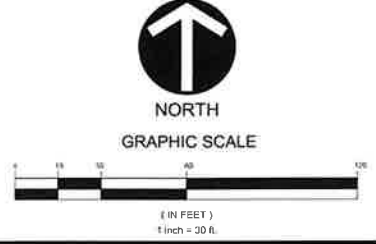
- ### UTILITY PLAN NOTES:
- THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE AND LOCAL LAWS, PROVISIONS AND POLICIES GOVERNING SAFETY AND HEALTH, INCLUDING THE FEDERAL CONSTRUCTION SAFETY ACT (PUBLIC LAW 91-594), FEDERAL REGISTER, CHAPTER 191, PART 191.16 REGULATIONS, OCCUPATIONAL SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION, AND SUBSEQUENT PUBLICATIONS UPDATING THESE REGULATIONS.
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXAMINING THE AREAS AND CONDITIONS UNDER WHICH THE PROJECT IS TO BE CONSTRUCTED PRIOR TO THE SUBMISSION OF A BID. SUBMISSION OF A BID SHALL BE CONSIDERED TO MEAN THE CONTRACTOR HAS REVIEWED THE SITE AND IS FAMILIAR WITH CONDITIONS AND CONSTRAINTS OF THE SITE.
 - BEFORE EXCAVATION ALL UNDERGROUND UTILITIES SHALL BE LOCATED IN THE FIELD BY THE PROPER AUTHORITIES. THE CONTRACTOR SHALL CONTACT PENNSYLVANIA ONE CALL SYSTEMS INC. AT 8-1-1. THE LOCATION OF UTILITIES AND UNDERGROUND STRUCTURES ARE APPROXIMATE AND MAY NOT ALL BE SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE EXISTENCE AND EXACT LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES.
 - IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO BID AND PERFORM ALL UTILITY WORK IN ACCORDANCE WITH APPLICABLE LOCAL AND STATE CODES AND REGULATIONS.
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR FEES ASSOCIATED WITH THE INSTALLATION, INSPECTING, TESTING AND FINAL ACCEPTANCE OF PROPOSED UTILITIES CONSTRUCTION.
 - UTILITIES SHALL BE INSTALLED IN ACCORDANCE WITH THE SPECIFICATIONS OF THE RESPECTIVE UTILITY COMPANY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENGINE UTILITIES ARE INSTALLED CORRECTLY TO MEET PROJECT REQUIREMENTS WHETHER PERFORMED BY THE CONTRACTOR OR NOT.
 - ALL CABLE, ELECTRIC, GAS, STORM AND SANITARY SEWER TELEPHONE AND WATER SERVICE LINE, FRENCHES LOCATED UNDER PROPOSED PAVEMENT AREAS SHALL BE BACKFILLED WITH 100% STONE MATERIAL TO THE PROPOSED PAVING SECTION IN ACCORDANCE WITH THE DETAIL SHOWN ON THE CONSTRUCTION DETAIL SHEETS.
 - THERE SHALL BE NO CONSTRUCTION OF ELECTRIC TRANSFORMERS, TELEPHONE OR CABLE JUNCTION BOXES, GAS METERS OR BARLAK DEVICES PLACED OVER ANY OTHER UNDERGROUND UTILITY.
 - CONTRACTOR TO PROVIDE SHOP DRAWINGS ON ALL STORM SEWER MANHOLES AND INLETS.
 - AN AS-BUILT DRAWING OF NEW UTILITY SERVICES SHALL BE PREPARED BY THE CONTRACTOR AND SUBMITTED TO THE OWNER UPON COMPLETION OF THE PROJECT.
 - ALL STORM PIPE SHALL BE HDPE, SMOOTH INTERIOR, CORRUGATED POLYETHYLENE PIPE UNLESS OTHERWISE NOTED. ALL STORM SEWER CONSTRUCTION MATERIALS AND METHODS SHALL BE IN ACCORDANCE WITH PENNDOT SPECIFICATIONS. ALL JOINTS SHALL BE WATER TIGHT.
 - THE CONTRACTOR SHALL ASSURE THAT THERE IS POSITIVE DRAINAGE TO THE EXISTING INLETS UPON PLACEMENT OF NEW PAVEMENT.
 - CONTRACTOR TO COORDINATE WITH THE BUILDING PLUMBING AND SITE ELECTRICAL PLANS TO ASSURE ACCURACY OF THE UTILITY CONNECTIONS TO THE BUILDING.
 - CONDUIT LOCATIONS TO SITE LIGHT POLES TO BE COORDINATED WITH THE SITE ELECTRICAL PLAN.
 - THE ALLEGHENY COUNTY HEALTH DEPARTMENT SHALL BE CONTACTED FOR INSPECTION OF ALL PRIVATE SANITARY SEWERS, WATER LINES, AND STORM SEWER LINES WHERE THEY COINCIDE TO A PUBLIC SEWER SYSTEM. ALL WATER AND SEWER LINES MUST BE INSTALLED BY A REGISTERED PLUMBER.
 - THE ROOF COLLECTOR SYSTEM SHALL BE TRAPPED PRIOR TO CONNECTING TO THE SITE STORM SEWER SYSTEM.
 - THE WATERLINE SHALL HAVE A MINIMUM OF 48" OF COVER AND BE AT LEAST 1 FOOT ABOVE THE SANITARY SEWER IF WITHIN 10 FEET OF THE HORIZONTAL DISTANCE OF THE SEWER.
 - CONTRACTOR MUST COORDINATE ALL UTILITY CONSTRUCTION WITH THE APPROPRIATE UTILITY COMPANIES. POTENTIAL EXISTING UTILITIES AT ALL PROPOSED CROSSINGS TO DETERMINE PROPER CLEARANCES. NOTIFY ENGINEER WITH ANY CONFLICTS.

GATEWAY ENGINEERS

A FULL-SERVICE CIVIL ENGINEERING FIRM

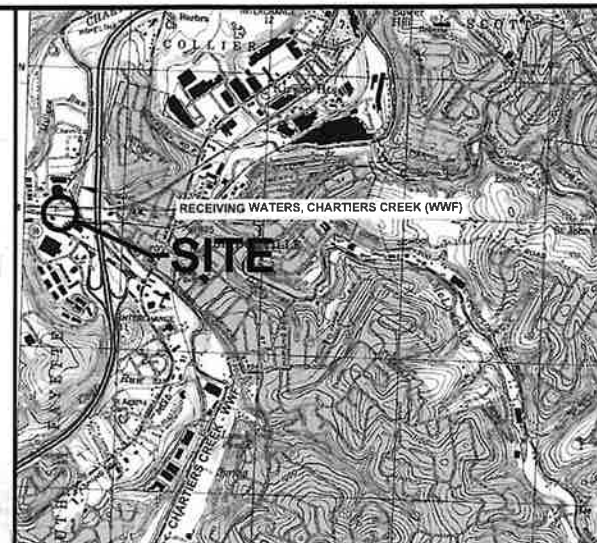
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Date	Description
2025-11-11	01 WPP Updates
2025-12-16	02 ACCO TECHNICAL RESPONSE
2026-01-14	03 TOWNSHIP SUBMISSION
	04
	05
	06
	07
	08

LAFAYETTE 180	UTILITY PLAN
NEWBURY DRIVE	Project Number: C-12199-0025
CUDDY, PA 15031	Drawing Scale: 1" = 30'
PREPARED FOR: CE - SF, LP	Date Issued: JAN 2025
295 WYOMA ROAD	Index Number: —
MARS, PA 16046	Drawn By: MCL/RRR
	Checked By: JMG
	Project Manager: JMG
	C300

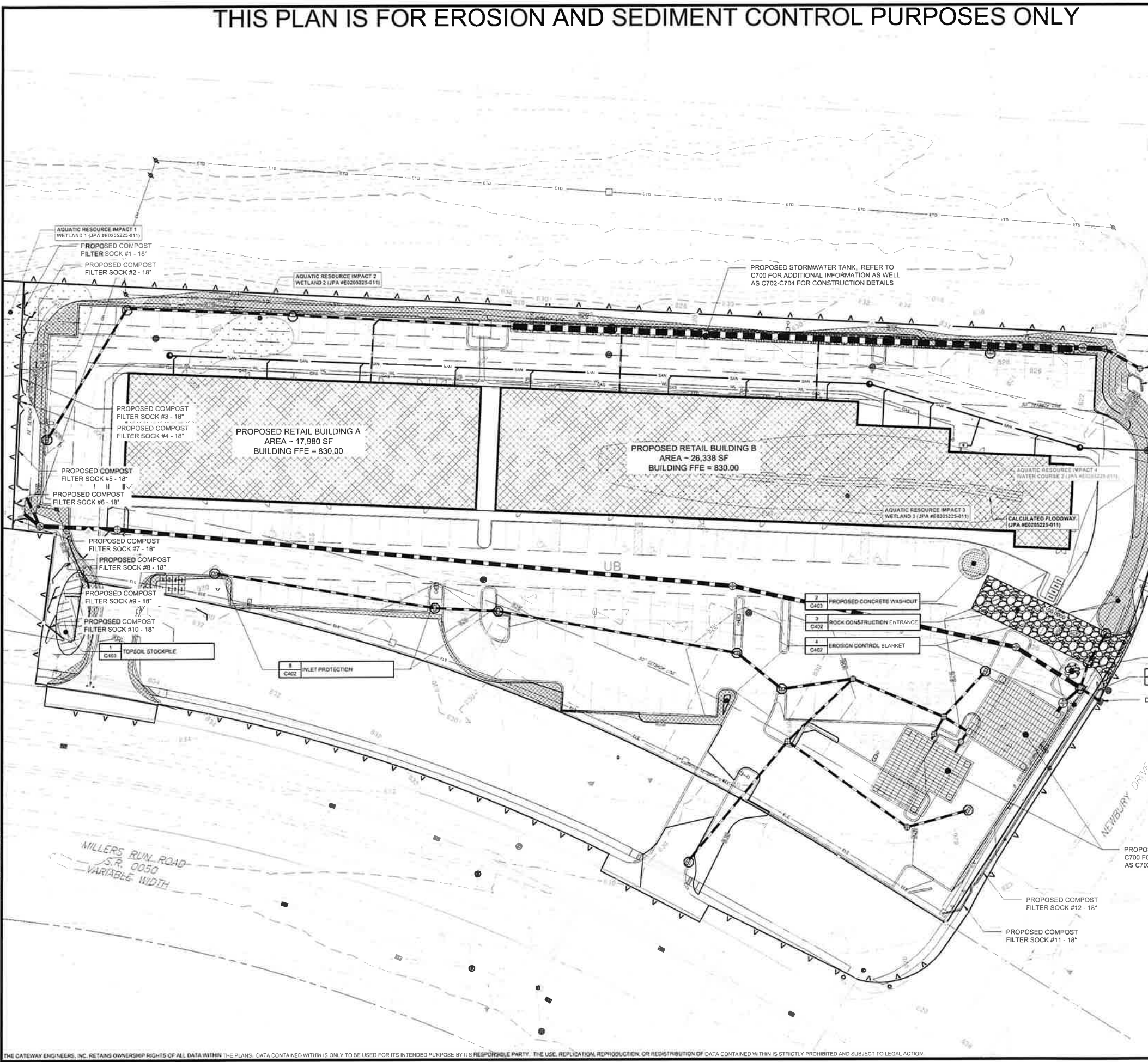


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 User: Ryan R. Richard, E.I.T.
 Scale: 1/4" = 30'

THIS PLAN IS FOR EROSION AND SEDIMENT CONTROL PURPOSES ONLY



SITE LOCATION MAP - USGS BRIDGEVILLE QUAD
SCALE 1" = 200'



EROSION AND SEDIMENT CONTROL LEGEND

4 C402	EROSION CONTROL BLANKET
3 C402	ROCK CONSTRUCTION ENTRANCE
8 C402	INLET PROTECTION
SOLID LINE	SOIL BOUNDARIES
DASHED LINE	LIMITS OF DISTURBANCE/PERMIT BOUNDARY
1 C704	PENNDOT STANDARD INLET BOX
2 C704	CLEANOUT
4 C704	PROPOSED 48" STORM MANHOLE
THICK DASHED LINE	PROPOSED STORM LINE
SHADING	AQUATIC RESOURCE WETLANDS
---	AQUATIC RESOURCE WATER COURSE
---	AQUATIC RESOURCE FLOODWAY

- ### EROSION AND SEDIMENT CONTROL NOTES:
- THE CONTRACTOR SHALL COMPLY AT ALL TIMES WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, PROVISIONS AND POLICIES GOVERNING SAFETY AND HEALTH, INCLUDING THE FEDERAL CONSTRUCTION SAFETY ACT (PUBLIC LAW 91-59) FEDERAL REGISTER, CHAPTER XXV, PART 1926 OF TITLE 29 REGULATIONS, OCCUPATIONAL SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION, AND SUBSEQUENT PUBLICATIONS UPDATING THESE REGULATIONS.
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXAMINING THE AREAS AND CONDITIONS UNDER WHICH THE PROJECT IS TO BE CONSTRUCTED PRIOR TO THE SUBMISSION OF THE BID. SUBMISSION OF A BID SHALL BE CONSIDERED TO MEAN THE CONTRACTOR HAS REVIEWED THE SITE AND IS FAMILIAR WITH CONDITIONS AND CONSTRAINTS OF THE SITE.
 - BEFORE EXCAVATION ALL UNDERGROUND UTILITIES SHALL BE LOCATED IN THE FIELD BY THE PROPER AUTHORITIES. THE CONTRACTOR SHALL NOTIFY PENNSYLVANIA ONE CALL SYSTEMS INC AT 1-800-343-1776. THE LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES ARE APPROXIMATE AND MAY NOT ALL BE SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE EXISTENCE AND EXACT LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES.
 - CONTRACTOR TO PROVIDE SHOP DRAWINGS ON ALL STORM SEWER MANHOLES, INLETS, AND DETENTION SYSTEMS.
 - AN AS-BUILT DRAWING OF NEW UTILITY SERVICES SHALL BE PREPARED BY THE CONTRACTOR AND SUBMITTED TO THE OWNER UPON COMPLETION OF THE PROJECT.
 - ALL STORM PIPE SHALL BE HOPE, SMOOTH INTERIOR, CORRUGATED POLYETHYLENE PIPE UNLESS OTHERWISE NOTED. ALL STORM SEWER CONSTRUCTION MATERIALS AND METHODS SHALL BE IN ACCORDANCE WITH PENNDOT SPECIFICATIONS. ALL JOINTS SHALL BE WATER-TIGHT.
 - CONTRACTOR SHALL REFER TO OTHER PLANS WITHIN THIS CONSTRUCTION SET FOR OTHER PERTINENT INFORMATION.
 - THE CONTRACTOR SHALL ASSURE THAT THERE IS POSITIVE DRAINAGE TO THE INLETS UPON PLACEMENT OF NEW PAVEMENT.
 - CONTRACTOR IS REQUIRED TO NOTIFY AN ENVIRONMENTAL PROFESSIONAL (EP) PRIOR TO ANY EXCAVATION AND HAVE THEM ON-SITE TO OVERSEE ALL EXCAVATION AND HANDLING OF MATERIAL. THE CONTRACTOR SHALL REFER TO THE SOIL MANAGEMENT PLAN / MATERIAL MANAGEMENT PLAN PREPARED BY CHIBINS & ASSOCIATES, LLC AND THE ON-SITE EP FOR THE REQUIRED SOIL HANDLING, STOCKPILE, DISPOSAL, ETC PROCEDURES.

PROJECT AREAS

TOTAL PERMIT AREA = 5.48 ACRES
DISTURBED AREA = 5.48 ACRES

NORTH GRAPHIC SCALE

(IN FEET)
1 inch = 30 ft.

GATEWAY ENGINEERS
A FULL-SERVICE CIVIL ENGINEERING FIRM

REVISION RECORD

Date	No.	W/P Updates
2025-11-11	01	W/P Updates
2025-12-18	02	ACCD TECHNICAL RESPONSE
	03	
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARS, PA 16648

EROSION AND SEDIMENTATION CONTROL PLAN
Project Number: C-12198-0025
Drawing Scale: 1" = 30'
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/HRR
Checked By: JMG
Project Manager: JMG

C400

STANDARD EROSION AND SEDIMENT CONTROL PLAN NOTES

- ALL EARTH DISTURBANCES, INCLUDING CLEARING AND GRUBBING AS WELL AS CUTS AND FILLS SHALL BE DONE IN ACCORDANCE WITH THE APPROVED E&S PLAN. A COPY OF THE APPROVED DRAWINGS (STAMPED, SIGNED AND DATED BY THE REVIEWING AGENCY) MUST BE AVAILABLE AT THE PROJECT SITE AT ALL TIMES. THE REVIEWING AGENCY SHALL BE NOTIFIED OF ANY CHANGES TO THE APPROVED PLAN PRIOR TO IMPLEMENTATION OF THOSE CHANGES. THE REVIEWING AGENCY MAY REQUIRE A WRITTEN SUBMITTAL OF THOSE CHANGES FOR REVIEW AND APPROVAL AT ITS DISCRETION.
- AT LEAST 7 DAYS PRIOR TO STARTING ANY EARTH DISTURBANCE ACTIVITIES, INCLUDING CLEARING AND GRUBBING, THE OWNER AND/OR OPERATOR SHALL INVITE ALL CONTRACTORS, THE LANDOWNER, APPROPRIATE MUNICIPAL OFFICIALS, THE E&S PLAN PREPARER, THE PCSM PLAN PREPARER, THE LICENSED PROFESSIONAL RESPONSIBLE FOR OVERSIGHT OF CRITICAL STAGES OF IMPLEMENTATION OF THE PCSM PLAN, AND A REPRESENTATIVE FROM THE LOCAL CONSERVATION DISTRICT TO AN ON-SITE PRE-CONSTRUCTION MEETING.
- AT LEAST 3 DAYS PRIOR TO STARTING ANY EARTH DISTURBANCE ACTIVITIES, OR EXPANDING INTO AN AREA PREVIOUSLY UNMARKED, THE PENNSYLVANIA ONE CALL SYSTEM INC. SHALL BE NOTIFIED AT 1-800-242-1776 FOR THE LOCATION OF EXISTING UNDERGROUND UTILITIES.
- ALL EARTH DISTURBANCE ACTIVITIES SHALL PROCEED IN ACCORDANCE WITH THE SEQUENCE PROVIDED ON THE PLAN DRAWINGS. DEVIATION FROM THAT SEQUENCE MUST BE APPROVED IN WRITING FROM THE LOCAL CONSERVATION DISTRICT OR BY THE DEPARTMENT PRIOR TO IMPLEMENTATION.
- AREAS TO BE FILLED ARE TO BE CLEARED, GRUBBED, AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS AND OTHER OBJECTIONABLE MATERIAL.
- CLEARING, GRUBBING, AND TOPSOIL STRIPPING SHALL BE LIMITED TO THOSE AREAS DESCRIBED IN EACH STAGE OF THE CONSTRUCTION SEQUENCE. GENERAL SITE CLEARING, GRUBBING AND TOPSOIL STRIPPING MAY NOT COMMENCE IN ANY STAGE OR PHASE OF THE PROJECT UNTIL THE E&S BMPs SPECIFIED BY THE BMP SEQUENCE FOR THAT STAGE OR PHASE HAVE BEEN INSTALLED AND ARE FUNCTIONING AS DESCRIBED IN THIS E&S PLAN.
- AT NO TIME SHALL CONSTRUCTION VEHICLES BE ALLOWED TO ENTER AREAS OUTSIDE THE LIMIT OF DISTURBANCE BOUNDARIES SHOWN ON THE PLAN MAPS. THESE AREAS MUST BE CLEARLY MARKED AND FENCED OFF BEFORE CLEARING AND GRUBBING OPERATIONS BEGIN.
- TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED AT THE LOCATIONS(S) SHOWN ON THE PLAN MAP(S) IN THE AMOUNT NECESSARY TO COMPLETE THE FINISH GRADING OF ALL EXPOSED AREAS THAT ARE TO BE STABILIZED BY VEGETATION. EACH STOCKPILE SHALL BE PROTECTED IN THE MANNER SHOWN ON THE PLAN DRAWINGS. STOCKPILE HEIGHTS SHALL NOT EXCEED 35 FEET. STOCKPILE SLOPES SHALL BE 2H:1V OR FLATTER.
- IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO MINIMIZE THE POTENTIAL FOR EROSION AND SEDIMENT POLLUTION AND NOTIFY THE LOCAL CONSERVATION DISTRICT AND/OR THE REGIONAL OFFICE OF THE DEPARTMENT.
- ALL BUILDING MATERIALS AND WASTES SHALL BE REMOVED FROM THE SITE AND RECYCLED OR DISPOSED OF IN ACCORDANCE WITH THE DEPARTMENT'S SOLID WASTE MANAGEMENT REGULATIONS AT 25 PA. CODE 260.1 ET SEQ., 271.1, AND 287.1 ET SEQ. NO BUILDING MATERIALS OR WASTES OR UNUSED BUILDING MATERIALS SHALL BE BURNED, BURIED, DUMPED, OR DISCHARGED AT THE SITE.
- ALL OFF-SITE WASTE AND BORROW AREAS MUST HAVE AN E&S PLAN APPROVED BY THE LOCAL CONSERVATION DISTRICT OR THE DEPARTMENT FULLY IMPLEMENTED PRIOR TO BEING ACTIVATED.
- THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ANY MATERIAL BROUGHT ON SITE IS CLEAN FILL. FORM FP-001 MUST BE RETAINED BY THE PROPERTY OWNER FOR ANY FILL MATERIAL AFFECTED BY A SPILL OR RELEASE OF A REGULATED SUBSTANCE BUT QUALIFYING AS CLEAN FILL DUE TO ANALYTICAL TESTING.
- ALL PUMPING OF WATER FROM ANY WORK AREA SHALL BE DONE ACCORDING TO THE PROCEDURE DESCRIBED IN THIS PLAN, OVER UNDISTURBED VEGETATED AREAS.
- VEHICLES AND EQUIPMENT MAY NEITHER ENTER DIRECTLY NOR EXIT DIRECTLY FROM THE PROJECT SITE ONTO PUBLIC ROADS.
- UNTIL THE SITE IS STABILIZED, ALL EROSION AND SEDIMENT BMPs SHALL BE MAINTAINED PROPERLY. MAINTENANCE SHALL INCLUDE INSPECTIONS OF ALL EROSION AND SEDIMENT BMPs AFTER EACH RUNOFF EVENT AND ON A WEEKLY BASIS. ALL PREVENTATIVE AND REMEDIAL MAINTENANCE WORK, INCLUDING CLEAN OUT, REPAIR, REPLACEMENT, RE-GRADING, RE-SEEDING, RE-MULCHING AND RE-NETTING MUST BE PERFORMED IMMEDIATELY. IF THE E&S BMPs FAIL TO PERFORM AS EXPECTED, REPLACEMENT BMPs, OR MODIFICATIONS OF THOSE INSTALLED MUST BE REQUIRED.
- A LOG SHOWING DATES THAT E&S BMPs WERE INSPECTED AS WELL AS ANY DEFICIENCIES FOUND AND THE DATE THEY WERE CORRECTED SHALL BE MAINTAINED ON THE SITE AND BE MADE AVAILABLE TO REGULATORY AGENCY OFFICIALS AT THE TIME OF INSPECTION.
- SEDIMENT TRACKED ONTO ANY PUBLIC ROADWAY OR SIDEWALK SHALL BE RETURNED TO THE CONSTRUCTION SITE BY THE END OF EACH WORK DAY AND DISPOSED IN THE MANNER DESCRIBED IN THIS PLAN. IN NO CASE SHALL THE SEDIMENT BE WASHED, SHOVELLED, OR SWEEPED INTO ANY ROADSIDE DITCH, STORM SEWER, OR SURFACE WATER.
- ALL SEDIMENT REMOVED FROM BMPs SHALL BE DISPOSED OF IN THE MANNER DESCRIBED ON THE PLAN DRAWINGS.
- AREAS WHICH ARE TO BE TOPSOILED SHALL BE SCARIFIED TO A MINIMUM DEPTH OF 3 TO 5 INCHES - 6 TO 12 INCHES ON COMPACTED SOILS - PRIOR TO PLACEMENT OF TOPSOIL. AREAS TO BE VEGETATED SHALL HAVE A MINIMUM 4 INCHES OF TOPSOIL IN PLACE PRIOR TO SEEDING AND MULCHING. FILL SLOPES SHALL HAVE A MINIMUM OF 2 INCHES OF TOPSOIL.
- ALL FILLS SHALL BE COMPACTED AS REQUIRED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED PROBLEMS. FILL INTENDED TO SUPPORT BUILDINGS, STRUCTURES AND CONDUITS, ETC. SHALL BE COMPACTED IN ACCORDANCE WITH LOCAL REQUIREMENTS OR CODES. REFER TO GEOTECHNICAL REPORT AND CROSS SECTIONS.
- ALL EARTHEN FILLS SHALL BE PLACED IN COMPACTED LAYERS NOT TO EXCEED 9 INCHES IN THICKNESS.
- FILL MATERIALS SHALL BE FREE OF FROZEN PARTICLES, BRUSH, ROOTS, SOD, OR OTHER FOREIGN OR OBJECTIONABLE MATERIALS THAT WOULD INTERFERE WITH OR PREVENT CONSTRUCTION OF SATISFACTORY FILLS.
- FROZEN MATERIALS OR SOFT, MUCKY, OR HIGHLY COMPRESSIBLE MATERIALS SHALL NOT BE INCORPORATED INTO FILLS.
- FILL SHALL NOT BE PLACED ON SATURATED OR FROZEN SURFACES.
- SEEPS OR SPRINGS ENCOUNTERED DURING CONSTRUCTION SHALL BE HANDLED IN ACCORDANCE WITH THE STANDARD AND SPECIFICATION FOR SUBSURFACE DRAIN OR OTHER APPROVED METHOD.
- ALL GRADED AREAS SHALL BE PERMANENTLY STABILIZED IMMEDIATELY UPON REACHING FINISHED GRADE. CUT SLOPES IN COMPETENT BEDROCK AND ROCK FILLS NEED NOT BE VEGETATED. SEEDED AREAS WITHIN 50 FEET OF A SURFACE WATER, OR AS OTHERWISE SHOWN ON THE PLAN DRAWINGS, SHALL BE BLANKETED ACCORDING TO THE STANDARDS OF THIS PLAN.
- IMMEDIATELY AFTER EARTH DISTURBANCE ACTIVITIES CEASE IN ANY AREA OR SUB-AREA OF THE PROJECT, THE OPERATOR SHALL STABILIZE ALL DISTURBED AREAS. DURING NON-GERMINATING MONTHS, MULCH OR PROTECTIVE BLANKETING SHALL BE APPLIED AS DESCRIBED IN THE PLAN. AREAS NOT AT FINISHED GRADE, WHICH WILL BE REACTIVATED WITHIN 1 YEAR, MAY BE STABILIZED IN ACCORDANCE WITH THE TEMPORARY STABILIZATION SPECIFICATIONS. THOSE AREAS WHICH WILL NOT BE REACTIVATED WITHIN 1 YEAR SHALL BE STABILIZED IN ACCORDANCE WITH THE PERMANENT STABILIZATION SPECIFICATIONS.
- PERMANENT STABILIZATION IS DEFINED AS A MINIMUM UNIFORM, PERENNIAL 70% VEGETATIVE COVER OR OTHER PERMANENT NON-VEGETATIVE COVER WITH A DENSITY SUFFICIENT TO RESIST ACCELERATED EROSION. CUT AND FILL SLOPES SHALL BE CAPABLE OF RESISTING FAILURE DUE TO SLUMPING, SLIDING, OR OTHER MOVEMENTS.
- E&S BMPs SHALL REMAIN FUNCTIONAL AS SUCH UNTIL ALL AREAS TRIBUTARY TO THEM ARE PERMANENTLY STABILIZED OR UNTIL THEY ARE REPLACED BY ANOTHER BMP APPROVED BY THE LOCAL CONSERVATION DISTRICT OR THE DEPARTMENT.
- UPON COMPLETION OF ALL EARTH DISTURBANCE ACTIVITIES AND PERMANENT STABILIZATION OF ALL DISTURBED AREAS, THE OWNER AND/OR OPERATOR SHALL CONTACT THE LOCAL CONSERVATION DISTRICT FOR AN INSPECTION PRIOR TO REMOVAL/CONVERSION OF THE E&S BMPs.
- AFTER FINAL SITE STABILIZATION HAS BEEN ACHIEVED, TEMPORARY EROSION AND SEDIMENT BMPs MUST BE REMOVED OR CONVERTED TO PERMANENT POST CONSTRUCTION STORMWATER MANAGEMENT BMPs. AREAS DISTURBED DURING REMOVAL OR CONVERSION OF THE BMPs SHALL BE STABILIZED IMMEDIATELY IN ORDER TO ENSURE RAPID RE-VEGETATION OF DISTURBED AREAS, SUCH REMOVAL/CONVERSIONS ARE TO BE DONE ONLY DURING THE GERMINATING SEASON.
- UPON COMPLETION OF ALL EARTH DISTURBANCE ACTIVITIES AND PERMANENT STABILIZATION OF ALL DISTURBED AREAS, THE OWNER AND/OR OPERATOR SHALL CONTACT THE LOCAL CONSERVATION DISTRICT TO SCHEDULE A FINAL INSPECTION.
- FAILURE TO CORRECTLY INSTALL E&S BMPs, FAILURE TO PREVENT SEDIMENT-LOADED RUNOFF FROM LEAVING THE CONSTRUCTION SITE, OR FAILURE TO TAKE IMMEDIATE CORRECTIVE ACTION TO RESOLVE FAILURE OF E&S BMPs MAY RESULT IN ADMINISTRATIVE, CIVIL, AND/OR CRIMINAL PENALTIES BEING INSTITUTED BY THE DEPARTMENT AS DEFINED IN SECTION 607 OF THE PENNSYLVANIA CLEAN STREAMS LAW, THE CLEAN STREAMS LAW PROVIDES FOR UP TO \$10,000 PER DAY IN CIVIL PENALTIES, UP TO \$10,000 IN SUMMARY CRIMINAL PENALTIES, AND UP TO \$25,000 IN MIDDLEMANOR CRIMINAL PENALTIES FOR EACH VIOLATION.
- CONCRETE WASH WATER SHALL BE HANDLED IN THE MANNER DESCRIBED ON THE PLAN DRAWINGS. IN NO CASE SHALL IT BE ALLOWED TO ENTER ANY SURFACE WATERS OR GROUNDWATER SYSTEMS.
- EROSION CONTROL BLANKETING SHALL BE INSTALLED ON ALL SLOPES 3H:1V OR STEEPER WITHIN 50 FEET OF A SURFACE WATER AND ON ALL OTHER DISTURBED AREAS SPECIFIED ON THE PLAN MAPS AND/OR DETAIL SHEETS.
- FILL MATERIAL FOR EMBANKMENTS SHALL BE FREE OF ROOTS, OR OTHER WOODY VEGETATION, ORGANIC MATERIAL, LARGE STONES, AND OTHER OBJECTIONABLE MATERIALS. THE EMBANKMENT SHALL BE COMPACTED IN MAXIMUM 10" LAYERED LIFTS AT 90% DENSITY.
- SPREAD TOPSOIL, SEED AND MULCH ALL DISTURBED AREAS, USING THE SPECIFIED SEEDING REQUIREMENTS. ALL DISTURBED AREAS MUST BE TEMPORARILY STABILIZED IF REMAINING IDLE FOR MORE THAN 4 DAYS. IN THE ABSENCE OF A SOIL TEST, LIME SHOULD BE ADDED AT A RATE OF 275 LBS/1000 S.F. LIKEWISE, THE FERTILIZER RATES SHOULD BE INCREASED TO 1250 LBS/AC. OF 8-16-16 AND 2000 LBS/AC. OF 5-10-10.

1	STANDARD E&S NOTES
C401	

MAINTENANCE AND INSPECTION SCHEDULE

CONTROL MEASURE	INSPECT	PROBLEMS TO LOOK FOR	POSSIBLE REMEDIES
VEGETATION	WEEKLY AND AFTER EVERY RUNOFF EVENT	SEDIMENT AT TOE OF SLOPE RILLS AND GULLIES FORMING BARE SOIL PATCHES	CHECK FOR TOP-OF-SLOPE DIVERSION AND INSTALL IF NEEDED. FILL RILLS AND REGRADE GULLIED SLOPES. RESEED, FERTILIZE, AND MULCH BARE AREAS.
ROCK CONSTRUCTION ENTRANCES	DAILY	SINK HOLES OR RUTS SEDIMENT ON PUBLIC ROADWAY	ADD ROCK TO BRING TO SPECIFIED DIMENSIONS. SWEEP MATERIAL BACK TO PROJECT SITE. DO NOT WASH ROADWAY WITH WATER.
COMPOST FILTER SOCK	WEEKLY AND AFTER EVERY RUNOFF EVENT	SEDIMENT ACCUMULATION ON UPSTREAM SIDE PONDING WATER	REMOVE SEDIMENT WHEN LEVEL REACHES HALF OF THE EFFECTIVE HEIGHT. REMOVE SEDIMENT OR ADD ADDITIONAL SILT/SOX TO REDUCE SLOPE LENGTH.
INLET PROTECTION	WEEKLY AND AFTER EVERY RUNOFF EVENT	SEDIMENT ACCUMULATION SEDIMENT ACCUMULATED IN INLET FILTER BAG TO 1/2 FULL RUNOFF ESCAPING AROUND BARRIER FILTER BAG TORN OR DAMAGED	REMOVE SEDIMENT AND DISPOSE ON SITE. CLEAN AND/OR REPLACE BAG. REBUILD BARRIER. REPLACE BAG.
SLOPE BLANKET	WEEKLY AND AFTER EVERY RUNOFF EVENT	LOSS OF PROTECTIVE LINING TORN/PUNCTURED FABRIC	RESHAPE AS NECESSARY AND REPLACE FABRIC. OVERLAP WITH CONTINUOUS PIECE OF MATTING. SECURELY ANCHOR WITH PROPER STAPLES.
CONCRETE WASHOUT FACILITY	DAILY	DAMAGED OR LEAKING WASHOUT ACCUMULATION OF MATERIALS (50% CAPACITY)	DEACTIVATED AND REPAIRED OR REPLACED AS NECESSARY. PLASTIC LINERS SHOULD BE REPLACED WITH EACH CLEANING.
PUMPED WATER FILTER BAG	WHEN IN USE	SEDIMENT ACCUMULATION IN BAG WHEN IT REACHES HALF FULL LEAKY DISCHARGE HOSE	REPLACE FILTER BAG AND DISPOSE AT DEP APPROVED LOCATION. SECURE CLAMP CONNECTING DISCHARGE HOSE AND FILTER BAG.

- ALL MAINTENANCE INSPECTIONS SHALL BE LOGGED ONTO DEP FORM # 3800-FM-8CW271D, DATED 04/20/25, AND KEPT ON THE SITE AT ALL TIMES.
- ALL REPAIRS TO BE COMPLETED WITHIN 24 HOURS OF DISCOVERY.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL TEMPORARY CONTROL MEASURES DURING CONSTRUCTION.
- ALL DEVICES SHOULD BE INSPECTED WEEKLY AND AFTER EVERY RUNOFF EVENT. IF, DURING THESE INSPECTIONS, ANY DEVICE IS FOUND TO BE CLOGGED, DAMAGED, HALF-FULL OF SILT OR NOT FULLY OPERATIONAL, THE DEVICE(S) SHALL BE CLEANED OF ALL DEBRIS.
- UPON COMPLETION OF THE PROJECT, RECYCLING OR DISPOSAL OF ALL TEMPORARY EROSION CONTROL MATERIALS SHALL BE THE RESPONSIBILITY OF THE EARTHMOVING CONTRACTOR.
- CONSTRUCTION WASTE INCLUDES BUT IS NOT LIMITED TO: EXCESS SOIL MATERIALS, SILT REMOVED FROM EROSION CONTROL FACILITIES, EXCESS BUILDING MATERIALS, CONCRETE WASH WATER, SANITARY WASTES, AND OBSOLETE EROSION CONTROL MATERIALS (SILT FENCE, SILT BAGS, ETC.) OBSOLETE EROSION CONTROL MATERIALS AND EXCESS BUILDING MATERIALS MUST BE DISPOSED OF AT A DEP APPROVED SITE.
- SILT REMOVED FROM SEDIMENT CONTROL FACILITIES SHALL BE SPREAD ON THE TOPSOIL STOCKPILE, OR IN LAWN OR LANDSCAPE AREAS.

2	MAINTENANCE SCHEDULE
C401	

TABLE 11.1
CUBIC YARDS OF TOPSOIL REQUIRED FOR APPLICATION TO VARIOUS DEPTHS

DEPTH (IN)	PER 1,000 SQUARE FEET	PER ACRE
1	3.1	134
2	6.2	268
3	9.3	403
4	12.4	537
5	15.5	672
6	18.6	806
7	21.7	940
8	24.8	1,074

TEMPORARY SEEDING SPECIFICATIONS:

- LIMESTONE:** RAW, GROUND AGRICULTURAL LIMESTONE CONTAINING MORE THAN 90 PERCENT CALCIUM CARBONATES (ADJUST PH LEVELS TO 8.5 TO 7.0) APPLY AT A RATE OF 410 LBS/1000 S.Y., UNLESS OTHERWISE INDICATED BY SOIL TESTS.
- COMMERCIAL FERTILIZER:** IN ABSENCE OF A SOIL TEST, USE 100 LBS / 1000 S.Y. OF 10-10-10 MIXED INTO SEED BED PRIOR TO SEEDING, OR IN A TANK WITH SEED WHEN HYDROSEEDING.
- MULCH:** CLEAN OAT OR WHEAT STRAW SHALL BE FREE FROM MATURE SEED-BEARING STALKS OR ROOTS OF PROHIBITED OR NOXIOUS WEEDS AS DEFINED BY THE PENNSYLVANIA SEED ACT '947, APPLY AT A RATE OF 3 TONS/AC (139 LBS/1000 SF). PRECAUTION SHALL BE TAKEN TO STABILIZE THE MULCH UNTIL THE VEGETATIVE COVER IS ESTABLISHED.
- SEED MIXTURE:** SEED MIXTURE SHALL BEAR A GUARANTEED STATEMENT OF ANALYSIS AND SHALL BE COMPOSED OF THE FOLLOWING VARIETIES AND MIXED IN THE PROPORTIONS SPECIFIED
- | TEMPORARY SEED MIXTURE | PROPORTION BY WEIGHT | MINIMUM PURITY | MINIMUM GERMINATION |
|------------------------|----------------------|----------------|---------------------|
| ANNUAL RYEGRASS | 100% | 94% | 90% |
- THREE (3) PERCENT REDTOP MAY BE ADDED TO THE LAWN MIXTURE ON SLOPE AREAS OR FOR LATE SPRING OR LATE FALL SEEDING.
- SEEDING RATE:** TEMPORARY SEEDING SHALL BE APPLIED AT A RATE OF TEN (10) POUNDS PER ONE THOUSAND (1,000) SQUARE YARDS.
- NOTES:** ALL AREAS TO BE SEEDED SHALL BE LOOSENEED TO A DEPTH OF AT LEAST TWO INCHES BY MECHANICAL MEANS, OR AS APPROVED BY THE LANDSCAPE ARCHITECT.
- MULCH SEEDED AREAS IMMEDIATELY AFTER SEEDING.
- SEED ALL DISTURBED AREAS WITH THE TEMPORARY SEED MIXTURE EXCEPT THOSE AREAS SHOWN TO BE SEEDED WITH STEEP SLOPE MIXTURE, UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- HYDROSEEDING WITH TACIFER SHALL BE USED IN LIEU OF MULCH.
- PERMANENT SEEDING SPECIFICATIONS:**
- TOPSOIL:** 4" MIN. TOPSOIL SHALL BE REQUIRED ON ALL DISTURBED AREAS AFTER SITE HAS BEEN RECLAIMED/RESTORED.
- LIMESTONE:** RAW, GROUND AGRICULTURAL LIMESTONE CONTAINING MORE THAN 90 PERCENT CALCIUM CARBONATES (ADJUST PH LEVELS TO 8 TO 7.0) APPLY AT A RATE OF 240 LBS / 1000 S.Y., UNLESS OTHERWISE INDICATED BY SOIL TESTS.
- COMMERCIAL FERTILIZER:** IN ABSENCE OF A SOIL TEST, USE 210 LBS / 1000 S.Y. OF 10-10-20 MIXED INTO SEED BED PRIOR TO SEEDING, OR IN A TANK WITH SEED WHEN HYDROSEEDING.
- INOCULANT:** INOCULANT FOR TREATING LEGUMINOUS SEEDS SHALL BE A STANDARD COMMERCIAL PRODUCT CONSISTING OF A SUITABLE CARRIER CONTAINING A CULTURE OF NITROGEN FIXING BACTERIA SPECIFIC FOR SEEDS TO BE INOCULATED. INOCULANT SHALL NOT BE USED LATER THAN DATE INDICATED ON THE CONTAINER.
- MULCH:** CLEAN OAT OR WHEAT STRAW SHALL BE FREE FROM MATURE SEED-BEARING STALKS OR ROOTS OF PROHIBITED OR NOXIOUS WEEDS AS DEFINED BY THE PENNSYLVANIA SEED ACT '947, APPLY AT A RATE OF 3 TONS/AC (139 LBS/1000 SF). PRECAUTION SHALL BE TAKEN TO STABILIZE THE MULCH UNTIL THE VEGETATIVE COVER IS ESTABLISHED.
- SEED MIXTURE:** SEED MIXTURE SHALL BEAR A GUARANTEED STATEMENT OF ANALYSIS AND SHALL BE COMPOSED OF THE FOLLOWING VARIETIES AND MIXED IN THE PROPORTIONS SPECIFIED
- | PERMANENT SEEDING MIXTURE | PROPORTION BY WEIGHT |
|---|----------------------|
| SORGHASTRUM NITANS, TOMAHAWK (INDIANGRASS, TOMAHAWK) | 31.1% |
| LOLIUM MULTIFLORUM (ANNUAL RYEGRASS) | 20.0% |
| ANDROPOGON CEPRANDI, NIAGARA (BIG BLUESTEM, NIAGARA) | 14.0% |
| ELYMUS VIRGINICUS, MADISON (VIRGINIA WILDRYE, MADISON) | 10.0% |
| ELYMUS CANADENSIS (CANADA WILDRYE) | 7.0% |
| AGROSTIS PERENNANS, ALBANY PINE BUSH NY ECOTYPE (AULTUN BENTGRASS, ALBANY PINE BUSH NY ECOTYPE) | 4.0% |
| PANICUM VIRGATUM, SHAWNEE (SWITCHGRASS, SHAWNEE) | 4.0% |
| PANICUM CLANDESTINUM, TIGOA (DEERTONGUE, TIGOA) | 3.0% |
| ECHINACEA PURPUREA (PURPLE CONEFLOWER) | 1.5% |
| CHAMAECRISTA FASCICULATA, PA ECOTYPE (PARTIBIDGE PEA, PA ECOTYPE) | 1.3% |
| HELIOPSIS HELIANTHOIDES, PA ECOTYPE (OXEYE SUNFLOWER, PA ECOTYPE) | 1.2% |
| COREOPSIS LANCOLATA (LANCELEAF COREOPSIS) | 1.0% |
| RUBROCKIA HIRTIA, COASTAL PLAIN NY ECOTYPE (BLACKEYED SUSAN, COASTAL PLAIN NY ECOTYPE) | 1.0% |
| MORONDA FISTULOSA, FORT INDIANTOWN GAP-PA ECOTYPE (WILD BERGMONI, FORT INDIANTOWN GAP-PA ECOTYPE) | 0.3% |
| ASTER LAEVIS, NY ECOTYPE (SMOOTH BLUE ASTER, NY ECOTYPE) | 0.1% |
| ASTER NOVAE ANGLIAE, PA ECOTYPE (NEW ENGLAND ASTER, PA ECOTYPE) | 0.1% |
| SOLIDAGO JUNCEA, PA ECOTYPE (EARLY GOLDENROD, PA ECOTYPE) | 0.1% |
| SOLIDAGO NEMORALIS, PA ECOTYPE (GRAY GOLDENROD, PA ECOTYPE) | 0.1% |
- SEEDING RATE:** PERMANENT SEEDING SHALL BE APPLIED AT A RATE OF 21 POUNDS PER 1,000 SQUARE FEET.
- PREPARATION OF SEEDING:** HYDROSEEDING WITH TACIFER SHALL BE USED IN LIEU OF MULCH.
- GRADE AS NECESSARY TO BRING SUBGRADE TO A TRUE, SMOOTH SLOPE PARALLEL TO AND SIX INCHES BELOW FINISHED GRADE. PLACE TOPSOIL OVER SPECIFIED AREAS TO A DEPTH SUFFICIENTLY GREATER THAN SIX (6) OR EIGHT (8) INCHES SO THAT AFTER SETTLEMENT AND LIGHT ROLLING, THE COMPLETE WORK WILL CONFORM TO LINES GRADES AND ELEVATIONS SHOWN.
- FERTILIZER AND AGRICULTURAL LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE SOIL BY ROTOTILLING OR OTHER METHOD TO A MINIMUM DEPTH OF FOUR (4) INCHES. THE ENTIRE SURFACE SHALL THEN BE REGRADED AND ROLLED. AREAS TO BE SEEDED SHALL THEN BE LOOSENEED TO A DEPTH OF TWO (2) INCHES. SEEDING SHALL BE DONE IN TWO SEPARATE OPERATIONS. THE SECOND SEEDING SHALL BE DONE IMMEDIATELY AFTER THE FIRST AND AT RIGHT ANGLES TO THE FIRST SEEDING AND LIGHTLY RAKED INTO THE SOIL. MULCH SEEDED AREAS IMMEDIATELY AFTER SEEDING.

TABLE 11.2
SOIL AMENDMENT APPLICATION RATE EQUIVALENTS

SOIL AMENDMENT	PERMANENT SEEDING APPLICATION RATE			NOTES
	PER ACRE	PER 1,000 SQ. FT.	PER 1,000 SQ. YD.	
AGRICULTURAL LIME	6 TONS	240 LB.	2,480 LB.	OR AS PER SOIL TEST; MAY NOT BE REQUIRED IN AGRICULTURAL FIELDS.
10-20-20 FERTILIZER	1,000 LB.	25 LB.	210 LB.	OR AS PER SOIL TEST; MAY NOT BE REQUIRED IN AGRICULTURAL FIELDS.
TEMPORARY SEEDING APPLICATION RATE				
AGRICULTURAL LIME	1 TON	40 LB.	410 LB.	TYPICALLY NOT REQUIRED FOR TOPSOIL STOCKPILES.
10-10-10 FERTILIZER	500 LB.	12.5 LB.	100 LB.	TYPICALLY NOT REQUIRED FOR TOPSOIL STOCKPILES.

NOTE: A COMPOST BLANKET WHICH MEETS THE REQUIRED STANDARDS MAY BE SUBSTITUTED FOR THE SOIL AMENDMENTS SHOWN IN TABLE 11.2.

3	SEEDING SPECIFICATIONS
C401	

CHARTERED 1810
CORPORATION
GATEWAY ENGINEERS
A FULL-SERVICE CIVIL ENGINEERING FIRM

PROFESSIONAL ENGINEER
PENNSYLVANIA

REVISION RECORD

Date	No	Description
2025-11-11	01	WPP Update
2025-12-16	02	ACCD TECHNICAL NOTES RESPONSE
	03	
	04	
	05	
	06	
	07	
	08	

4
C401 VISUAL SITE INSPECTION REPORT

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031

PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARS, PA 16846

EROSION AND SEDIMENTATION CONTROL DETAILS

Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number:
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

CLEAN FILL AND ENVIRONMENTAL DUE DILIGENCE:

THE NPDES PERMIT COVERS THE MOVING, DEPOSITING, STOCKPILING OR STORING OF SOIL, ROCK OR EARTH MATERIALS. IF THE SITE WILL NEED TO HAVE FILL IMPORTED FROM AN OFF SITE LOCATION, THE RESPONSIBILITY FOR PERFORMING ENVIRONMENTAL DUE DILIGENCE AND THE DETERMINATION OF CLEAN FILL WILL RESIDE WITH THE CONTRACTOR RESPONSIBLE FOR EARTHMOVING OPERATIONS.

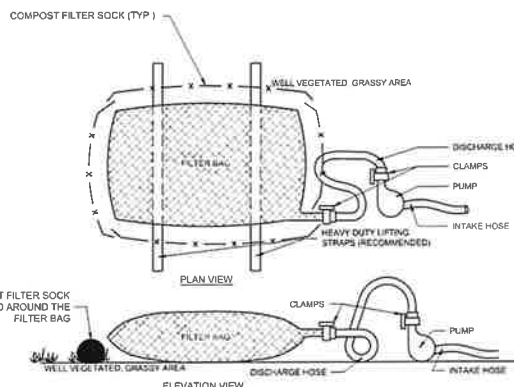
ENVIRONMENTAL DUE DILIGENCE MEANS PERFORMING INVESTIGATIVE TECHNIQUES FOR THE IMPORTED MATERIAL. THIS INCLUDES, BUT IS NOT LIMITED TO, VISUAL PROPERTY INSPECTIONS, ELECTRONIC DATA BASE SEARCHES, REVIEW OF PROPERTY OWNERSHIP, REVIEW OF PROPERTY USE HISTORY, SANBORN MAPS, ENVIRONMENTAL QUESTIONNAIRES, TRANSACTION SCREENS, ANALYTICAL TESTING, AND ENVIRONMENTAL ASSESSMENTS OR AUDITS. ANALYTICAL TESTING IS NOT A REQUIRED PART OF DUE DILIGENCE UNLESS VISUAL INSPECTION AND/OR REVIEW OF THE PAST LAND USE OF THE PROPERTY INDICATES THAT THE FILL MAY HAVE BEEN SUBJECTED TO A SPILL OR RELEASE OF REGULATED SUBSTANCE, IF THE FILL MAY HAVE BEEN AFFECTED BY A SPILL OR RELEASE OF A REGULATED SUBSTANCE, IT MUST BE TESTED TO DETERMINE IF IT QUALIFIES AS CLEAN FILL. TESTING SHOULD BE PERFORMED IN ACCORDANCE WITH APPENDIX A OF THE DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) POLICY MANAGEMENT OF FILL. FILL MATERIAL THAT DOES NOT QUALIFY AS CLEAN FILL IS REGULATED FILL. REGULATED FILL IS WASTE AND MUST BE MANAGED IN ACCORDANCE WITH THE DEP MUNICIPAL OR RESIDUAL WASTE REGULATIONS BASED ON 25 PA CODE, CHAPTERS 287 RESIDUAL WASTE MANAGEMENT OR 271 MUNICIPAL WASTE MANAGEMENT, WHICHEVER IS APPLICABLE.

CLEAN FILL IS DEFINED AS UNCONTAMINATED, NON-WATER SOLUBLE, NON-DECOMPOSABLE, INERT, SOLID MATERIAL. THE TERM INCLUDES SOIL, ROCK, STONE, DREDGED MATERIAL, USED ASPHALT, AND BRICK, BLOCK OR CONCRETE FROM CONSTRUCTION AND DEMOLITION ACTIVITIES THAT IS SEPARATE FROM OTHER WASTE AND IS RECOGNIZABLE AS SUCH. THE TERM DOES NOT INCLUDE MATERIALS PLACED IN OR ON THE WATERS OF THE COMMONWEALTH UNLESS OTHERWISE AUTHORIZED. (THE TERM USED, ASPHALT DOES NOT INCLUDE MILLED ASPHALT OR ASPHALT THAT HAS BEEN PROCESSED FOR RE-USE.)

1 CLEAN FILL AND ENVIRONMENTAL DUE DILIGENCE

- UPON COMPLETION OF THE PROJECT, RECYCLING OR DISPOSAL OF ALL MATERIALS WHICH COULD CAUSE POLLUTION SHALL BE THE RESPONSIBILITY OF THE EARTHMOVING CONTRACTOR.
- CONSTRUCTION WASTE INCLUDES, BUT IS NOT LIMITED TO, EXCESS SOIL MATERIALS, SILT REMOVED FROM EROSION CONTROL FACILITIES, EXCESS BUILDING MATERIALS, CONCRETE WASH WATER, SANITARY WASTES, AND OBSOLETE EROSION CONTROL MATERIALS (SILT FENCE, SILT SOCKS, ETC.).
- SILT REMOVED FROM SEDIMENT CONTROL FACILITIES SHALL BE SPREAD ON THE TOPSOIL STOCKPILE, OR IN LAWN OR LANDSCAPE AREAS.
- OBSOLETE EROSION CONTROL MATERIALS AND EXCESS BUILDING MATERIALS MUST BE DISPOSED OF AT A DEP APPROVED SITE.
- THERE WERE NO KNOWN NATURALLY OCCURRING GEOLOGIC FORMATIONS OR SOIL CONDITIONS LOCATED ON THE SITE THAT HAVE THE POTENTIAL TO CAUSE POLLUTION DURING CONSTRUCTION. IF SUCH CONDITIONS ARE ENCOUNTERED, STOP WORK AND CONTACT THE GEOGRAPHICAL ENGINEER AND/OR DEP GEOLOGICAL ROCK FORMATIONS OR SOIL CONDITIONS THAT ARE IDENTIFIED BY THE GEOLOGICAL ENGINEER ON SITE AS HAZARDOUS TO DEGRADE WATER QUALITY WILL BE ISOLATED IN PLACE OR WILL BE RELOCATED ON SITE FOLLOWED BY APPROPRIATE CAPPING WITH AN IMPERVIOUS SURFACE SUCH AS PAVEMENT OR A BUILDING, OR WILL BE EXCAVATED AND REMOVED TO AN OFF-SITE DISPOSAL AREA APPROVED TO HANDLE HAZARDOUS WASTE, DEPENDING ON THE LEVEL OF CONTAMINATION. BLENDING OF SOIL MAY ACHIEVE ACCEPTABLE STANDARDS.

2 RECYCLING AND WASTE DISPOSAL



NOTES:
1. LOW VOLUME FILTER BAGS SHALL BE MADE FROM NON-WOVEN GEOTEXTILE MATERIAL, SEWN WITH HIGH STRENGTH, DOUBLE STITCHED 1" TYPE SEAMS. THEY SHALL BE CAPABLE OF TRAPPING PARTICLES LARGER THAN 150 MICRONS. HIGH VOLUME FILTER BAGS MAY BE MADE FROM WOVEN GEOTEXTILES THAT MEET THE FOLLOWING STANDARDS:

PROPERTY	TEST METHOD	MINIMUM STANDARD
Avg. Break Strength	ASTM D-4814	200 LBS
UV Resist.	ASTM D-2434	200 HRS
UV Resist.	ASTM D-2434	200 HRS
UV Resist.	ASTM D-2434	200 HRS
UV Resist.	ASTM D-2434	200 HRS
UV Resist.	ASTM D-2434	200 HRS

2. A SUITABLE MEANS OF ACCESSING THE BAG WITH MACHINERY REQUIRED FOR DISPOSAL PURPOSES MUST BE PROVIDED. FILTER BAGS SHALL BE REPLACED WHEN THEY HAVE BECOME 1/2 FULL. SPARE BAGS SHALL BE KEPT AVAILABLE FOR REPLACEMENT OF THOSE THAT HAVE FAILED OR ARE FILLED. IT IS RECOMMENDED THAT BAGS BE PLACED ON STRIPS TO FACILITATE REMOVAL.
3. BAGS SHALL BE LOCATED IN WELL-VEGETATED (GRASSY) AREA AND DISCHARGE ONTO STABLE EROSION RESISTANT AREAS. WHERE THIS IS NOT POSSIBLE, A GEOTEXTILE UNDERLAYMENT AND FLOW PATH SHALL BE PROVIDED. BAGS MAY BE PLACED ON FILTER STONE TO INCREASE DISCHARGE CAPACITY. BAGS SHALL NOT BE PLACED ON SLOPES GREATER THAN 3% FOR SLOPES EXCEEDING 3%, CLEAN ROCK OR OTHER NON-ERODIBLE AND NON-POLLUTING MATERIAL, MAY BE PLACED UNDER THE BAG TO REDUCE SLOPE STEEPNESS.

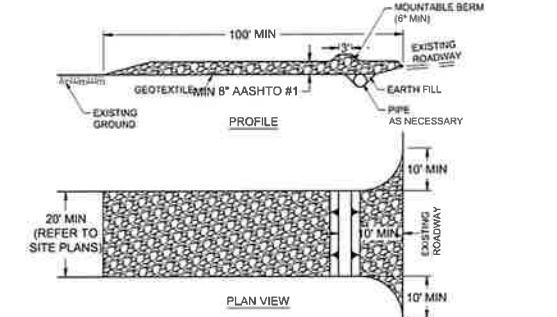
4. NO DOWN SLOPE SEDIMENT BARRIER IS REQUIRED FOR MOST INSTALLATIONS. COMPOST BERM OR COMPOST FILTER SOCK SHOULD BE INSTALLED BELOW BAGS LOCATED WITHIN 5' OF RECEIVING STREAM OR WHERE GRASSY AREA IS NOT AVAILABLE. A COMPOST BERM OR COMPOST FILTER SOCK SHALL BE PLACED BELOW ANY BAG DISCHARGING TO A SPECIAL PROTECTION SURFACE WATER.

5. THE PUMP DISCHARGE HOSE SHALL BE INSERTED INTO THE BAGS IN THE MANNER SPECIFIED BY THE MANUFACTURER AND SECURELY CLAMPED. A PIECE OF PVC PIPE IS RECOMMENDED FOR THIS PURPOSE.
6. THE PUMPING RATE SHALL BE NO GREATER THAN 750 GPM OR 12" THE MAXIMUM SPECIFIED BY THE MANUFACTURER, WHICHEVER IS LESS. PUMP INTAKES SHOULD BE FLOATING AND SCREENED.

7. FILTER BAGS SHALL BE INSPECTED DAILY; IF ANY PROBLEM IS DETECTED, PUMPING SHALL CEASE IMMEDIATELY AND NOT RESUME UNTIL THE PROBLEM IS CORRECTED.
8. FILTER BAGS SHALL BE PLACED ON A FLAT AND VEGETATED OR PERMANENTLY STABILIZED GROUND. THE FILTERED WATER BEING PUMPED FROM THE BAG SHALL PASS THROUGH AN ADDITIONAL BAG CONTROL PRIOR TO LEAVING THE SITE.

9. WHEN LOCATED WITHIN A STREAM THAT HAS IMPAIRMENTS THAT REQUIRE ABACT BMP'S, A 12" COMPOST FILTER SOCK SHALL BE PLACED AROUND THE FILTER BAG AT ALL TIMES.

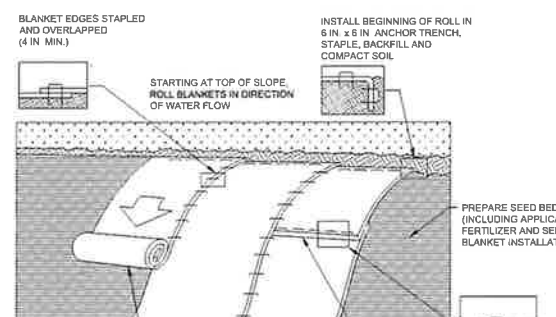
6 PUMPED WATER FILTER BAG



- NOTES:
1. TOPSOIL SHALL BE REMOVED PRIOR TO INSTALLATION OF ROCK CONSTRUCTION ENTRANCE.
2. EXTEND ROCK OVER WIDTH OF ENTRANCE.
3. RUNOFF SHALL BE DIVERTED FROM ROADWAY TO A SUITABLE SEDIMENT REMOVAL BMP PRIOR TO ENTERING ROCK CONSTRUCTION ENTRANCE.
4. MOUNTABLE BERM SHOULD BE INSTALLED WHEREVER OPTIONAL CULVERT PIPE IS USED. PIPE TO BE SIZED APPROPRIATELY FOR SIZE OF DITCH BEING CROSSED.
5. REFER TO SITE PLANS FOR SIZE AND LOCATION WHERE APPLICABLE. REFER TO HOP PLANS FOR SIZE AND LOCATION ON STATE ROADWAYS.

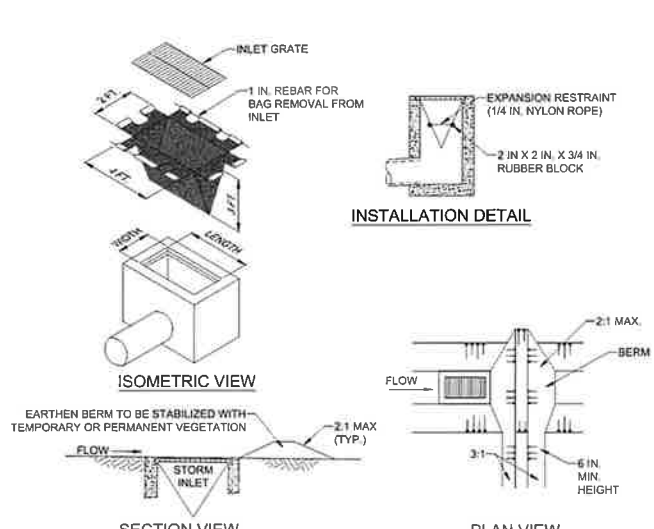
MAINTENANCE:
1. ROCK CONSTRUCTION ENTRANCE THICKNESS SHALL BE CONSTANTLY MAINTAINED TO THE SPECIFIED DIMENSIONS BY ADDING ROCK. A STOCKPILE SHALL BE MAINTAINED ON SITE FOR THIS PURPOSE.
2. ALL SEDIMENT DEPOSITED ON PAVED ROADWAYS SHALL BE REMOVED AND RETURNED TO THE CONSTRUCTION SITE IMMEDIATELY. IF EXCESSIVE AMOUNTS OF SEDIMENT ARE BEING DEPOSITED ON ROADWAY, EXTEND LENGTH OF ROCK CONSTRUCTION ENTRANCE BY 5' INCREMENTS UNTIL CONDITION IS ALLEVIATED OR INSTALL WASH RACK, WASHING THE ROADWAY OR SWEEPING THE DEPOSIT INTO THE ROADWAY DITCHES, CULVERTS, OR OTHER DRAINAGE WAYS IS NOT ACCEPTABLE.

3 ROCK CONSTRUCTION ENTRANCE



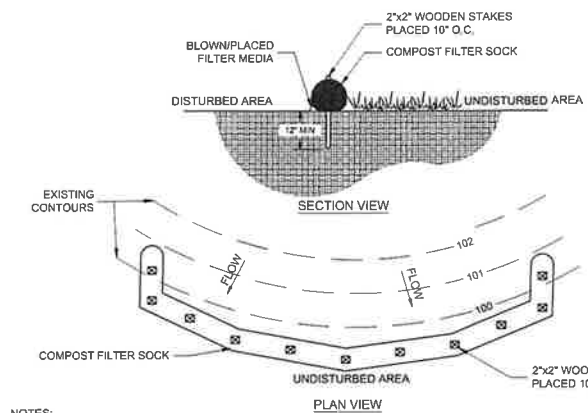
- NOTES:
SEED AND SOIL AMENDMENTS SHALL BE APPLIED ACCORDING TO THE RATES IN THE PLAN DRAWINGS PRIOR TO INSTALLING THE BLANKET.
PROVIDE ANCHOR TRENCH AT TOE OF SLOPE IN SIMILAR FASHION AS AT TOP OF SLOPE.
SLOPE SURFACE SHALL BE FREE OF ROCKS, CLODS, STICKS, AND GRASS.
BLANKET SHALL HAVE GOOD CONTINUOUS CONTACT WITH UNDERLYING SOIL THROUGHOUT ENTIRE LENGTH. LAY BLANKET LOOSELY AND STAKE OR STAPLE TO MAINTAIN DIRECT CONTACT WITH SOIL. DO NOT STRETCH BLANKET.
THE BLANKET SHALL BE STAPLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
BLANKETED AREAS SHALL BE INSPECTED WEEKLY AND AFTER EACH RUNOFF EVENT UNTIL PERENNIAL VEGETATION IS ESTABLISHED TO A MINIMUM UNIFORM 70% COVERAGE THROUGHOUT THE BLANKETED AREA. DAMAGED OR DISPLACED BLANKETS SHALL BE RESTORED OR REPLACED WITHIN 1 CALENDAR DAYS.

4 EROSION CONTROL BLANKET



- NOTES:
MAXIMUM DRAINAGE AREA = 1/2 ACRE.
INLET PROTECTION SHALL NOT BE REQUIRED FOR INLET TRIBUTARY TO SEDIMENT BASIN OR TRAP. BERMS SHALL BE REQUIRED FOR ALL INSTALLATIONS.
ROLLED EARTHEN BERM IN ROADWAY SHALL BE MAINTAINED UNTIL ROADWAY IS STONED. ROAD SUBBASE BERM ON ROADWAY SHALL BE MAINTAINED UNTIL ROADWAY IS PAVED. EARTHEN BERM IN CHANNEL SHALL BE MAINTAINED UNTIL PERMANENT STABILIZATION IS COMPLETED OR REMAIN PERMANENTLY.
AT A MINIMUM, THE FABRIC SHALL HAVE A MINIMUM GRAB TENSILE STRENGTH OF 120 LBS., A MINIMUM BURST STRENGTH OF 200 PSI, AND A MINIMUM TRAPEZOIDAL TEAR STRENGTH OF 50 LBS. FILTER BAGS SHALL BE CAPABLE OF TRAPPING ALL PARTICLES NOT PASSING A NO. 40 SIEVE.
INLET FILTER BAGS SHALL BE INSPECTED ON A WEEKLY BASIS AND AFTER EACH RUNOFF EVENT. BAGS SHALL BE EMPTIED AND RINSED OR REPLACED WHEN HALF FULL OR WHEN FLOW CAPACITY HAS BEEN REDUCED SO AS TO CAUSE FLOODING OR BYPASSING OF THE INLET. DAMAGED OR CLOGGED BAGS SHALL BE REPLACED. A SUPPLY SHALL BE MAINTAINED ON SITE FOR REPLACEMENT OF BAGS. ALL NEEDED REPAIRS SHALL BE INITIATED IMMEDIATELY AFTER THE INSPECTION. DISPOSE ACCUMULATED SEDIMENT AS WELL AS ALL USED BAGS ACCORDING TO THE PLAN NOTES.
DO NOT USE ON MAJOR PAVED ROADWAYS WHERE PONDING MAY CAUSE TRAFFIC HAZARDS.

8 INLET PROTECTION



- NOTES:
1. SOCK FABRIC SHALL MEET THE STANDARDS OF TABLE 4.1. COMPOST SHALL MEET THE STANDARDS OF TABLE 4.2.
2. COMPOST FILTER SOCK SHALL BE PLACED AT EXISTING LEVEL GRADE. BOTH ENDS OF THE SOCK SHALL BE EXTENDED AT LEAST 8 FEET UP SLOPE AT 45 DEGREES TO THE MAIN SOCK ALIGNMENT. MAXIMUM SLOPE LENGTH ABOVE ANY SOCK SHALL NOT EXCEED THAT SHOWN ON FIGURE 4.2. STAKES MAY BE INSTALLED IMMEDIATELY DOWNSLOPE OF THE SOCK IF SO SPECIFIED BY THE MANUFACTURER.
3. TRAFFIC SHALL NOT BE PERMITTED TO CROSS FILTER SOCKS.
4. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT REACHES HALF THE ABOVEGROUND HEIGHT OF THE SOCK AND DISPOSED IN THE MANNER DESCRIBED ELSEWHERE IN THE PLAN.
5. SOCKS SHALL BE INSPECTED WEEKLY AND AFTER EACH RUNOFF EVENT. DAMAGED SOCKS SHALL BE REPAIRED ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS OR REPLACED WITHIN 24 HOURS OF INSPECTION.
6. BIODEGRADABLE FILTER SOCKS SHALL BE REPLACED AFTER 6 MONTHS; PHOTODEGRADABLE SOCKS AFTER ONE YEAR. POLYPROPYLENE SOCKS SHALL BE REPLACED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
7. UPON STABILIZATION OF THE AREA TRIBUTARY TO THE SOCK, STAKES SHALL BE REMOVED. THE SOCK MAY BE LEFT IN PLACE AND VEGETATED OR REMOVED. IN THE LATTER CASE, THE MESH SHALL BE CUT OPEN AND THE MULCH SPREAD AS A SOIL SUPPLEMENT.
8. SILT SOCK FABRIC MATERIAL SHALL BE MULTI-FILAMENT POLYPROPYLENE (MFFP), MINIMUM FUNCTIONAL LONGEVITY ONE YEAR.

TABLE 4.1 COMPOST SOCK FABRIC MINIMUM SPECIFICATIONS

MATERIAL TYPE	3 mil HDPE	5 mil HDPE	5 mil HDPE	MULTI-FILAMENT POLYPROPYLENE (MFFP)	HEAVY DUTY MULTI-FILAMENT POLYPROPYLENE (HDMFFP)
SOCK DIAMETERS	12" 18"	12" 18" 24" 32"	12" 18" 24" 32"	12" 18" 24" 32"	12" 18" 24" 32"
MESH OPENING	3/8"	3/8"	3/8"	3/8"	1/8"
TENSILE STRENGTH		26 PSI	26 PSI	44 PSI	202 PSI
ULTRAVIOLET STABILITY % ORIG. STRENGTH (ASTM G-155)	23% AT 1000 HR.	23% AT 1000 HR.		100% AT 1000 HR.	100% AT 1000 HR.
MINIMAL FUNCTIONAL LONGEVITY	6 MONTHS	9 MONTHS	6 MONTHS	1 YEAR	2 YEARS

TWO-PLY SYSTEMS

INNER CONTAINMENT NETTING	HDPE BIAXIAL NET CONTINUOUSLY WOUND FUSION-WELDED JUNCTURES 3/4" x 3/4" MAX. APERTURE SIZE
OUTER FILTRATION MESH	COMPOSITE POLYPROPYLENE FABRIC (WOVEN LAYER AND NON-WOVEN FLEECE MECHANICALLY FUSED VIA NEEDLE PUNCH) 3/16" MAX. APERTURE SIZE

SOCK FABRICS COMPOSED OF BURLAP MAY BE USED ON PROJECTS LASTING 6 MONTHS OR LESS.

TABLE 4.2 COMPOST STANDARDS

ORGANIC MATTER CONTENT	25% - 100% (DRY WEIGHT BASIS)
ORGANIC PORTION	FIBROUS AND ELONGATED
Fm	5.5 - 8.5
MOISTURE CONTENT	30% - 60%
PARTICLE SIZE	30%-50% PASS THRU 3/8" SIEVE
SOLUBLE SALT CONCENTRATION	5.0 g/sin (mm-hal-cm) MAXIMUM

5 COMPOST FILTER SOCK

GATEWAY ENGINEERS
A FULL-SERVICE CIVIL ENGINEERING FIRM

COMMONWEALTH OF PENNSYLVANIA
REGISTERED PROFESSIONAL ENGINEER
No. 143076
Exp. 03/31/2028

No.	Date	REVISION RECORD
01	2025-11-11	WPP/UG/016
02	2025-12-16	ACCQ/TECHNICAL NPDES RESPONSE
03		
04		
05		
06		
07		
08		

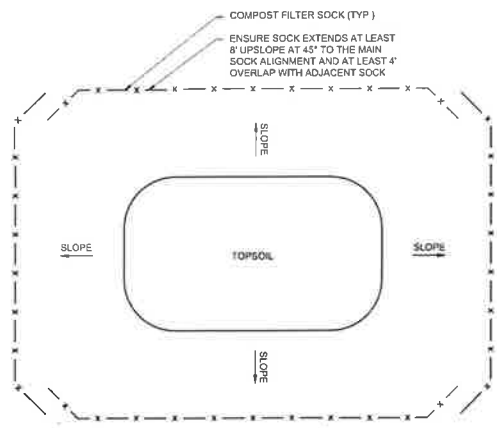
LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR: CE-SF, LP
295 MYOMA ROAD
MATS, PA 16046

EROSION AND SEDIMENTATION CONTROL DETAILS

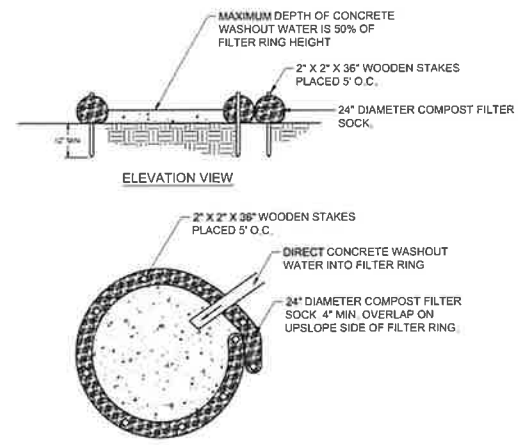
Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: ...
Drawn By: MCLR/RRR
Checked By: JMG
Project Manager: JMG

C402

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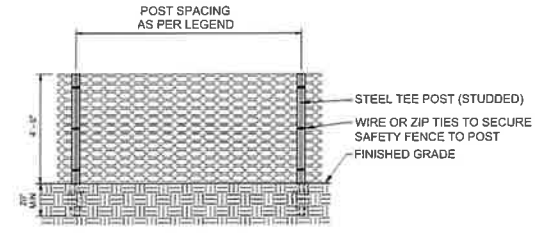


- NOTES:**
1. TOPSOIL STOCKPILE LOCATIONS SHALL BE DETERMINED BY THE CONTRACTOR DURING CONSTRUCTION PER SITE PHASING. ADDITIONAL COORDINATION WITH THE ALLEGHENY COUNTY CONSERVATION DISTRICT MAY BE REQUIRED FOR APPROVAL OF STOCKPILE LOCATIONS.
 2. THE AREA DOWNSLOPE FROM THE COMPOST FILTER SOCK MAY NOT BE UNDER DEVELOPMENT OR OTHERWISE DISTURBED, UNLESS DOWNSLOPE PERIMETER BMPs ARE PROVIDED.
 3. ENSURE DOWNSLOPE COVERAGE ON ALL SIDES OF TOPSOIL STOCKPILE.
 4. COMPOST FILTER SOCK SIZE SHALL BE PROVIDED IN ACCORDANCE WITH FIGURE 4.2.

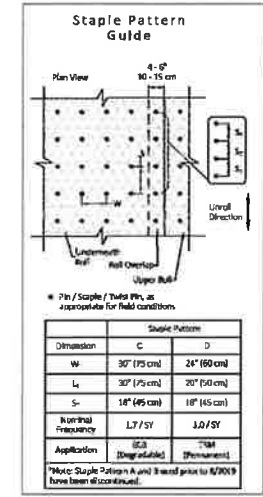


- NOTES:**
1. INSTALL ON FLAT GRADE FOR OPTIMAL PERFORMANCE.
 2. 16" DIAMETER FILTER SOCK MAY BE STACKED ONTO DOUBLE 24" DIAMETER SOCKS IN PYRAMIDAL CONFIGURATION FOR ADDED HEIGHT.
 3. A SUITABLE IMPERVIOUS GEOMEMBRANE LINER SHALL BE PLACED AT THE LOCATION PRIOR TO THE INSTALLING THE SOCKS.
 4. WASHOUT FACILITIES SHOULD NOT BE PLACED WITHIN 50 FT OF STORM DRAINS, OPEN DITCHES, OR SURFACE WATERS.

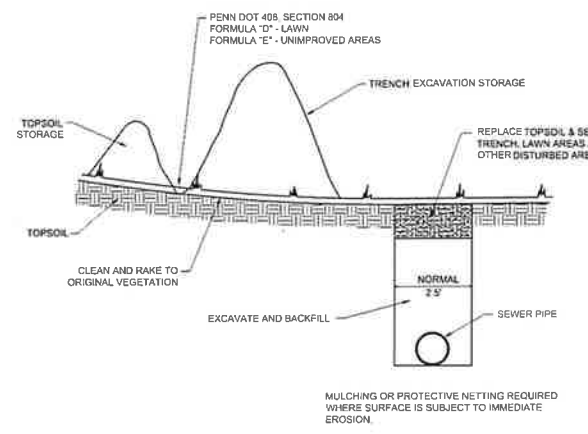
48" SAFETY FENCE, 72" T-POSTS	
SAF12	48" ORANGE FENCE, 12 FEET O.C.
SAF11	48" ORANGE FENCE, 11 FEET O.C.
SAF10	48" ORANGE FENCE, 10 FEET O.C.
SAF9	48" ORANGE FENCE, 9 FEET O.C.
SAF8	48" ORANGE FENCE, 8 FEET O.C.
SAF7	48" ORANGE FENCE, 7 FEET O.C.
SAF6	48" ORANGE FENCE, 6 FEET O.C.



- NOTES:**
1. ORANGE CONSTRUCTION FENCE SHALL BE PLACED AROUND ALL ENVIRONMENTAL FEATURES TO BE PROTECTED ON THE SITE.
 2. SAFETY FENCE SHOULD BE FASTENED SECURELY TO THE T-POSTS.
 3. THE FENCING MUST REMAIN IN PLACE DURING ALL PHASES OF CONSTRUCTION. ANY CHANGE OF THE PROTECTIVE FENCING MUST BE APPROVED.



4 STAPLE PATTERN FOR EROSION CONTROL BLANKET



6 EROSION CONTROL FOR SEWER TRENCHES

1 TOPSOIL STOCKPILE DETAIL

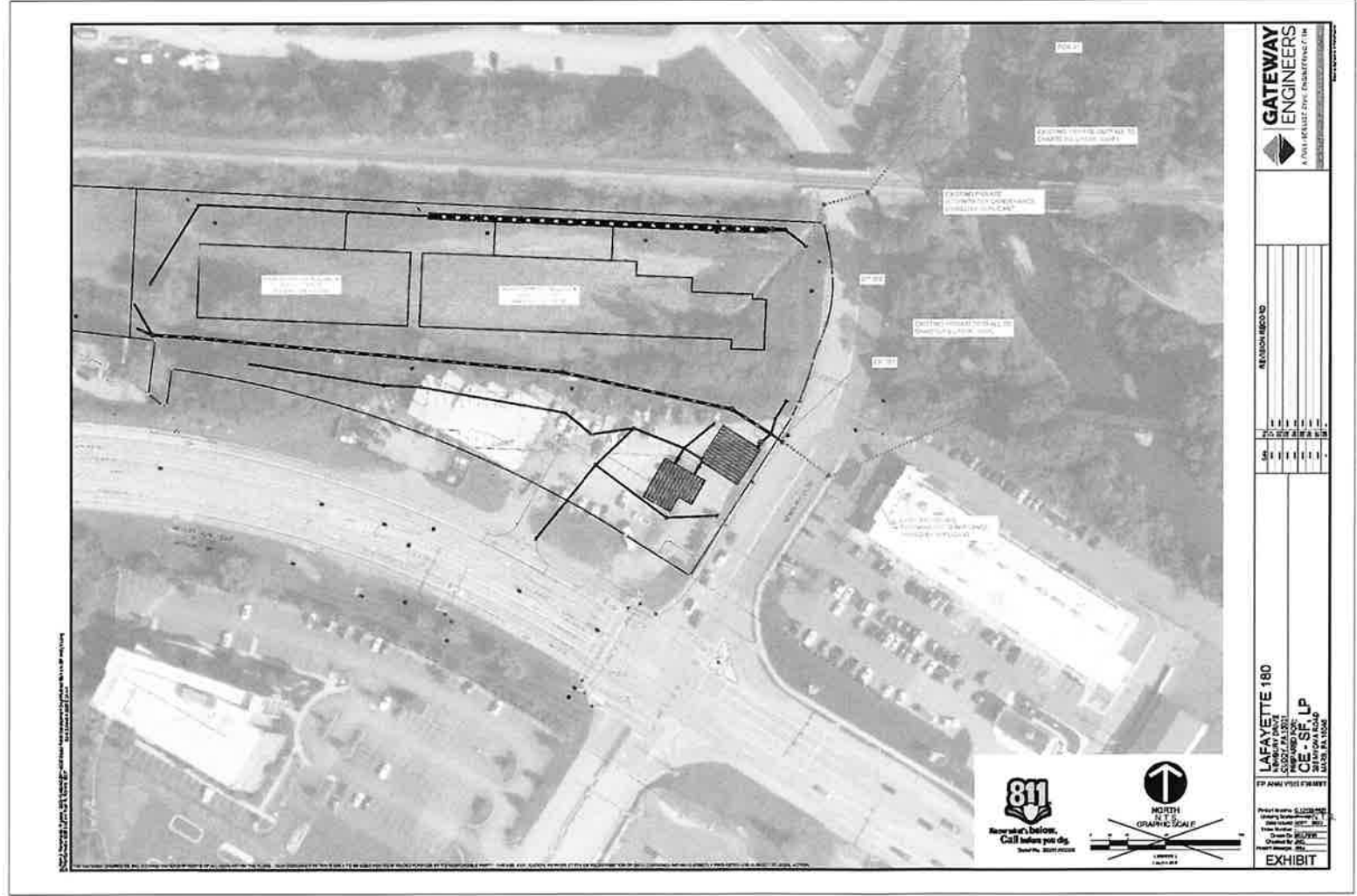
2 CONCRETE WASHOUT AREA

3 ORANGE CONSTRUCTION FENCE

SITE SOIL TYPES:				
SYMBOL	DESCRIPTION	SLOPE	LIMITATIONS	REMEDIAL ACTIONS
UB	URBAN LAND	GENTLY	<ul style="list-style-type: none"> CUTBANKS CAVE CORROSIVE TO CONCRETE AND STEEL HAZARD OF EROSION SEASONAL HIGH WATER TABLE PERMEABILITY LANDSLIDES SLOW PERCOLATION PIPING POOR SOURCES OF TOPSOIL FROST ACTION SHRINK SWELL POTENTIAL SINKHOLE SLOPE SUSCEPTIBILITY TO SLIPS 	<p>SOILS SUSCEPTIBLE TO HIGH WATER TABLES AND/OR PIPING AND SEEPING:</p> <ul style="list-style-type: none"> PROVIDE PUMPED WATER SEDIMENT REMOVAL FACILITIES USE CLAY EMBANKMENT CORES UTILIZE ANTI-SEEP COLLARS OR GRAVEL PACKS <p>SOILS SUSCEPTIBLE TO MODERATE OR HIGH EROSION POTENTIAL:</p> <ul style="list-style-type: none"> LIMIT TIME OF EXPOSURE USE EROSION CONTROL BLANKETS SELECTION OF SEED MIXTURES WITH RAPIDLY GERMINATING SPECIES SODDING USE OF SPECIAL STABILIZATION PRODUCTS (E.G. CELLULAR GRIDS, INTERLOCKING CONCRETE BLOCKS,) <p>SOILS SUSCEPTIBLE TO SLIPS & LANDSLIDES:</p> <ul style="list-style-type: none"> PREVENT SATURATION OF SLOPES PROVIDE ANCHORING OR RETAINING SYSTEMS PROVIDE BENCHING TO CATCH FALLING DEBRIS SEE GEOTECHNICAL NOTE BELOW <p>SOILS SUSCEPTIBLE TO CUTBANKS CAVE:</p> <ul style="list-style-type: none"> PREVENT SATURATION OF SLOPES PROVIDE ANCHORING OR RETAINING SYSTEMS PROVIDE BENCHING TO CATCH FALLING DEBRIS PROVIDE TRENCH BOXES FOR UTILITY INSTALLATION <p>SOILS CORROSIVE TO CONCRETE/STEEL:</p> <ul style="list-style-type: none"> MINIMIZE THE AMOUNT OF SOIL DISTURBANCE PROVIDE PROTECTIVE COATING TO CONCRETE AND STEEL PROVIDE EXTRA CONCRETE AND STEEL THICKNESS <p>SOILS THAT ARE POOR SOURCES OF TOPSOIL:</p> <ul style="list-style-type: none"> PERFORM SOIL TESTS TO DETERMINE PROPER APPLICATION OF SOIL AMENDMENTS AND PROPER MOISTURE CONTENT FOR PROPOSED VEGETATIVE COVER IMPORT TOPSOIL AS NEEDED

NOTE:
A GEOTECHNICAL ENGINEER WILL REVIEW THE SOIL CAPABILITY OF THE SITE AND MAKE RECOMMENDATIONS TO THE OWNER. SOILS HAVING UNSTABLE COMPOSITION, SLIP AND LANDSLIDE POTENTIAL ARE NOT TO BE PLACED AS FILL MATERIAL AT AREAS HAVING 2:1 SLOPES OR NEAR PROXIMITY TO SUCH SLOPES. LOADING PLANES OF SUCH PLACED FILLS SHALL BEAR ON SUITABLE SOILS KEYED INTO VIRGIN SOILS. REFER TO CUTFILL DETAILS ON PLANS. ALL CUT SITUATIONS THAT EXPOSE SUCH SOILS TO UNSUPPORTED BEARING BASE REQUIRE THOSE SOILS TO BE EXCAVATED AND REPLACED WITH SUITABLE SOILS THAT DO NOT HAVE THE MENTIONED CHARACTERISTICS.

5 SITE SOIL DATA



- NOTES:**
1. DETAIL NOT TO SCALE
 2. FOR MORE INFORMATION SEE EROSION POTENTIAL ANALYSIS MEMO INCLUDED IN NPDES SUBMITTAL.

7 EROSION POTENTIAL ANALYSIS EXHIBIT



REVISION RECORD	
No.	Date
01	2025-11-11
02	2025-12-16
03	
04	
05	
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08	

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MAARS, PA 16046

EROSION AND SEDIMENTATION CONTROL DETAILS
Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: ---
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG
C403

N. P. & E. Engineers, Inc. 12/16/2025 11:57 AM Ryan R. Rebertus, E.I.T. Sheet Date: 12/16/2025 11:57 AM

CONSTRUCTION SEQUENCE

THE LAFAYETTE ISD PROJECT WILL CONSIST OF A NEW PLANNED SHOPPING CENTER, PARKING LOTS, ACCESS DRIVES, UTILITY INSTALLATION, LANDSCAPING, AND E&S/PCSM BMPs. ALL E&S AND PCSM FACILITIES SHALL BE INSTALLED IN ACCORDANCE WITH THE APPROVED E&S/PCSM PLAN AND THE EROSION AND SEDIMENT POLLUTION CONTROL PROGRAM MANUAL DATED MARCH, 2012 OR LATEST VERSION.

A GENERALIZED CONSTRUCTION SEQUENCE IS PROVIDED BELOW. THE CONSTRUCTION SEQUENCE IS INTENDED TO PROVIDE A GENERAL COURSE OF ACTION IN ORDER TO CONFORM TO THE APPLICABLE REGULATORY AGENCY REQUIREMENTS FOR TEMPORARY AND PERMANENT SOIL EROSION AND SEDIMENT POLLUTION CONTROL. ALL NECESSARY PERMITS FOR PROPER AND COMPLETE EXECUTION OF WORK PERTAINING TO THIS PLAN, WHETHER SPECIFICALLY MENTIONED OR NOT, ARE TO BE PERFORMED BY THE CONTRACTOR. IT IS THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN ALL NECESSARY PERMITS AND TO REPORT UPON EVERY DETAILED PIECE OF MATERIAL OR EQUIPMENT. THE CONTRACTOR SHALL COMPLY WITH ALL REQUIREMENTS LISTED IN THIS SECTION. THE CONTRACTOR MAY BE REQUIRED TO ALTER CONTROLS BASED ON EFFECTIVENESS OF CONTROLS OR DIFFERING CONDITIONS ENCOUNTERED.

1. CONTRACTOR SHALL CONTACT CE-SF, LP, SOUTH FAYETTE TOWNSHIP ENGINEER, PROFESSIONAL ENGINEER, AND THE ALLEGHENY COUNTY CONSERVATION DISTRICT TO SETUP A PRE-CONSTRUCTION MEETING AT THE PROJECT SITE AT LEAST SEVEN (7) DAYS PRIOR TO THE START OF CONSTRUCTION ACTIVITIES.
2. STAKE OUT LIMITS OF DISTURBANCE FOR GRADING OPERATIONS FOR THE LATTER SITE.
3. ORANGE CONSTRUCTION FENCE SHALL BE PLACED AROUND ANY AND ALL ENVIRONMENTAL FEATURES TO BE PROTECTED ON THE SITE.
4. INSTALL THE ROCK CONSTRUCTION ENTRANCE OFF NEWBURY DRIVE AT THE LOCATION SHOWN ON THE PLAN DRAWINGS AND IN ACCORDANCE WITH THE CONSTRUCTION DETAIL.
5. CONCRETE WASHOUT CAN BE INSTALLED AS SHOWN ON PLAN. WASHOUT AREA MUST BE INSTALLED PRIOR TO POURING ANY CONCRETE AT THE SITE.
6. CLEAR AND GRUB AND INSTALL ALL COMPOST FILTER SOCKS IN THE LOCATIONS SHOWN ON THE PLAN AND IN ACCORDANCE WITH THE DETAILS PROVIDED AND CLEAR AND GRUB ONLY THE AREA NECESSARY TO INSTALL PERMANENT E&S CONTROLS. INSTALL INLET PROTECTION AT EXISTING INLETS ON THE PROPERTY.
7. ONCE ALL CONTROLS ARE INSTALLED AS DETAILED ABOVE, DEPOSITION OF THE EXISTING APPURTENANCES MAY COMMENCE. ALL CONSTRUCTION DEBRIS SHALL BE REMOVED BY THE CONTRACTOR AND DISPOSED OF AT A DEP APPROVED WASTE SITE IN ACCORDANCE WITH ALL LOCAL AND STATE CODES AND PERMIT REQUIREMENTS. IF A BAMP FAILS DURING A RUNOFF EVENT, THE CONTRACTOR SHALL REPLACE/FIX THE DAMAGED BAMP WITHIN 24 HOURS FOLLOWING THE FAILURE OCCURRENCE.
8. BEGIN CUT AND FILL OPERATIONS FOR THE SITE. DURING GRADING OPERATIONS, THE FILL SLOPES SHALL BE GRADED TO PROVIDE POSITIVE DRAINAGE AND PREVENT PONDING WATER AT THE END OF EACH DAY. AS SOON AS THE PROPOSED FILL SLOPES ARE BROUGHT TO FINAL GRADE, IMMEDIATELY PLACE TOPSOIL, SEED AND MULCH IN REGULAR 1' VERTICAL INCREMENTS TO PROMOTE EARLY STABILIZATION. NOTE: INSTALL EROSION CONTROL BLANKETING ON THE SLOPES GREATER THAN 3% HORIZONTAL TO 1 VERTICAL (3:1).
9. BEGIN CONSTRUCTION OF PROPOSED STORM STRUCTURES, STORM SEWERS, AND UNDERGROUND TANKS 1 & 2. AS EACH NEW INLET IS INSTALLED, IMMEDIATELY INSTALL INLET PROTECTION IN THE NEW STRUCTURE, AS INDICATED ON THE PCSM PLAN (C700) AND SHOWN ON THE CONSTRUCTION DETAILS (C202-C704). REFER TO UTILITY LINE INSTALLATION REQUIREMENTS BELOW FOR STORM SEWER INSTALLATION. A LICENSED PROFESSIONAL ENGINEER SHALL BE CONTACTED AND PRESENT ON SITE TO OVERSEE THE INSTALLATION OF THE UNDERGROUND TANKS, WHICH IS A CRITICAL STAGE OF CONSTRUCTION. NOTICE MUST BE GIVEN AT LEAST 3 DAYS PRIOR TO CONSTRUCTION OF THE TRENCH. REFER TO THE SEQUENCE BELOW FOR UNDERGROUND TANK INSTALLATION:
 - 9.1. PROTECT TRENCH AREA FROM COMPACTION PRIOR TO INSTALLATION WITH ORANGE CONSTRUCTION FENCE.
 - 9.2. AFTER INSTALLATION, PREVENT SEDIMENT LADEN WATER FROM ENTERING INLETS AND PIPES BY INSTALLING INLET PROTECTION.
 - 9.3. INSTALL AND MAINTAIN PROPER EROSION AND SEDIMENT CONTROL MEASURES, SUCH AS COMPOST FILTER SOCK, DURING CONSTRUCTION TO PREVENT SEDIMENT LADEN WATER FROM ENTERING THE TRENCH AREAS.
 - 9.4. EXCAVATE TRENCH BOTTOM TO A UNIFORM, LEVEL UN-COMPACTED SUBGRADE FREE FROM ROCKS AND DEBRIS. DO NOT COMPACT SUBGRADE.
 - 9.5. PLACE NONWOVEN GEOTEXTILE ALONG BOTTOM AND SIDES OF TRENCH. NONWOVEN GEOTEXTILE ROLLS SHOULD OVERLAP BY A MINIMUM OF 16 INCHES WITHIN THE TRENCH. FOLD BACK AND SECURE EXCESS GEOTEXTILE DURING STONE PLACEMENT.
 - 9.6. INSTALL UPSTREAM AND DOWNSTREAM CONTROL STRUCTURES, CLEANOUTS, ETC.
 - 9.7. PLACE UNIFORM GRADE, CLEAN-WASHED AGGREGATE IN 8-INCH LIFTS, LIGHTLY COMPACTING BETWEEN LIFTS.
 - 9.8. INSTALL CONTINUOUS PERFORATED PIPE AS INDICATED ON PLANS. BACKFILL WITH UNIFORM GRADE, CLEAN-WASHED AGGREGATE IN 8-INCH LIFTS, LIGHTLY COMPACTING BETWEEN LIFTS.
 - 9.9. FOLD AND SECURE NONWOVEN GEOTEXTILE OVER TRENCH, WITH MINIMUM OVERLAP OF 16-INCHES.
 - 9.10. PLACE 6-INCH LIFT OF APPROVED BACKFILL MATERIAL OVER TRENCH UNTIL SUBGRADE OF THE PAVING AREA IS MET, AS INDICATED ON PLANS.
 - 9.11. DO NOT REMOVE INLET PROTECTION ON OTHER EROSION AND SEDIMENT CONTROL MEASURES UNTIL SITE IS FULLY STABILIZED.
 - 9.12. ANY SEDIMENT THAT ENTERS INLETS DURING CONSTRUCTION IS TO BE REMOVED WITHIN 24 HOURS.
10. ONCE CONCRETE WASHOUT AREA HAS BEEN INSTALLED, BEGIN CONSTRUCTION OF THE PROPOSED PLANNED SHOPPING CENTER.
11. INSTALL SITE SERVICE UTILITIES (SEWER, GAS, WATER, AND ELECTRIC) AS SHOWN ON THE PLAN. REFER TO THE UTILITY LINE INSTALLATION REQUIREMENTS BELOW.
12. COMPLETE REMAINDER OF FINE GRADING OPERATIONS IN PREPARATION OF SITE PAVING. SITE PAVING WILL INCLUDE ALL PARKING AREAS AND ROADWAYS AROUND THE PROPOSED PLANNED SHOPPING CENTER. BEGIN PAVING OPERATIONS, INSTALLATION OF SITE FURNISHINGS, CONSTRUCTION OF SIDEWALKS, AND LANDSCAPING AROUND SHOPPING CENTER.
13. REMOVE THE ROCK CONSTRUCTION ENTRANCE AS PAVING IS COMPLETED.
14. SPREAD TOPSOIL IN NON-PAVED AREAS FOR SEEDING. SEED AND MULCH ALL DISTURBED AREAS, USING THE SPECIFIED SEEDING REQUIREMENTS. ALL DISTURBED AREAS MUST BE TEMPORARILY STABILIZED IF REMAINING BARE OR ANTICIPATED TO REMAIN BARE. NO MORE THAN 15,000 SQUARE FEET OF DISTURBED AREA MAY BEACH FINAL GRADE BEFORE INITIATING SEEDING AND MULCHING OPERATIONS.
15. RESEED ALL DISTURBED AREAS IF 70% PERENNIAL VEGETATIVE COVER IS NOT ESTABLISHED AFTER 30 DAYS.
16. REMOVE ALL COMPOST FILTER SOCK AND INLET PROTECTION FOLLOWING COMPLETION OF THE ABOVE STEPS AND AFTER THE SITE HAS ACHIEVED A UNIFORM 70% PERENNIAL VEGETATIVE COVER ON UNPAVED AREAS.
17. UPON COMPLETION OF THE PROJECT, THE CONTRACTOR SHALL COMPLETE A NOTICE OF TERMINATION (FORM 3800-PM-BCW0228) AND PROVIDE ALL REQUIRED SUPPORTING DOCUMENTS.

UTILITY LINE INSTALLATION REQUIREMENTS

1. CONTRACTOR IS REQUIRED TO NOTIFY AN ENVIRONMENTAL PROFESSIONAL (EP) PRIOR TO ANY EXCAVATION AND HAVE THEM ON-SITE TO OVERSEE ALL EXCAVATION AND HANDLING OF MATERIAL.
2. WORK CREWS AND EQUIPMENT FOR TRENCHING, PLACEMENT OF PIPE, PLUG INSTALLATION AND BACKFILLING WILL BE SELF-CONTAINED AND SEPARATE FROM CLEARING AND GRUBBING AND SITE RESTORATION AND STABILIZATION OPERATIONS.
3. DAILY TRENCH EXCAVATION SHALL BE LIMITED TO THE LENGTH OF PIPE PLACEMENT. PLUG INSTALLATION AND BACKFILLING THAT CAN BE COMPLETED THE SAME DAY.
4. WATER WHICH ACCUMULATES IN THE OPEN TRENCH SHALL BE MANAGED UNDER THE SUPERVISION OF THE ENVIRONMENTAL PROFESSIONAL ACCORDING TO THE GUIDELINES SPECIFIED BY THE SOIL MANAGEMENT/MATERIAL MANAGEMENT PLAN REFERENCED IN DETAIL C404.
5. ON THE DAY FOLLOWING PIPE PLACEMENT AND TRENCH BACKFILLING, THE DISTURBED AREA SHALL BE GRADED TO FINAL CONTIGUOUS AND APPROPRIATE TEMPORARY EROSION AND SEDIMENT POLLUTION CONTROL MEASURES/FACILITIES WILL BE INSTALLED. SEEDING AND MULCHING OF ALL DISTURBED AREAS WILL BE DONE AT THE END OF EACH WEEK.

1	C404	CONSTRUCTION SEQUENCE
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MANAGEMENT OF SOIL

BECAUSE OF THE CONSTITUENTS OF CONCERN (COCs) DETECTED IN SOIL SAMPLES COLLECTED AT THE SITE, EXCAVATED SOIL THAT IS DETERMINED TO BE POTENTIALLY IMPACTED, WHETHER DISPOSED OFF-SITE OR USED AS CUT AND FILL MATERIAL, SHOULD BE HANDLED IN ACCORDANCE WITH THE FOLLOWING PROTOCOL. THE EXISTING GRADING PLAN DOES NOT CALL FOR LARGE SCALE CUTTING AND FILLING ACTIVITIES. IN FACT, THE CONSTRUCTION PLAN CALLS FOR THE IMPORTATION OF LARGE AMOUNTS OF CLEAN FILL MATERIAL TO BE IMPORTED TO THE SITE TO RAISE THE GRADE OF THE ENTIRE SITE. HOWEVER, IF ANY SOIL IS EXCAVATED AT THE SITE, SPECIFIC STEPS MUST BE CONDUCTED TO ENSURE THAT ANY SOIL REMOVED FROM THE SITE IS MANAGED PROPERLY. THE FOLLOWING ARE THE REQUIRED ACTIONS THAT MUST BE TAKEN IF SOILS ARE EXCAVATED FOR THE INSTALLATION OF STORMWATER INFRASTRUCTURE, UTILITIES, FOOTER CONSTRUCTION OR ANY OTHER REASON OR IF CONTAMINATED SOIL IS UNEXPECTEDLY ENCOUNTERED ON ANY PART OF THE SITE DURING CONSTRUCTION ACTIVITIES.

ENVIRONMENTAL OVERSIGHT

IF EXCAVATION WORK IS PERFORMED, AN ENVIRONMENTAL PROFESSIONAL (EP) SHOULD BE NOTIFIED PRIOR TO THE INITIATION OF EXCAVATION ACTIVITIES. THE ROLE OF THE EP IS TO ASSESS THE PROPOSED EXCAVATION TO ENSURE THAT IF IMPACTED SOIL IS ENCOUNTERED, IT IS HANDLED PROPERLY. THE DETERMINATION OF IMPACTED SOIL WILL BE BASED PRIMARILY ON THE DEPTH AND LOCATION OF THE EXCAVATION TO DETERMINE IF THE EXCAVATION WILL BE PERFORMED SOLELY IN THE RECENTLY PLACED SHALLOW CLEAN FILL MATERIAL OR IF IT WILL EXTEND DOWN INTO THE EXISTING SOIL. IF THE EXCAVATION IS DETERMINED TO BE ONLY IN THE CLEAN FILL MATERIAL, NO FURTHER OVERSIGHT WILL BE REQUIRED. IF THE ENVIRONMENTAL PROFESSIONAL DETERMINES THAT THE EXCAVATION IS DEEP ENOUGH, AND IN THE AREA OF THE OBSERVED ARSENIC EXCEEDANCE (AREA OF CONCERN #3), ADDITIONAL OVERSIGHT WILL BE REQUIRED, AND THE EP MUST BE PRESENT AT THE TIME OF EXCAVATION. THE FINAL DETERMINATION OF POTENTIALLY IMPACTED MATERIAL WILL BE BASED UPON VISUAL AND OLFACTORY CHARACTERISTICS AS WELL AS HEADSPACE READINGS OBTAINED USING A PHOTOIONIZATION DETECTOR (PID).

HANDLING OF CONTAMINATED SOIL

ONCE POTENTIALLY CONTAMINATED SOIL HAS BEEN REMOVED FROM THE GROUND, IT MUST BE HANDLED AS RESIDUAL WASTE. THIS WASTE MATERIAL MUST BE HANDLED IN A WAY THAT MINIMIZES THE POTENTIAL FOR CONTAMINATION MIGRATION. CONTAMINATED SOIL THAT HAS BEEN REMOVED FROM THE GROUND MUST NOT COME IN CONTACT WITH OTHER SOILS ON THE SITE. THEREFORE, THIS SOIL MUST BE STOCKPILED ON PLASTIC. THE PLASTIC MUST BE A MINIMUM OF 6-MIL IN THICKNESS AND IT MUST BE PLACED IN AN AREA THAT IS NOT PRONE TO SIGNIFICANT AMOUNTS OF SURFACE WATER RUNOFF.

SOIL STOCKPILE MANAGEMENT

CONTAMINATED SOILS MUST BE STORED IN ACCORDANCE WITH THE APPLICABLE SECTIONS OF 25 PA CODE §299.101-299.154 OF THE WASTE MANAGEMENT REGULATIONS. IN ADDITION TO THE GENERAL REQUIREMENTS SET FORTH IN 299.131, THE CONTAMINATED SOIL PILES WILL BE COMPLETELY AND SECURELY COVERED, FOR THE DURATION OF THE STORAGE PERIOD WITH AN IMPERMEABLE MATERIAL OF SUFFICIENT STRENGTH, THICKNESS, ANCHORING OR WEIGHTING TO PREVENT TEARING OR LIFTING OF THE COVER, INFILTRATION OF PRECIPITATION OR SURFACE WATER RUN-OFF, AND EXPOSURE OF THE SOIL TO THE ATMOSPHERE. STEPS MUST ALSO BE TAKEN TO DETER PUBLIC ACCESS TO THE STORAGE PILES. THIS MAY INCLUDE FENCING, SIMILAR BARRIERS, SECURITY PATROLS OR WARNING SIGNS.

DISPOSAL OF SOIL

PRIOR TO DISPOSAL OF POTENTIALLY CONTAMINATED SOIL, AN EVALUATION MUST BE CONDUCTED TO DETERMINE WHETHER THE SOIL CAN BE USED AS CLEAN FILL OR IF IT MUST BE MANAGED AS REGULATED FILL AS PER THE PADEP MANAGEMENT OF FILL POLICY, DATED JANUARY 16, 2021 (DOCUMENT # 25B-2182-773).

IN THE MANAGEMENT OF FILL POLICY DOCUMENT "CLEAN FILL" IS DEFINED AS "UNCONTAMINATED, NON-WATER SOLUBLE, NON-DECOMPOSABLE, INERT SOLID MATERIAL USED TO LEVEL AND AREA OR BRING AN AREA TO A SPECIFIED GRADE." TO BE CONSIDERED "CLEAN FILL" THE MATERIAL MUST NOT EXCEED THE NUMERIC VALUES SPECIFIED IN TABLE 3 (MEDIUM SPECIFIC CONCENTRATIONS (MSCS) FOR ORGANIC REGULATED SUBSTANCES IN SOIL) AND TABLE 4 (MSCS FOR INORGANIC REGULATED SUBSTANCES IN SOIL OF APPENDIX A IN 25 PA. CODE CHAPTER 250 (RELATED TO ADMINISTRATION OF LAND RECYCLING PROGRAM)). THE APPLICABLE NUMERIC CLEAN FILL CONCENTRATION LIMIT IS DETERMINED BY COMPARISON OF THE DETECTED CONTAMINANT CONCENTRATIONS WITH THE LOWER OF THE GENERICS SOIL TO GROUNDWATER VALUE MSC OR THE DIRECT CONTACT RESIDENTIAL MSC.

PURSUANT TO 25 PA. CODE 271.2 (B)(2) AND 287.101(B)(2), USE OF CLEAN FILL DOES NOT REQUIRE A PERMIT UNDER THE SWMA OR THE MUNICIPAL OR RESIDUAL WASTE REGULATIONS. CLEAN FILL MAY BE USED IN ACCORDANCE WITH ALL APPLICABLE REQUIREMENTS GOVERNING THE PLACEMENT OR USE OF CLEAN FILL, INCLUDING 25 PA. CODE CHAPTER 102 (RELATING TO EROSION AND SEDIMENT CONTROL) AND 25 PA. CODE CHAPTER 103 (RELATING TO DAM SAFETY AND WATERWAY MANAGEMENT). PERSONS USING CLEAN FILL MUST ALSO COMPLY WITH THE FUGITIVE EMISSIONS REGULATIONS UNDER 25 PA. CODE, CHAPTER 123 (RELATING TO STANDARDS FOR CONTAMINANTS) ISSUED UNDER THE AIR POLLUTION CONTROL ACT. THE USE OF CLEAN FILL MAY BE REGULATED UNDER OTHER ENVIRONMENTAL LAWS AND REGULATIONS.

THEFORE, THE CLEAN FILL ELIGIBILITY DETERMINATION MAY INCLUDE, AMONG OTHER THINGS, ANALYZING SAMPLES FOR THE PARAMETERS LISTED IN TABLE 1 OF THE GUIDANCE TO DETERMINE IF THE FILL CONTAINS CONTAMINANT CONCENTRATIONS WHICH EXCEED THE CFCLS, WHICH THE GUIDANCE DEFINES AS WITH THE EXCEPTION OF PCBs AND CHLORIDE, THE CONCENTRATIONS OF REGULATED SUBSTANCES THAT DO NOT EXCEED THE NUMERIC VALUES SPECIFIED IN TABLE 3 (MEDIUM SPECIFIC CONCENTRATIONS (MSCS) FOR ORGANIC REGULATED SUBSTANCES IN SOIL) AND TABLE 4 (MEDIUM-SPECIFIC CONCENTRATIONS (MSCS) FOR INORGANIC REGULATED SUBSTANCES IN SOIL) / JANUARY 16, 2021 / PAGE 111 CHAPTER 250 (RELATING TO ADMINISTRATION OF LAND RECYCLING PROGRAM).

IF THE SOIL DOES NOT MEET THE DEFINITION OF CLEAN FILL OR IF THE CONCENTRATIONS EXCEED THE CFCLS, THE MATERIAL MUST BE MANAGED IN ACCORDANCE WITH THE DEPARTMENT'S MUNICIPAL OR RESIDUAL WASTE REGULATIONS, 25 PA. CODE § 271.2 AND § 287.2.

REGULATED FILL IS DEFINED AS FILL MATERIAL THAT HAS BEEN AFFECTED BY A RELEASE OF A REGULATED SUBSTANCE AND IS NOT "UNCONTAMINATED MATERIAL" AS DEFINED IN THE MANAGEMENT OF FILL POLICY DOCUMENT. FILL THAT IS DETERMINED TO BE "REGULATED FILL" CAN BE USED BY PERSONS WHO HAVE APPLIED FOR AND OBTAINED COVERAGE UNDER THE PADEP'S GENERAL PERMIT NO. WNGR096. BENEFICIAL USE OF REGULATED FILL, OTHERWISE, THE MATERIAL WILL NEED TO BE DISPOSED OF IN A PADEP APPROVED LANDFILL, FOLLOWING THE RESPECTIVE LANDFILL PERMITTING CONDITIONS. IN ADDITION, THE WASTE TRANSPORTATION SAFETY ACT (ACT 90 OF 2002, CHAPTER 90) REQUIRES ALL WASTE TRANSPORTATION VEHICLES (TRUCKS AND TRUCK TRACTORS WITH A REGISTERED GROSS VEHICLE WEIGHT GREATER THAN 12,000 LBS., AND TRAILERS WITH A REGISTERED GROSS VEHICLE WEIGHT GREATER THAN 10,000 LBS.) TRANSPORTING MUNICIPAL OR RESIDUAL WASTE TO WASTE PROCESSING OR DISPOSAL FACILITIES IN PENNSYLVANIA TO HAVE A VALID WASTE TRANSPORTER AUTHORIZATION. THEREFORE, ACT 90 CERTIFIED TRUCKS WOULD BE REQUIRED TO TRANSPORT THE REGULATED FILL MATERIAL.

MANAGEMENT OF CONTAMINATED GROUNDWATER

BECAUSE OF THE IDENTIFICATION OF CONSTITUENTS OF CONCERN (COCs) DETECTED IN GROUNDWATER SAMPLES COLLECTED AT THE SITE, ALL GROUNDWATER BROUGHT TO THE SURFACE DURING DEWATERING ACTIVITIES SHOULD BE HANDLED IN ACCORDANCE WITH THE FOLLOWING PROTOCOL. THE EXISTING GRADING PLAN DOES NOT CALL FOR LARGE SCALE CUTTING AND FILLING ACTIVITIES. IN FACT, THE CONSTRUCTION PLAN CALLS FOR THE IMPORTATION OF LARGE AMOUNTS OF CLEAN FILL MATERIAL TO BE IMPORTED TO THE SITE TO RAISE THE GRADE OF THE ENTIRE SITE. THEREFORE, IT IS UNLIKELY THAT GROUNDWATER WILL BE ENCOUNTERED. HOWEVER, IF ANY GROUNDWATER IS BROUGHT TO THE SURFACE, SPECIFIC STEPS MUST BE CONDUCTED TO ENSURE THAT ANY GROUNDWATER REMOVED FROM THE SITE IS MANAGED PROPERLY. THE FOLLOWING SECTIONS DETAIL THE REQUIRED ACTIONS THAT MUST BE TAKEN IF GROUNDWATER IS BROUGHT TO THE SURFACE FOR THE INSTALLATION OF STORMWATER MANAGEMENT INFRASTRUCTURE, UTILITIES, FOOTER CONSTRUCTION OR ANY OTHER REASON OR IF CONTAMINATED GROUNDWATER IS UNEXPECTEDLY ENCOUNTERED ON ANY PART OF THE PROPERTY DURING CONSTRUCTION ACTIVITIES.

ENVIRONMENTAL OVERSIGHT

IF DEWATERING WORK IS PERFORMED, THE ENVIRONMENTAL PROFESSIONAL (EP) SHOULD BE NOTIFIED PRIOR TO THE INITIATION OF DEWATERING ACTIVITIES. THE ROLE OF THE EP IS TO ASSESS THE GROUNDWATER TO DETERMINE WHETHER THE GROUNDWATER IS IMPACTED AND TO ENSURE THAT IF IMPACTED GROUNDWATER IS ENCOUNTERED, IT IS HANDLED PROPERLY. THE DETERMINATION OF IMPACTED GROUNDWATER WILL BE BASED UPON VISUAL, I.E., IDENTIFICATION OF A SHEEN, AND OLFACTORY OBSERVATIONS. IN ADDITION, A GRAB SAMPLE OF THE COLLECTED GROUNDWATER SHALL BE COLLECTED FOR WASTE CHARACTERIZATION PURPOSES.

HANDLING OF CONTAMINATED GROUNDWATER

ONCE CONTAMINATED GROUNDWATER HAS BEEN REMOVED FROM THE GROUND, IT MUST BE HANDLED PROPERLY. THIS WASTE MATERIAL MUST BE HANDLED IN A WAY THAT MINIMIZES THE POTENTIAL FOR CONTAMINATION MIGRATION. CONTAMINATED GROUNDWATER THAT HAS BEEN REMOVED FROM THE GROUND MUST NOT COME IN CONTACT WITH SITE SOILS, GROUNDWATER OR SURFACE WATER ON THE SITE. THEREFORE, THE COLLECTED GROUNDWATER MUST BE STORED IN A WATER-TIGHT CONTAINER. WHILE CONTAINING CONTAMINATED GROUNDWATER, THE COLLECTION TANK SHOULD BE INSPECTED DAILY FOR EVIDENCE OF LEAKS. THE CONTENTS OF THE COLLECTION TANK MUST BE PROPERLY PROFILED BEFORE EMPTYING THE TANK INTO A TANKER TRUCK FOR OFF-SITE DISPOSAL OR RECYCLING AT A LICENSED FACILITY.

AS DISCUSSED ABOVE, THE WASTE TRANSPORTATION SAFETY ACT (ACT 90 OF 2002, CHAPTER 90) REQUIRES ALL WASTE TRANSPORTATION VEHICLES (TRUCKS AND TRUCK TRACTORS WITH A REGISTERED GROSS VEHICLE WEIGHT GREATER THAN 12,000 LBS., AND TRAILERS WITH A REGISTERED GROSS VEHICLE WEIGHT GREATER THAN 10,000 LBS.) TRANSPORTING MUNICIPAL OR RESIDUAL WASTE TO WASTE PROCESSING OR DISPOSAL FACILITIES IN PENNSYLVANIA TO HAVE A VALID WASTE TRANSPORTER AUTHORIZATION. THEREFORE, ACT 90 CERTIFIED TRUCKS WOULD BE REQUIRED TO TRANSPORT THE IMPACTED GROUNDWATER AS WELL. IN ADDITION, THE WATER WOULD NEED TO BE DISPOSED OF AT A PERMITTED DISPOSAL FACILITY FOLLOWING ALL APPLICABLE PERMIT REQUIRED CHARACTERIZATION/APPROVAL REQUIREMENTS.

HEALTH & SAFETY PLAN

BECAUSE THERE WAS NOT A SINGLE DETECTION OF ANY COMPOUND IN ANY OF THE 36 SOIL SAMPLES DETECTED ABOVE THE NON-RESIDENTIAL DIRECT CONTACT MSCS, THE RISK OF DERMAL CONTACT OR INCIDENTAL INGESTION IS CONSIDERED EXTREMELY MINIMAL. THEREFORE, A HEALTH & SAFETY PLAN IS NOT REQUIRED FOR THE RISK OF DERMAL CONTACT AND INCIDENTAL INGESTION OF CONTAMINATED SOIL FOR CONSTRUCTION WORKERS.

IN ADDITION, THE ONLY EXCEEDANCE OF GROUNDWATER MSC WAS THE DETECTION OF MANGANESE IN TWO OF THE GROUNDWATER SAMPLES COLLECTED DURING THE 2021 PHASE B ESA. THE RESPECTIVE MSC IN THIS CASE, WAS ESTABLISHED BY THE PADEP BASED UPON THE LIFETIME HEALTH ADVISORY LEVEL, WHICH IS A GUIDELINE ISSUED BY THE USEPA FOR THE LEVEL OF A DRINKING WATER CONTAMINANT BELOW WHICH ADVERSE HEALTH EFFECTS ARE NOT ANTICIPATED OVER A PERSON'S LIFETIME. THEREFORE, THIS MSC WAS BASED ON THE RISK OF UTILIZING A PARTICULAR WATER SOURCE AS A PRIMARY DRINKING SOURCE OVER A STANDARD LIFETIME OF A PERSON. THE MSC IS NOT AN INDICATOR OF RISK FOR DERMAL EXPOSURE TO A CONSTRUCTION WORKER IN A SHORT-TERM CONSTRUCTION PROJECT. COUPLED WITH THE FACT THAT THE WATER TABLE WILL MOST LIKELY NOT BE ENCOUNTERED DURING CONSTRUCTION ACTIVITIES, THERE IS NO NEED FOR A SPECIFIC HEALTH AND SAFETY PLAN FOR THE POTENTIAL EXPOSURE OF MANGANESE IN GROUNDWATER.

HOWEVER, A PROJECT-SPECIFIC HEALTH & SAFETY PLAN SHOULD BE PREPARED BY EACH CONTRACTOR TO ENSURE THE GENERAL SAFETY OF THE CONSTRUCTION WORKERS. A SITE-SPECIFIC HEALTH AND SAFETY PLAN (HASP) WILL BE PREPARED BY THE CONTRACTOR AND SUBCONTRACTORS FOR USE BY ITS PERSONNEL AT THE SUBJECT PROPERTY, WHICH WILL DETAIL HEALTH AND SAFETY PROTOCOLS TO BE FOLLOWED DURING CONSTRUCTION ACTIVITIES AT THE SITE. HASPs THAT INCLUDE PERSONNEL TRAINING RECORDS MUST BE PROVIDED TO THE OWNER'S REPRESENTATIVE AT LEAST ONE WEEK PRIOR TO THE COMMENCEMENT OF ACTIVITIES AT THE SUBJECT PROPERTY AND MUST BE MAINTAINED ON-SITE FOR THE DURATION THE CONTRACTOR OR SUBCONTRACTOR IS PERFORMING THESE ACTIVITIES.

ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE OSHA STANDARDS AND OSHA WORKER SAFETY REQUIREMENTS. IN ACCORDANCE WITH PENNSYLVANIA LAW, PERSONNEL CONDUCTING EARTHWORK ACTIVITIES AT THE SUBJECT PROPERTY SHOULD, AT A MINIMUM, RECEIVE OSHA 30-HOUR AND 20-HOUR TRAINING PRIOR TO PERFORMING SITE WORK.

BASED ON THE KNOWN CONTAMINANT CONCENTRATIONS, SITE WORKERS SHOULD WEAR LEVEL D PERSONAL PROTECTIVE EQUIPMENT (PPE), WHICH GENERALLY INCLUDES SAFETY GLASSES, GLOVES, BOOTS, AND A HARD HAT, IF NECESSARY. PPE LEVELS SHOULD BE RECONSIDERED AND MODIFIED DEPENDING ON THE CONDITIONS ENCOUNTERED AS THE PROJECT PROGRESSES. THE CONTRACTOR AND ITS SUBCONTRACTORS ARE RESPONSIBLE FOR MEETING AND ADHERING TO ALL APPLICABLE OSHA REQUIREMENTS.

2	C404	SOIL MANAGEMENT PLAN AND MATERIALS MANAGEMENT PLAN PROCEDURES
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REVISION RECORD

No.	Date	BY	DESCRIPTION
01	2025-11-11	MPP/Upd/aba	ADD TECHNICAL NPDES RESPONSE
02	2025-12-16	---	---
03	---	---	---
04	---	---	---
05	---	---	---
06	---	---	---
07	---	---	---
08	---	---	---

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031

PREPARED FOR:
CE - SF, LP
 295 MYOMMA ROAD
 MARKS, PA 16046

EROSION AND SEDIMENTATION CONTROL DETAILS

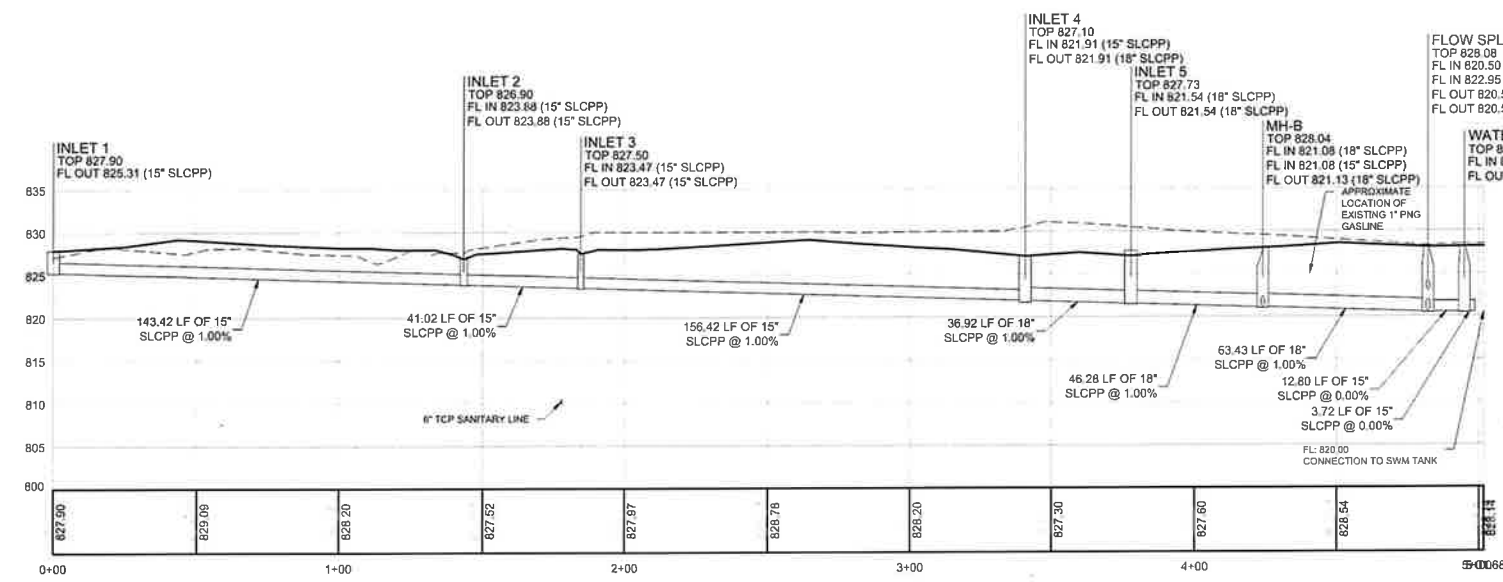
Project Number: C-12159-0025
 Drawing Scale: N/A
 Date Issued: AUG 2025
 Index Number: --
 Drawn By: MCL/RRR
 Checked By: JMG
 Project Manager: JMG

C404

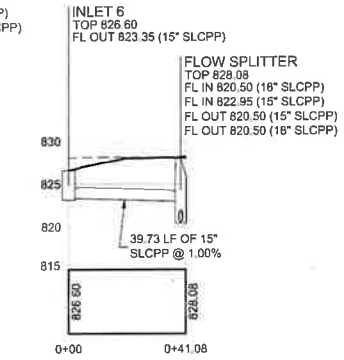
PROFILE NOTES:
 1. THE LENGTHS (L) SHOWN ON THIS PLAN ARE CENTERLINE STRUCTURE TO CENTERLINE STRUCTURE AND DO NOT ACCOUNT FOR SLOPES OR DEFINE ACTUAL LENGTHS OF PIPE.

PROFILE STRUCTURE ABBREVIATIONS

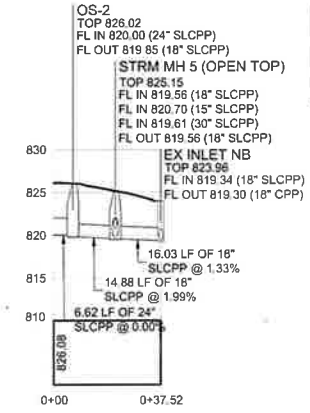
STM	STORM
SM	SANITARY
MH	MANHOLE
OS	OUTLET STRUCTURE
CO	CLEANOUT
HW	HEADWALL
EW	ENDWALL



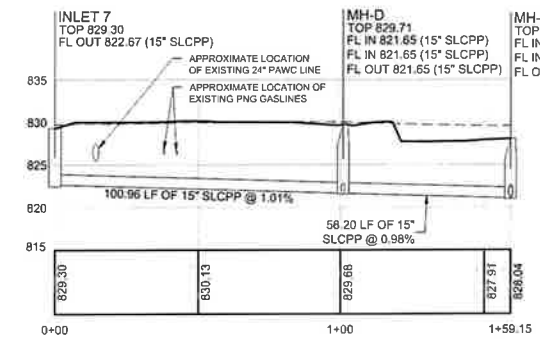
INLET 1 TO TANK PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



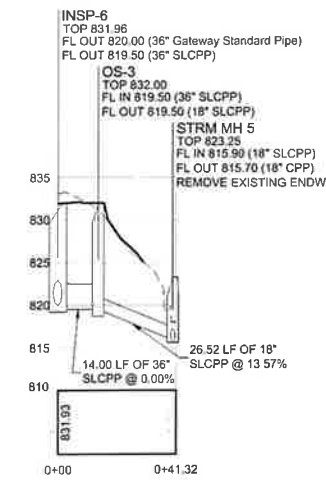
INLET 6 TO FLOW SPLITTER PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



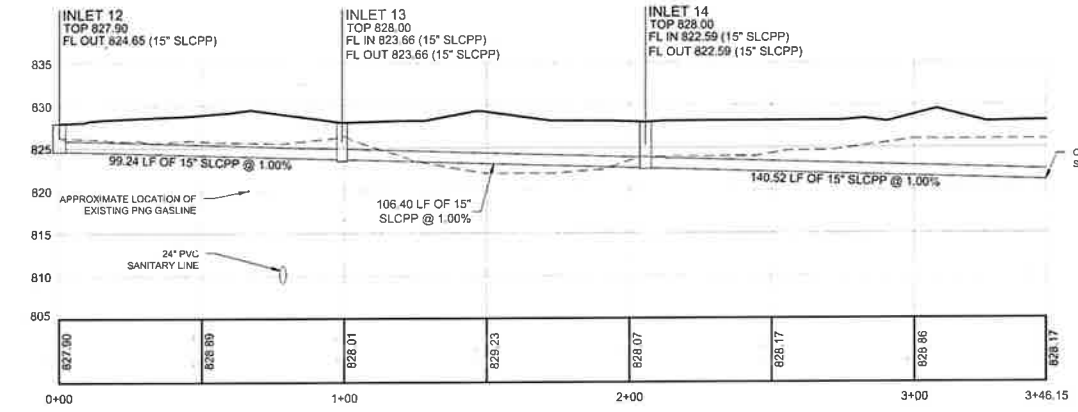
INSP-4 TO EX INLET NB PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



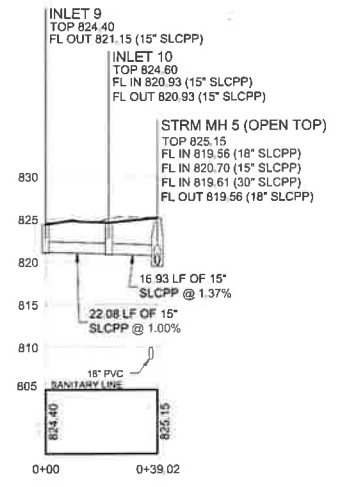
INLET 7 TO MH-B PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



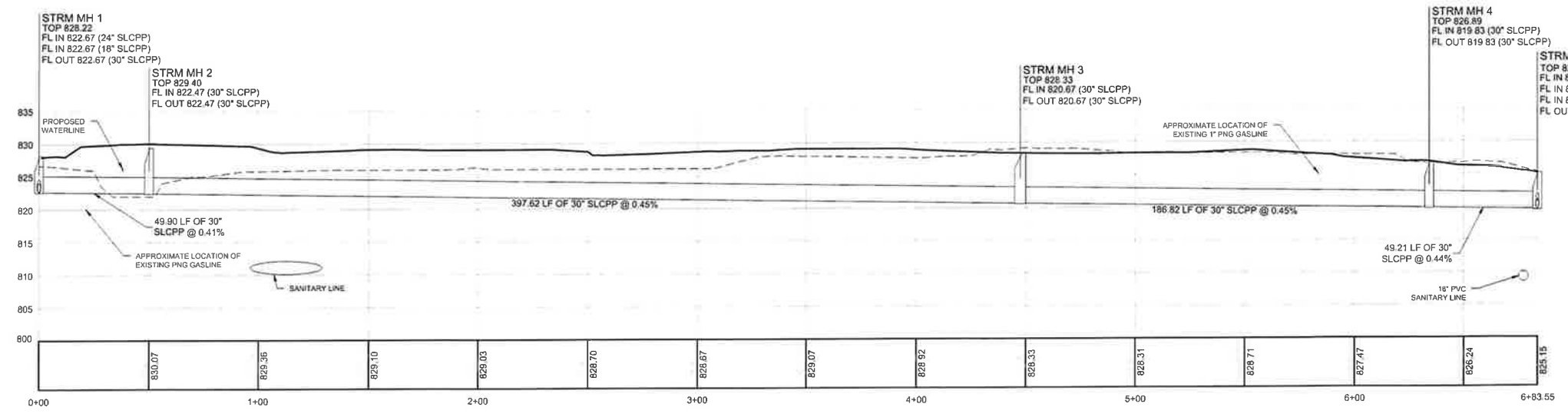
TANK 2 TO MH-5 PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



INLET 12 TO TANK 2 PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



INLET 9 TO STRM MH-5 PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'



STRM MH 1 TO INLET 12 (TYPE 4) PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'

DATE	NO.	DESCRIPTION
2025-11-11	01	WPP Updates
2025-12-16	02	ACCD TECHNICAL RESPONSE
2026-01-14	03	TOWNSHIP SUBMISSION

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
 PREPARED FOR:
CE-SF, LP
 295 MYOMA ROAD
 MARS, PA 16046

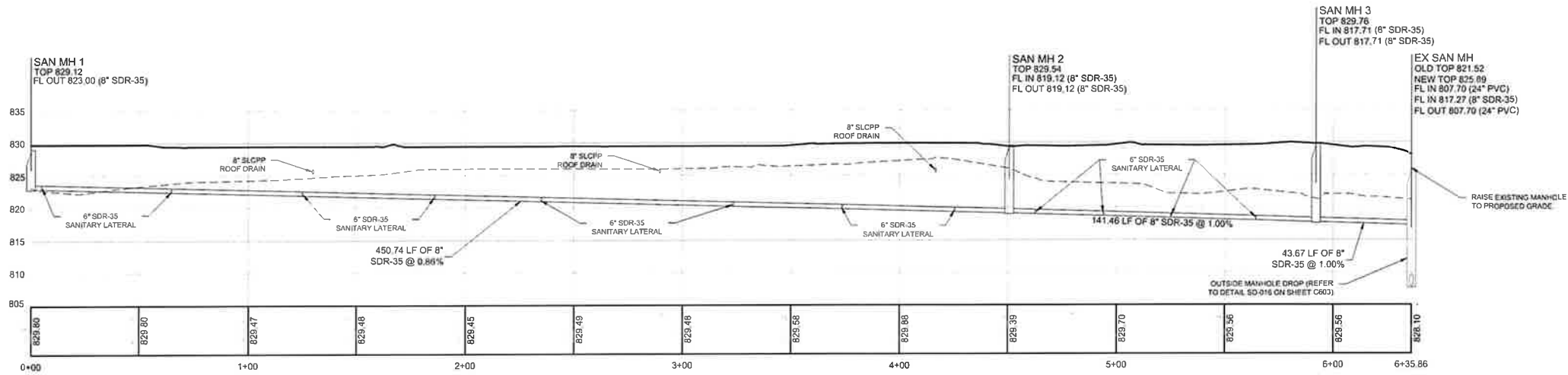


Know what's below.
 Call before you dig.
 811

STORM PROFILES
 Project Number: C-12199-0025
 Drawing Scale: AS NOTED
 Date Issued: AUG 2025
 Index Number: _____
 Drawn By: MCL/RRR
 Checked By: JMG
 Project Manager: JMG

C500

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SAN MH 1 TO EX SAN MH PROFILE
 HORIZONTAL SCALE: 1" = 30'
 VERTICAL SCALE: 1" = 10'

PROFILE NOTES:

1. THE LENGTHS (LF) SHOWN ON THIS PLAN ARE CENTERLINE STRUCTURE TO CENTERLINE STRUCTURE, AND DO NOT ACCOUNT FOR SLOPES OR DEFINE ACTUAL LENGTHS OF PIPE.

PROFILE STRUCTURE ABBREVIATIONS

STM	STORM
SAN	SANITARY
MH	MANHOLE
OS	OUTLET STRUCTURE
CO	CLEANOUT
HW	HEADWALL
EW	ENDWALL

GATEWAY ENGINEERS
 A FULL-SERVICE CIVIL ENGINEERING FIRM



REVISION RECORD

Date	No.	Description
2023-11-11	01	MPP Update
2023-12-16	02	ACCD TECHNICAL AIDES RESPONSE
2024-01-14	03	TOWNSHIP SUBMISSION
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
 PREPARED FOR:
CE - SF, LP
 295 MYOMA ROAD
 MARS, PA 16646

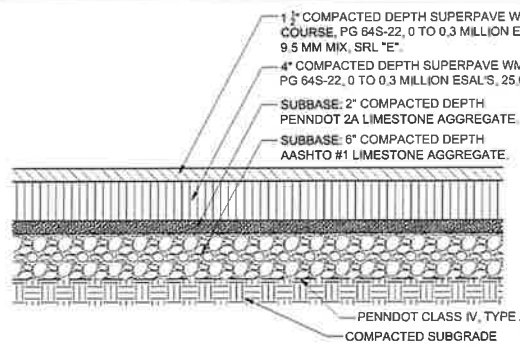
811
 Know what's below.
 Call before you dig.
 Serial No. 20231760058

SANITARY PROFILES

Project Number: C-12199-0025
 Drawing Scale: AS NOTED
 Date Issued: AUG 2023
 Index Number: ---
 Drawn By: MCL/RHR
 Checked By: JMG
 Project Manager: JMG

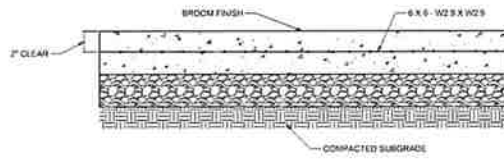
C501

\\pa01\planning\01\1000112109\00001\0020\0000\Drawings\Sanitary\Sheet\C500 - Storm and Sanitary Sewer Profiles.dwg
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 User: JMG



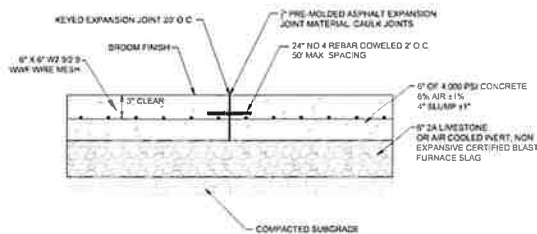
NOTES:
 1. THE CONTRACTOR SHALL SEAL THE GUTTER LINE WITH PG 64-22 ASPHALT CEMENT BY OVERLAPPING 3" ONTO THE WEDGE CURB AND EXTENDING 9" ONTO THE CARTWAY IN ACCORDANCE WITH PENNDOT PUBLICATION 408 AFTER THE COMPLETION OF THE FINAL WEARING COURSE.
 2. SUBGRADE DRAINS MAY BE REQUIRED WHERE UNDERGROUND WATER IS ENCOUNTERED.
 3. SUBGRADE SHALL BE COMPACTED TO VISUAL NON-MOVEMENT PRIOR TO PLACEMENT OF FABRIC AND SUBBASE.
 4. SUBBASE SHALL BE COMPACTED TO 70% RELATIVE DENSITY PER ASTM D4253 AND ASTM D4254.
 5. CONSTRUCTION MATERIALS AND INSTALLATION SHALL CONFORM TO PENNDOT PUBLICATION 408 UNLESS OTHERWISE INDICATED.

1 STANDARD DUTY BITUMINOUS PAVEMENT
C600



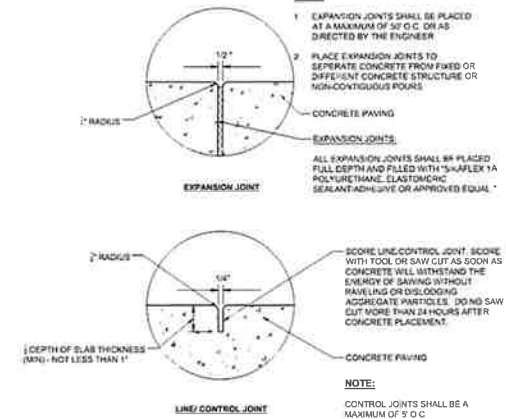
NOTES:
 1. CONTRACTOR SHALL PLACE CONTROL JOINTS AT INTERVALS EQUAL TO THE PAVEMENT WIDTH NOT TO EXCEED 12'
 2. EXPANSION JOINTS SHALL BE INSTALLED AT 50' MAXIMUM INTERVALS
 3. CURING SHALL BE BY ABSORPTIVE COVER, MOISTURE RETAINING COVER, OR APPROVED CURING COMPOUND. CONTRACTOR SHALL SUBMIT PROPOSED CURING METHOD TO OWNER FOR APPROVAL PRIOR TO CONCRETE INSTALLATION
 4. APPLY WATER BASED 40% SILANE PENETRATING SEALER AFTER A MINIMUM OF 28 DAYS PER MANUFACTURER'S RECOMMENDATIONS
 5. PROVIDE MINIMUM CROSS SLOPE OF 1.5% TO MAXIMUM CROSS SLOPE OF 2%

2 4" THICK CONCRETE SIDEWALK
C600



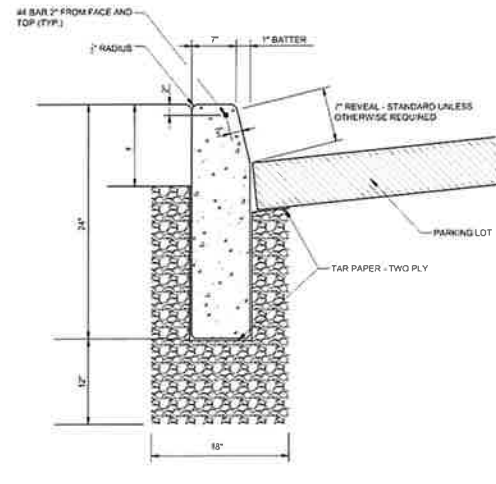
NOTES:
 1. A CONTRACTOR SHALL PLACE A TOOLED JOINT AT EVERY 20' O.C. MINIMUM
 2. APPLY LIQUID MEMBRANE FORMING CURING COMPOUND MEETING ASTM C 309 TYPE 1, CLASS A AND B PER MANUFACTURER'S RECOMMENDATIONS. APPLY WATER BASED 40% SILANE PENETRATING SEALER AFTER A MINIMUM OF 28 DAYS PER MANUFACTURER'S RECOMMENDATIONS

3 6" CONCRETE PAVEMENT
C600



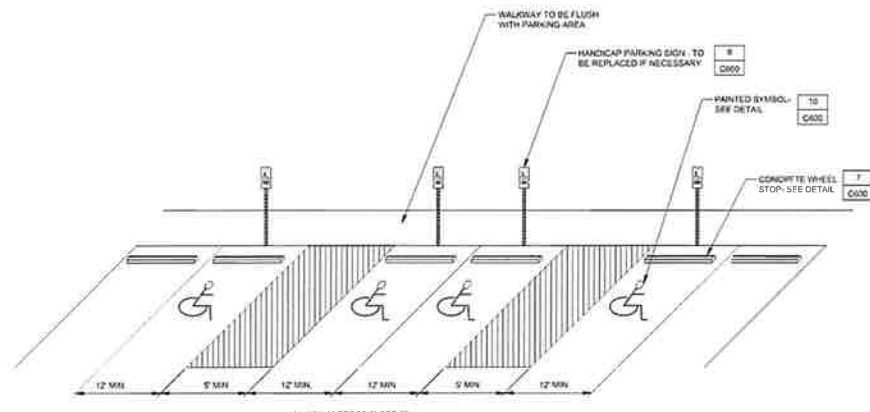
NOTE:
 1. EXPANSION JOINTS SHALL BE PLACED AT A MAXIMUM OF 50' O.C. DEAS DIRECTED BY THE ENGINEER
 2. PLACE EXPANSION JOINTS TO SEPARATE CONCRETE FROM FIXED OR DIFFERENT CONCRETE STRUCTURE OR NON-CONTIGUOUS POURS
 3. ALL EXPANSION JOINTS SHALL BE PLACED FULL DEPTH AND FILLED WITH S-FLEX 1A POLYURETHANE ELASTOMERIC SEALANT ADHESIVE OR APPROVED EQUAL
 4. SCORE LINE CONTROL JOINT: SCORE WITH TOOL OR SAW CUT AS SOON AS CONCRETE WILL WITHSTAND THE ENERGY OF SAWING WITHOUT RAVELING OR DISLODGING AGGREGATE PARTICLES. DO NOT SAW CUT MORE THAN 24 HOURS AFTER CONCRETE PLACEMENT
NOTE:
 CONTROL JOINTS SHALL BE A MAXIMUM OF 7' O.C.

4 CONCRETE EXPANSION/SCORE LINE/ CONTROL JOINT
C600



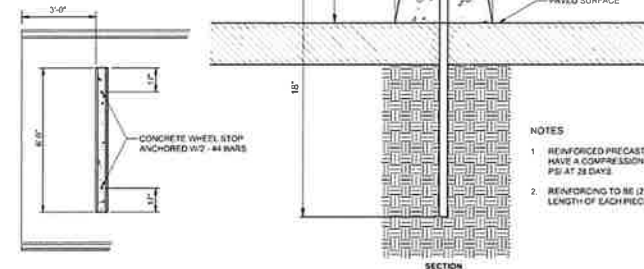
NOTE:
 CURB CONSTRUCTION JOINTS SHALL COINCIDE WITH TRANSVERSE JOINTS IN PAVEMENT OR BASE. MAXIMUM LENGTH 10' - 0"

5 CONCRETE DEEP CURB (7" INCH REVEAL)
C600



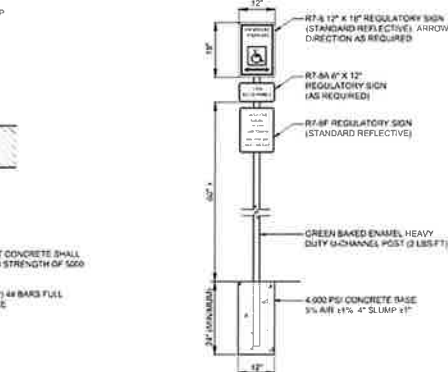
NOTES:
 1. REFER TO SITE PLAN FOR ADDITIONAL INFORMATION

6 TYPICAL HANDICAP PARKING
C600

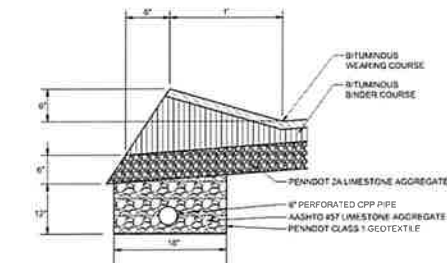


NOTES:
 1. REINFORCED PRECAST CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS
 2. REINFORCING TO BE (2) #4 BARS FULL LENGTH OF EACH PIECE

7 CONCRETE WHEEL STOP
C600

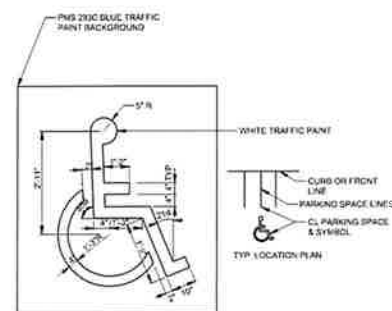


8 ACCESSIBLE PARKING SIGN
C600



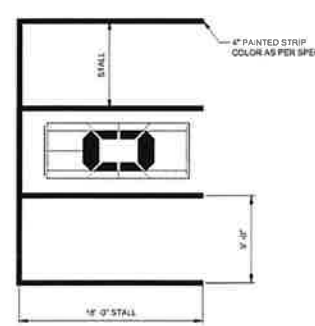
NOTES:
 1. THE CONTRACTOR SHALL SEAL THE CUTTER LINE WITH PG 64S-22 ASPHALT CEMENT BY OVERLAPPING 2" ONTO THE WEDGE CURB AND EXTENDING 9" ONTO THE CARTWAY IN ACCORDANCE WITH PENNDOT PUBLICATION 408 AFTER THE COMPLETION OF THE FINAL WEARING COURSE.
 2. SUBGRADE DRAINS MAY BE REQUIRED WHERE UNDERGROUND WATER IS ENCOUNTERED.
 3. CONSTRUCTION MATERIALS AND INSTALLATION SHALL CONFORM TO PENNDOT PUBLICATION 408 UNLESS OTHERWISE INDICATED.
 4. PLACE BITUMINOUS WEDGE CURB DRAINS AS DIRECTED BY OWNER.

9 BITUMINOUS WEDGE CURB
C600

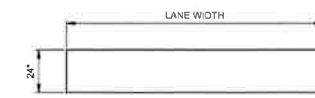


NOTE: SEE SITE PLAN FOR LOCATIONS

10 ADA SYMBOL
C600

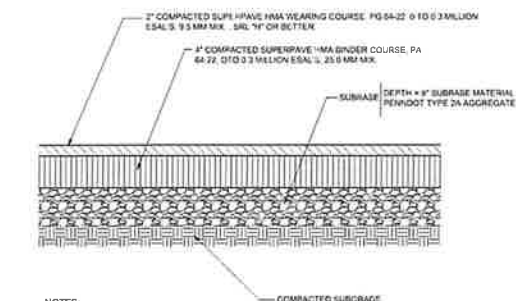


11 PARKING STALL DETAIL
C600



NOTE:
 STOP LINES ARE SOLID WHITE LINES THAT COMPLETELY TRAVERSE EACH TRAFFIC LANE. AT AN INTERSECTION WITH A STOP SIGN, THE STOP LINE SHOULD BE PLACED AT A LOCATION NOT LESS THAN 4' OR MORE THAN 30' FROM THE NEAR SIDE OF THE INTERSECTING ROADWAY TO ENSURE MAXIMUM SIGHT DISTANCE TO VEHICLES ON THE CROSSING ROUTE. WHEN USED ON MULTILANE APPROACH TO A SIGNALIZED INTERSECTION, THE STOP LINE MAY BE ADJUSTED TO ASSIST TURNING VEHICLES AND TO IMPROVE SIGHT DISTANCE FOR MOTORIST DESIRING TO MAKE A TURN ON RED.

12 STOP BAR
C600



NOTES:
 1. ONCE THE FINAL WEARING COURSE OF BITUMINOUS PAVING IS PLACED, THE CONTRACTOR SHALL SEAL THE GUTTER LINE WITH PG 64S-22 ASPHALT CEMENT BY OVERLAPPING 3" ONTO THE WEDGE CURB OR 1" FOR CONCRETE CURB AND EXTENDING 9" ONTO THE CARTWAY IN ACCORDANCE WITH PENNDOT PUBLICATION 408
 2. SUBGRADE DRAINS MAY BE REQUIRED WHERE UNDERGROUND WATER IS ENCOUNTERED
 3. CONSTRUCTION MATERIALS AND INSTALLATION MUST CONFORM TO PENNDOT 408

13 HEAVY DUTY BITUMINOUS PAVEMENT
C600

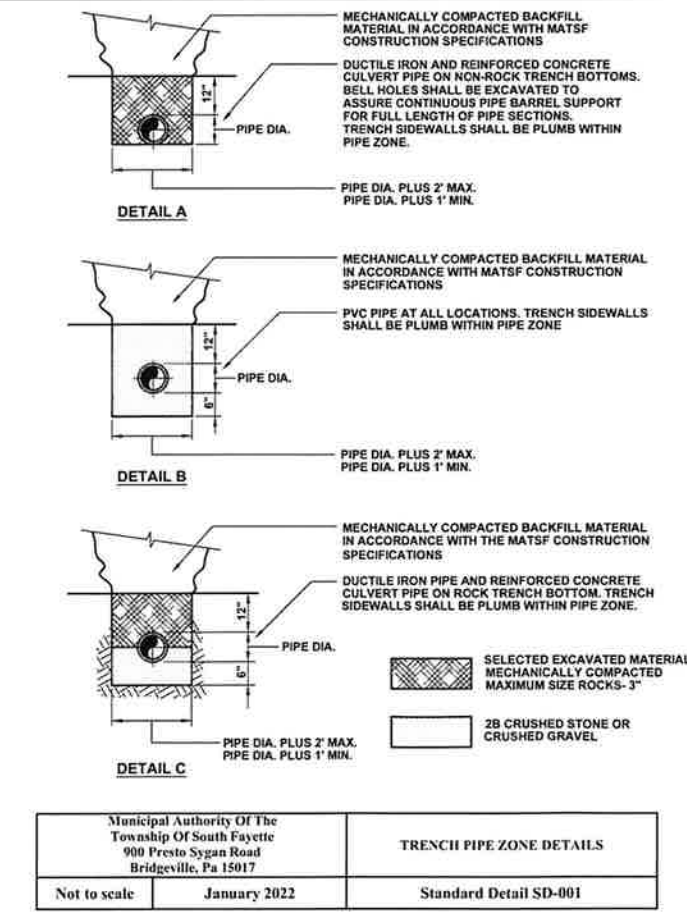
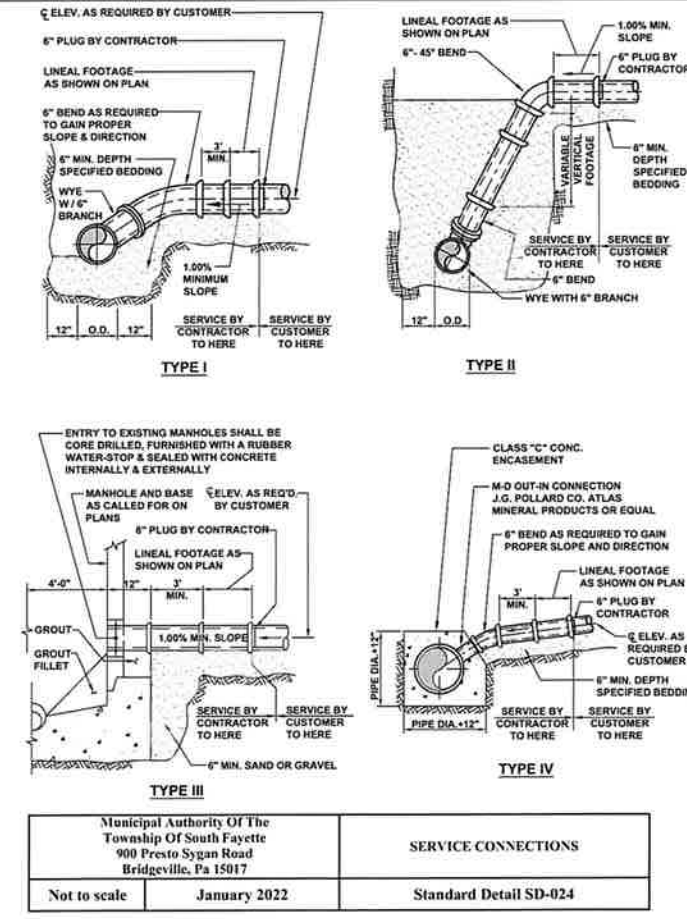
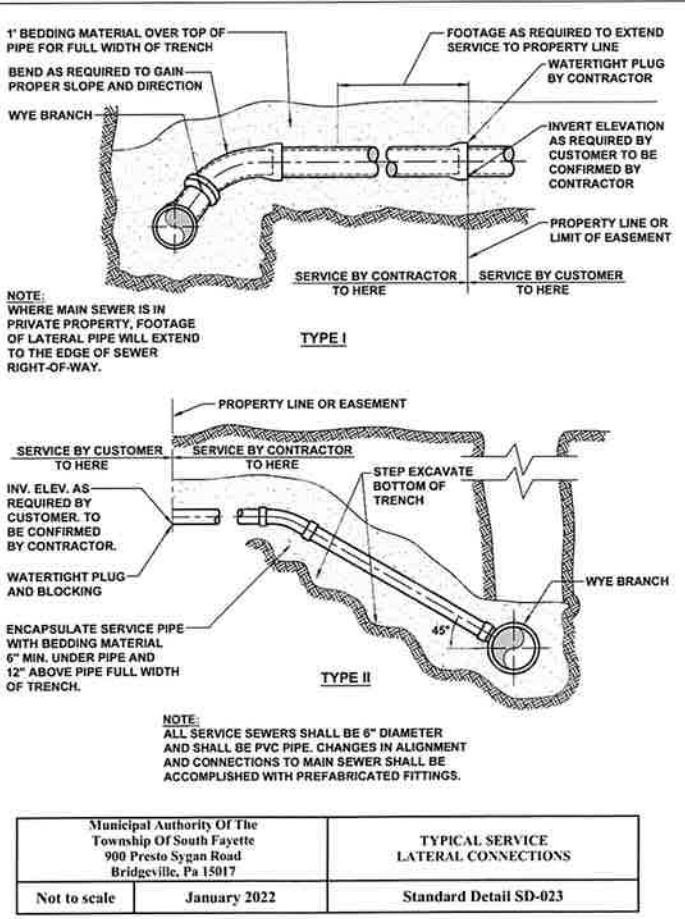
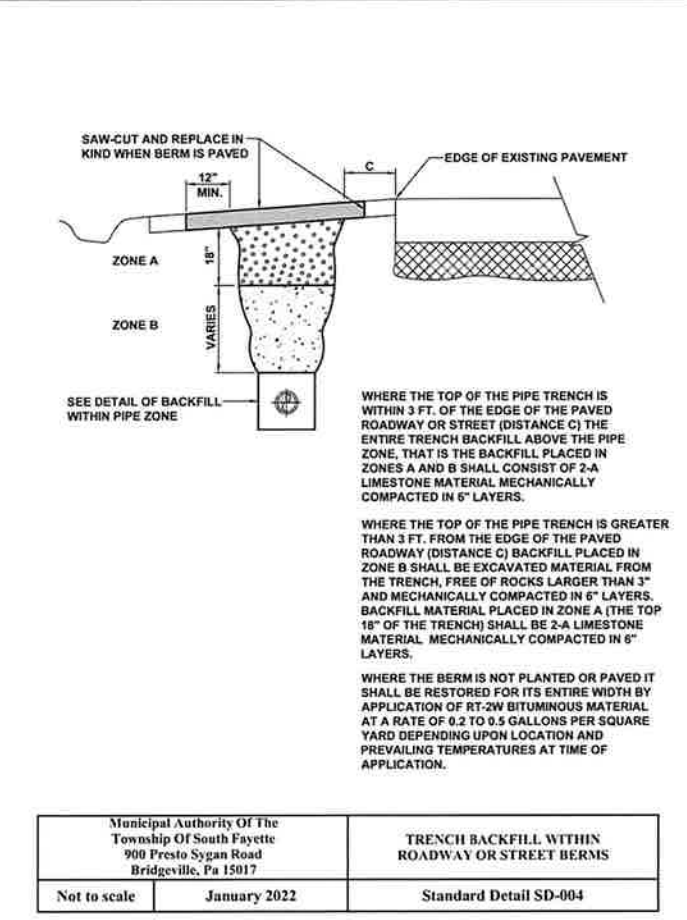
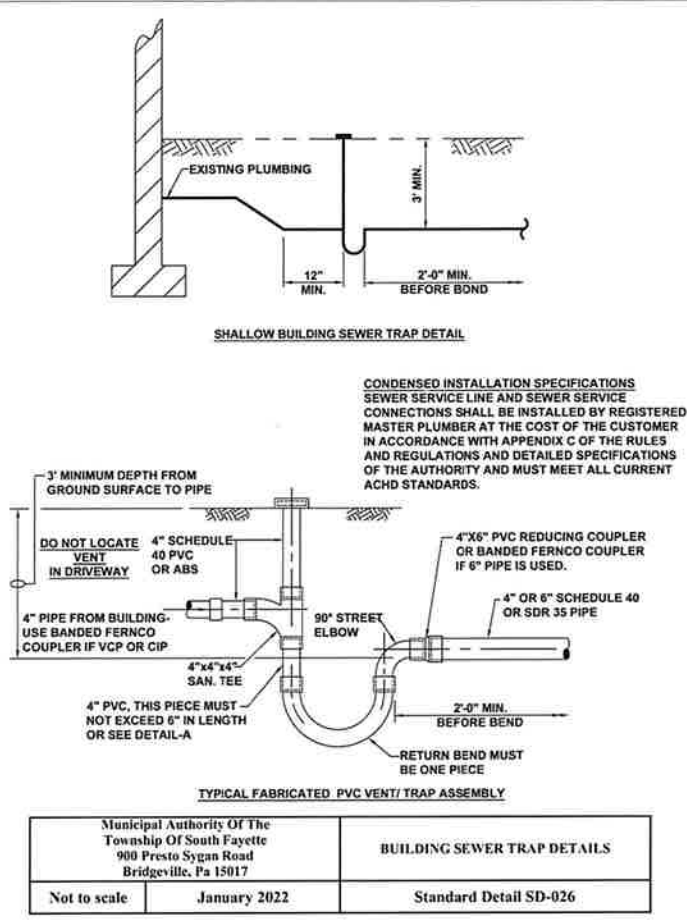
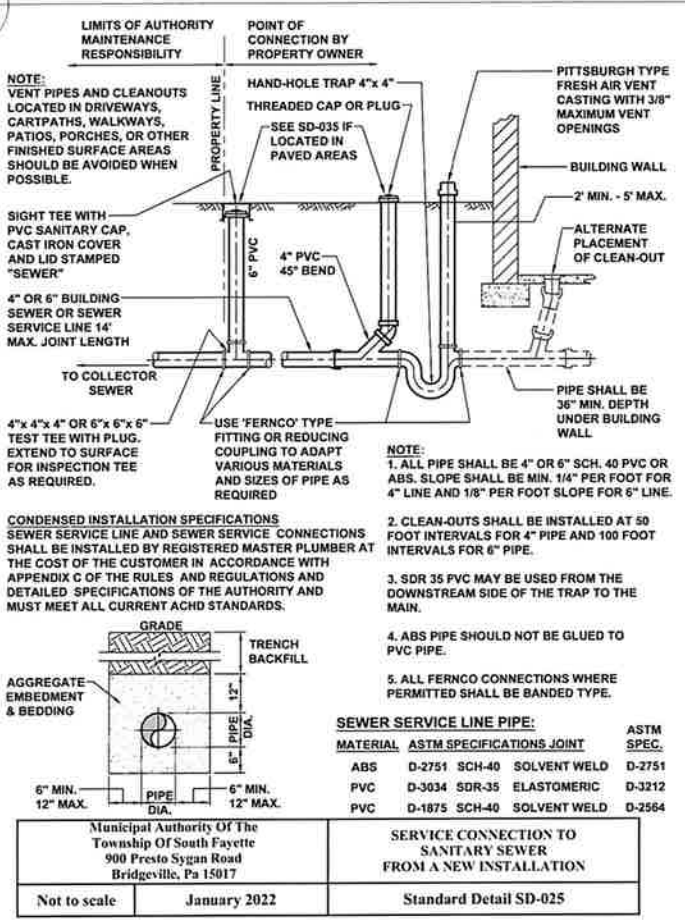
REVISION RECORD			
No.	Date	Description	By
01	2025-11-11	WPP Updates	JMG
02	2025-12-16	ACCD TECHNICAL NOTES RESPONSE	JMG
03	2026-01-14	TOWNSHIP SUBMISSION	JMG
04			
05			
06			
07			
08			

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
CE-SF-LP
 295 MYOMA ROAD
 MARS, PA 16846

CONSTRUCTION DETAILS
 Project Number: C-12199-0025
 Drawing Scale: N/A
 Date Issued: AUG 2025
 Index Number:
 Drawn By: MCL/RRR
 Checked By: JMG
 Project Manager: JMG

C600





Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	TYPICAL SERVICE LATERAL CONNECTIONS
Not to scale	Standard Detail SD-023

Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	SERVICE CONNECTIONS
Not to scale	Standard Detail SD-024

Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	TRENCH PIPE ZONE DETAILS
Not to scale	Standard Detail SD-001

GATEWAY ENGINEERS
A FULL-SERVICE CIVIL ENGINEERING FIRM

COMMONWEALTH OF PENNSYLVANIA
REGISTERED PROFESSIONAL ENGINEER
No. 100777
Exp. 12/31/2025

REVISION RECORD

No.	Date	Description
01	2025-11-11	WPP URGENT
02	2025-12-16	ACCD TECHNICAL NOTES RESPONSE
03	2025-01-14	TOWNSHIP SUBMISSION
04		
05		
06		
07		
08		

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031

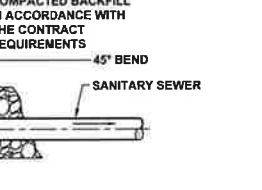
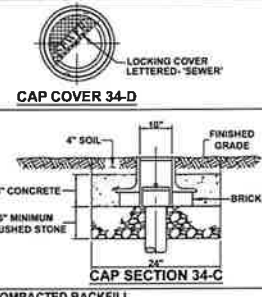
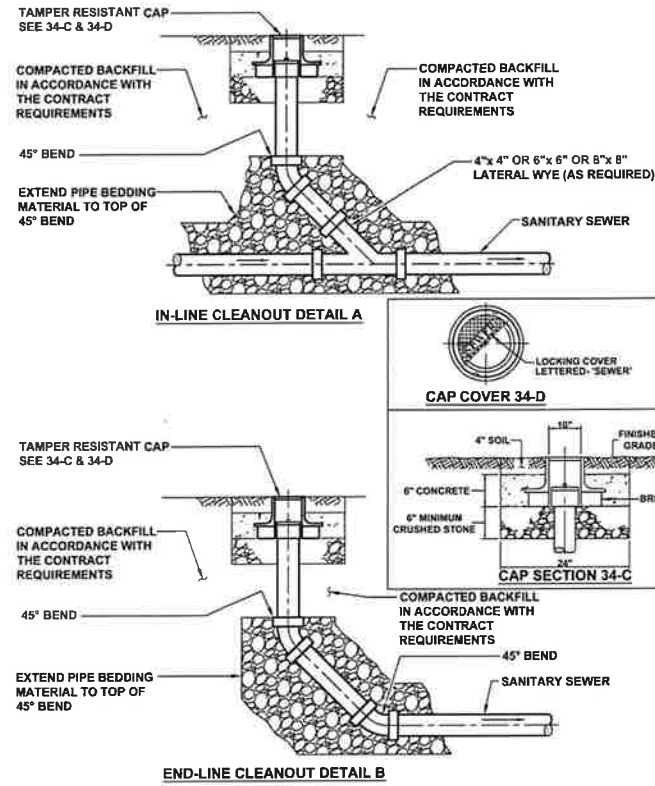
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARKS, PA 16046

CONSTRUCTION DETAILS

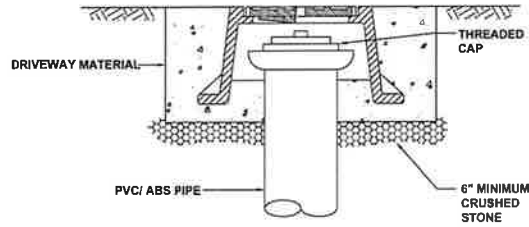
Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

C602

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3. Plot User: JMG
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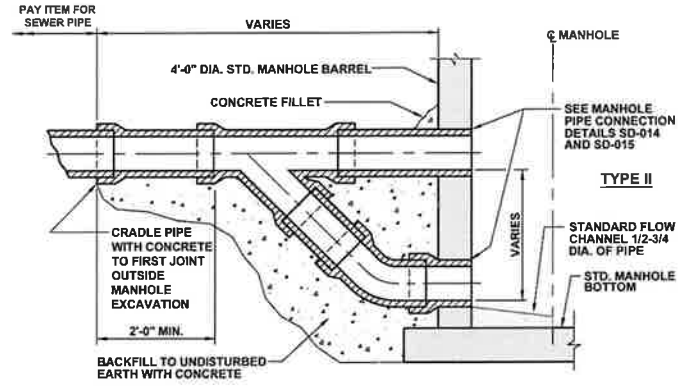
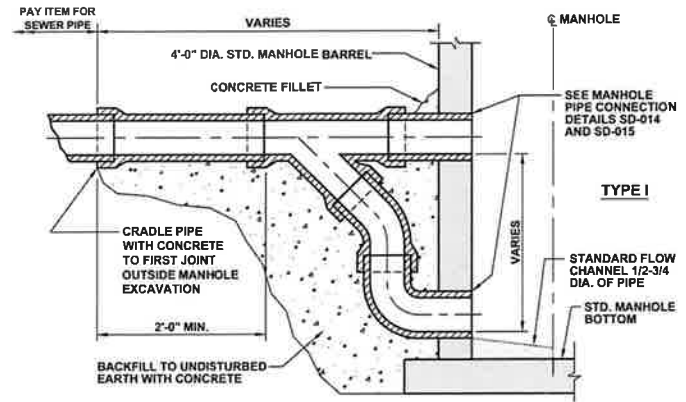


Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	MAIN LINE, IN-LINE, AND END LINE CLEANOUT FOR 6" AND 8" PVC SEWER
Not to scale	January 2022
Standard Detail SD-034	

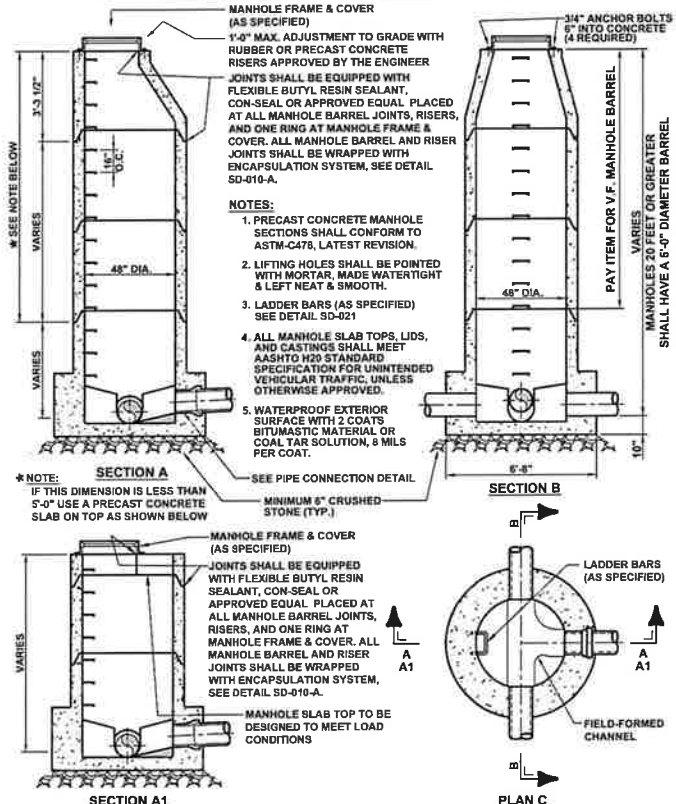


NOTE:
FRAME & COVER SHALL BE VALVCO INC. #668 OR APPROVED EQUAL.

Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	LATERAL SITE TEE/CLEANOUT IN PAVED AREAS
Not to scale	January 2022
Standard Detail SD-035	



Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	OUTSIDE MANHOLE DROP CONNECTION
Not to scale	January 2022
Standard Detail SD-016	



- NOTES:**
1. PRECAST CONCRETE MANHOLE SECTIONS SHALL CONFORM TO ASTM C476, LATEST REVISION.
 2. LIFTING HOLES SHALL BE POINTED WITH MORTAR, MADE WATERTIGHT & LEFT NEAT & SMOOTH.
 3. LADDER BARS (AS SPECIFIED) SEE DETAIL SD-021
 4. ALL MANHOLE SLAB TOPS, LIDS, AND CASTINGS SHALL MEET AASHTO H20 STANDARD SPECIFICATION FOR UNINTENDED VEHICULAR TRAFFIC, UNLESS OTHERWISE APPROVED.
 5. WATERPROOF EXTERIOR SURFACE WITH 2 COATS BITUMASTIC MATERIAL OR COAL TAR SOLUTION, 8 MILS PER COAT.

* NOTE: IF THIS DIMENSION IS LESS THAN 5'-0" USE A PRECAST CONCRETE SLAB ON TOP AS SHOWN BELOW

SEE PIPE CONNECTION DETAIL

MINIMUM 6" CRUSHED STONE (TYP.)

MANHOLE FRAME & COVER (AS SPECIFIED)

1'-0" MAX. ADJUSTMENT TO GRADE WITH RUBBER OR PRECAST CONCRETE RISERS APPROVED BY THE ENGINEER

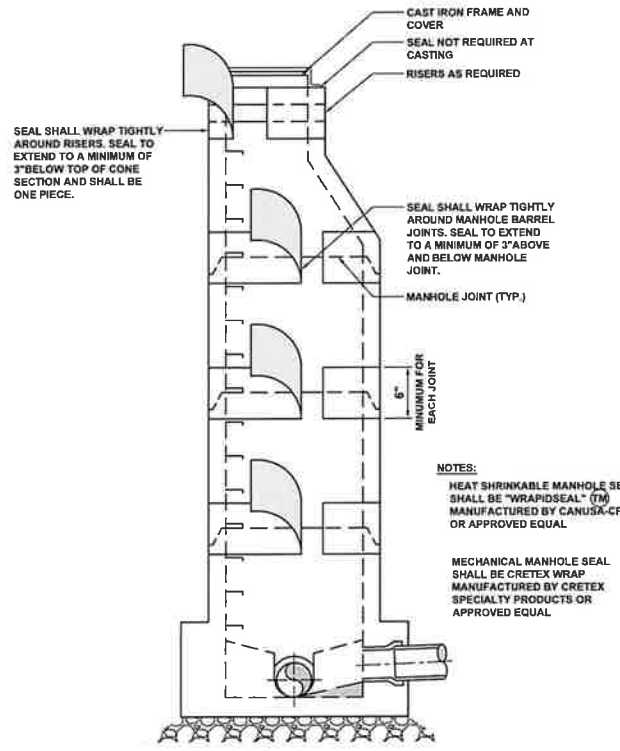
JOINTS SHALL BE EQUIPPED WITH FLEXIBLE BUTYL RESIN SEALANT, CON-SEAL OR APPROVED EQUAL PLACED AT ALL MANHOLE BARREL JOINTS, RISERS, AND ONE RING AT MANHOLE FRAME & COVER. ALL MANHOLE BARREL AND RISER JOINTS SHALL BE WRAPPED WITH ENCAPSULATION SYSTEM, SEE DETAIL SD-010-A.

MANHOLE SLAB TOP TO BE DESIGNED TO MEET LOAD CONDITIONS

LADDER BARS (AS SPECIFIED)

FIELD-FORMED CHANNEL

Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	PRECAST CONCRETE MANHOLE FOR SEWERS 8" TO 18"
Not to scale	January 2022
Standard Detail SD-010	



SEAL SHALL WRAP TIGHTLY AROUND RISERS. SEAL TO EXTEND TO A MINIMUM OF 3" BELOW TOP OF CONE SECTION AND SHALL BE ONE PIECE.

SEAL SHALL WRAP TIGHTLY AROUND MANHOLE BARREL JOINTS. SEAL TO EXTEND TO A MINIMUM OF 3" ABOVE AND BELOW MANHOLE JOINT.

MANHOLE JOINT (TYP.)

6" MINIMUM FOR EACH JOINT

NOTES:

HEAT SHRINKABLE MANHOLE SEAL SHALL BE "WRAPSEAL" (TM) MANUFACTURED BY CANUS/CPS OR APPROVED EQUAL

MECHANICAL MANHOLE SEAL SHALL BE CRETEX WRAP MANUFACTURED BY CRETEX SPECIALTY PRODUCTS OR APPROVED EQUAL

Municipal Authority Of The Township Of South Fayette 900 Presto Sygan Road Bridgeville, Pa 15017	MANHOLE JOINT ENCAPSULATION SYSTEM
Not to scale	January 2022
Standard Detail SD-010-A	



REVISION RECORD

No.	Date	Description
01	11-11-2025	WPP Updates
02	12-10-2025	ACCD TECHNICAL NPDES RESPONSE
03	01-14-2026	TOWNSHIP SUBMISSION
04		
05		
06		
07		
08		

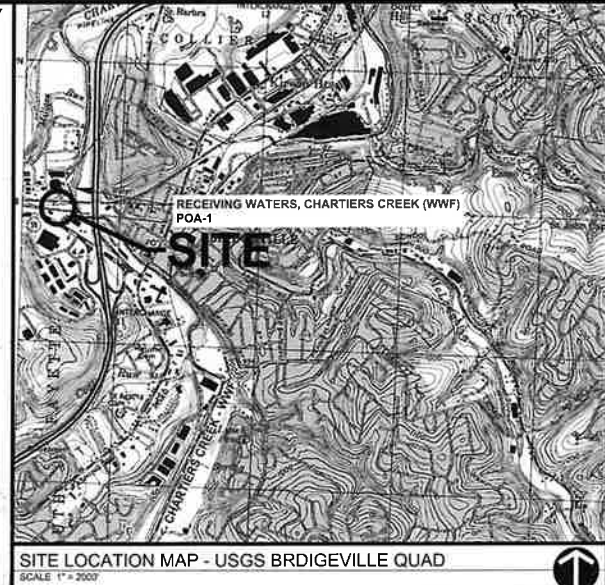
LAFAYETTE 180
NEWBURY DRIVE
CLUDDY, PA 15031
PREPARED FOR:
CE-SF, LP
295 MYCMA ROAD
MARKS, PA 16046

CONSTRUCTION DETAILS

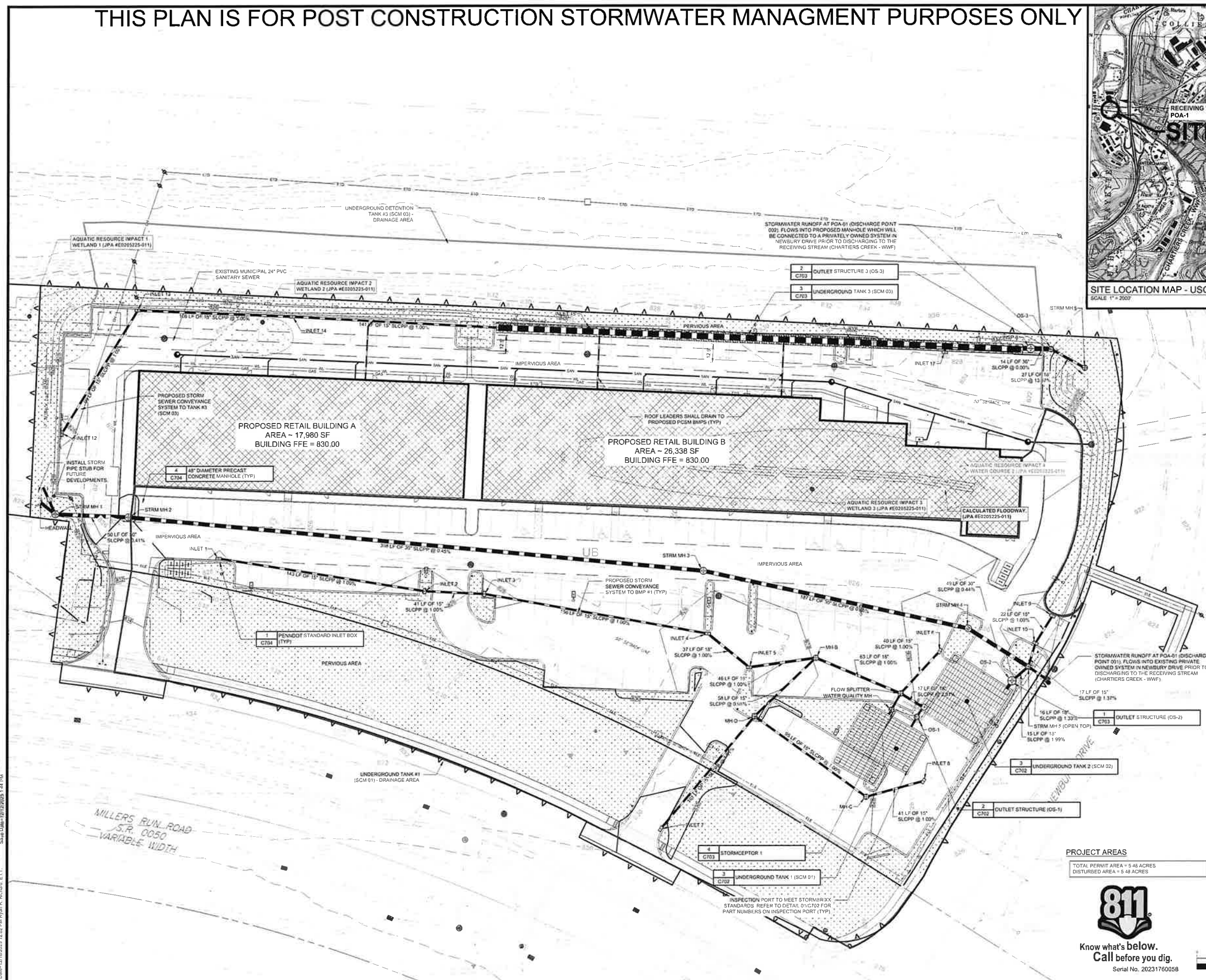
Project Number: C-12159-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

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THIS PLAN IS FOR POST CONSTRUCTION STORMWATER MANAGEMENT PURPOSES ONLY



GATEWAY ENGINEERS
 A FULL-SERVICE CIVIL ENGINEERING FIRM



POST CONSTRUCTION STORMWATER MANAGEMENT LEGEND

□	1 PENNDOT STANDARD INLET BOX C704
○	2 CLEANOUT C704
⊙	4 PROPOSED 48" STORM MANHOLE C704
—	PROPOSED STORM LINE
▨	SOIL BOUNDARIES
▤	LIMITS OF DISTURBANCE / PERMIT BOUNDARY
▧	AREAS TO BE PERMANENTLY SEEDED
▩	AQUATIC RESOURCE WETLANDS
▪	AQUATIC RESOURCE WATER COURSE
▫	AQUATIC RESOURCE FLOODWAY

- POST CONSTRUCTION STORMWATER MANAGEMENT NOTES:**
- THE CONTRACTOR SHALL COMPLY AT ALL TIMES WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, PROVISIONS AND POLICIES GOVERNING SAFETY AND HEALTH INCLUDING THE FEDERAL CONSTRUCTION SAFETY ACT (PUBLIC LAW 91-594) FEDERAL REGISTER, CHAPTER XVI, PART 593 OF TITLE 29 REGULATIONS OCCUPATIONAL SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION AND SUBSEQUENT PUBLICATIONS UPDATING THESE REGULATIONS.
 - THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXAMINING THE AREAS AND CONDITIONS UNDER WHICH THE PROJECT IS TO BE CONSTRUCTED PRIOR TO THE SUBMISSION OF THE BID. SUBMISSION OF A BID SHALL BE CONSTRUED TO MEAN THE CONTRACTOR HAS REVIEWED THE SITE AND IS FAMILIAR WITH CONDITIONS AND CONSTRAINTS OF THE SITE.
 - BEFORE EXCAVATION, ALL UNDERGROUND UTILITIES SHALL BE LOCATED IN THE FIELD BY THE PROPER AUTHORITIES. THE CONTRACTOR SHALL NOTIFY PENNSYLVANIA ONE CALL SYSTEMS, INC AT 1-800-482-4178. THE LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES ARE APPROXIMATE AND MAY NOT ALL BE SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE EXISTENCE AND EXACT LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES.
 - CONTRACTOR TO PROVIDE SHOP DRAWINGS ON ALL STORM SEWER MANHOLES, INLETS AND DETENTION SYSTEMS.
 - AN AS-BUILT DRAWING OF NEW UTILITY SERVICES SHALL BE PREPARED BY THE CONTRACTOR AND SUBMITTED TO THE OWNER UPON COMPLETION OF THE PROJECT.
 - ALL STORM PIPE SHALL BE HDPE SMOOTH INTERIOR CORRUGATED POLYETHYLENE PIPE UNLESS OTHERWISE NOTED. ALL STORM SEWER CONSTRUCTION MATERIALS AND METHODS SHALL BE IN ACCORDANCE WITH PENNDOT SPECIFICATIONS. ALL JOINTS SHALL BE WATERTIGHT.
 - CONTRACTOR SHALL REFER TO OTHER PLANS WITHIN THIS CONSTRUCTION SET FOR OTHER PERTINENT INFORMATION.
 - THE CONTRACTOR SHALL ASSURE THAT THERE IS POSITIVE DRAINAGE TO THE INLETS UPON PLACEMENT OF NEW PAVEMENT.
 - INLETS MUST BE SIZED TO ACCEPT THE SPECIFIED PIPE SIZES WITHOUT KNOCKING OUT ANY OF THE INLET CORNERS. ALL PIPES ENTERING OR EXISTING INLETS SHALL BE CUT FLUSH WITH THE INSIDE WALL OF THE INLET.
 - NO PERSON SHALL MODIFY, REMOVE, FILL, LANDSCAPE, OR ALTER ANY SWM BMPs, FACILITIES, AREAS, OR STRUCTURES WITHOUT THE WRITTEN APPROVAL OF THE TOWNSHIP.
 - CONTRACTOR IS REQUIRED TO NOTIFY AN ENVIRONMENTAL PROFESSIONAL (EP) PRIOR TO ANY EXCAVATION AND HAVE THEM ON-SITE TO OVERSEE ALL EXCAVATION AND HANDLING OF MATERIAL. THE CONTRACTOR SHALL REFER TO THE SOIL MANAGEMENT PLAN / MATERIAL MANAGEMENT PLAN PREPARED BY CHIRBA & ASSOCIATES, LLC AND THE ON-SITE EP FOR THE REQUIRED SOIL HANDLING, STOCKPILE, DISPOSAL, ETC PROCEDURES.

PROJECT AREAS
 TOTAL PERMIT AREA = 5.48 ACRES
 DISTURBED AREA = 5.48 ACRES

811
 Know what's below.
 Call before you dig.
 Serial No. 20231760058

GRAPHIC SCALE
 NORTH
 1 inch = 30 feet

REVISION RECORD

No.	Date	By	Description
1	2025-11-11	CI	UTPIP Updates
2	2025-12-16	CI	ACCD TECHNICAL IMPROVEMENTS

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
CE - SF, LP
 295 MYOMA ROAD
 MARS, PA 16046

POST CONSTRUCTION STORMWATER MANAGEMENT PLAN
 Project Number: C-12199-0023
 Drawing Scale: 1" = 30'
 Date Issued: AUG 2025
 Index Number:
 Drawn By: MCL/HRR
 Checked By: JMG
 Project Manager: JMG
C700

N:\Data & Drawings\GIS\Projects\12199-0023\12199-0023-CADD\Stormwater Management\12199-0023-0000 Post-Construction Stormwater Management_Plan.dwg
 Plot Date: 12/16/2025 12:02 PM Ryan K. Richards, E.I.T.
 Scale: 1" = 30'



REVISION RECORD

NO. 01 WPP Updates
02 ACCO TECHNICAL RESPONSE

Date	No.	Description
2025-11-11	01	WPP Updates
2025-12-16	02	ACCO TECHNICAL RESPONSE
	03	
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARRS, PA 16046

POST CONSTRUCTION
STORMWATER
MANAGEMENT DETAILS
Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number:
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

C702

Material Location	Description	Material Classification	Compaction/Density Requirement (NOTE 1)
FINAL FILL (fill starting from the top of the embedment fill layer. (NOTE 1 and 2))	Suitable Fill Materials as noted in the Project Geotechnical Report and noted on the Site Design Engineer's Plans	See Project Geotechnical Report and Site Design Engineer's Plans	Place Compact on Stairs (fill layer) to density fill. Use at least two full passes of the equipment to level the layer. Continue until 24 inches of total fill thickness has been placed above the tank. For AASHTO M30, a minimum of 90% of the Standard Proctor Maximum Dry Density is recommended.
EMBANKMENT FILL (fill immediately surrounding the sides and top of tank (NOTE 1))			After 24 inches of fill is placed, place fill in accordance with the engineer's record's relative compaction requirement or to 90% of the Standard Proctor Maximum Dry Density, whichever is greater.
BEDDING FILL (fill immediately below the tank (NOTE 1))	Sand Gravel Mixtures or Open Graded Crushed Aggregate Blends	AASHTO M44 A-1, A-2, A-3, A-4 AASHTO M43 3, 3.57, 4, 4D, 5, 5C, 5D	Place Compact on Stairs (fill layer) to density fill. Use at least two full passes of the equipment to level the layer. For AASHTO M43, a minimum of 90% of the Standard Proctor Maximum Dry Density is recommended.

CONSTRUCTION EQUIPMENT CHART

Equipment Make (NOTE 1)	Minimum Gross Vehicle Weight (lbs)	Minimum Fill Depth (min Tank In)
Packer Compactor	5,000	3
Compact Tank Loader (NOTE 1)	2,500	3
Subsoiler Backhoe (NOTE 1)	2,500	3
Low Ground Pressure Tracked Vehicle (NOTE 1)	20,000	3
Roller - Smooth Drum	10,000	3
Roller - Convex Drum	10,000	3
Dump Trucks and Pans	10,000	NOTE 2

COVER CHART

Use Loading Condition	Cover Thickness (inches)
Normal Turfload (American S.A. Vehicle Weight)	12
Passenger Vehicle Parking Lot	18
3/4" Gross Vehicle Weight	20
Passenger Vehicle Parking Lot with one axle AASHTO HS-20 vehicle	22
Transport (AASHTO HS-20 vehicle)	24
Tractor (AASHTO HS-20 vehicle)	26

DETAIL A PIPE WRAP

2 LAYER 600HD INFILTRATION CROSS SECTION

2 LAYER 600HD ACCESS POINT CROSS SECTION

2 LAYER 600HD PIPE INSTALLATION

FRAME AND COVER

TOP VIEW

SECTION A-A

STORMBRIXX STANDARD DETAILS
600HD SYSTEM - 2 LAYER - INFILTRATION

DRAWN BY
A Frye

CHECKED BY
J Jonke

DATE
12/23/2024

REV.
1

ACO, INC.

WEST SALES OFFICE
203 W. BIRCHDALE ST
DALLAS, TEXAS 75212
(972) 450-9552
Fax (972) 411-9999

EAST SALES OFFICE
94 STRALINGDALE DRIVE
HENTON, OH 44002
(440) 541-4754
Fax (440) 539-7235

SOUTHEAST SALES OFFICE
483 WYOMING RD. SUITE 4025
FORT MILL, SC 29715
(803) 540-7292
Fax (803) 542-1552

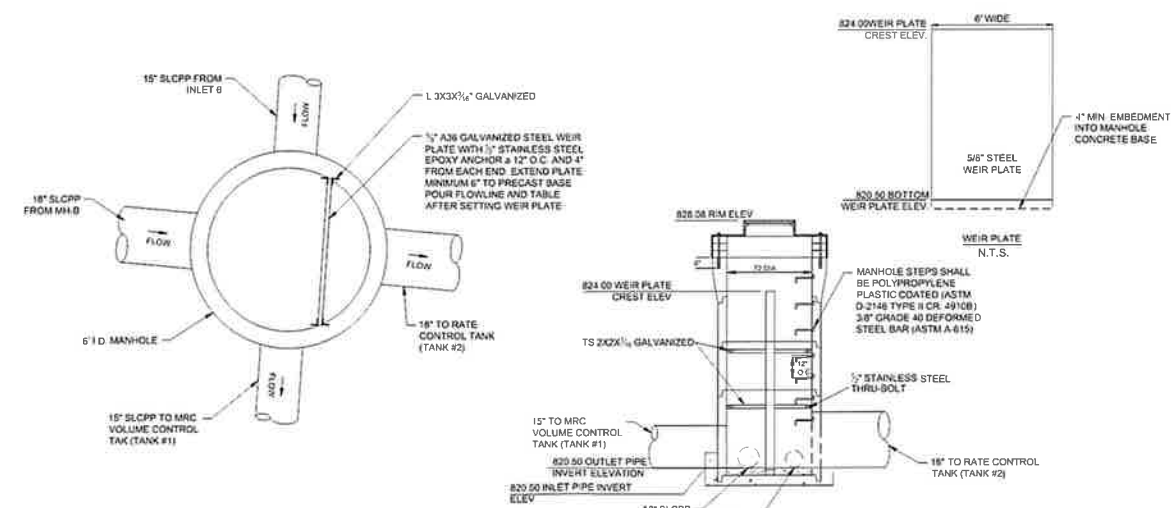
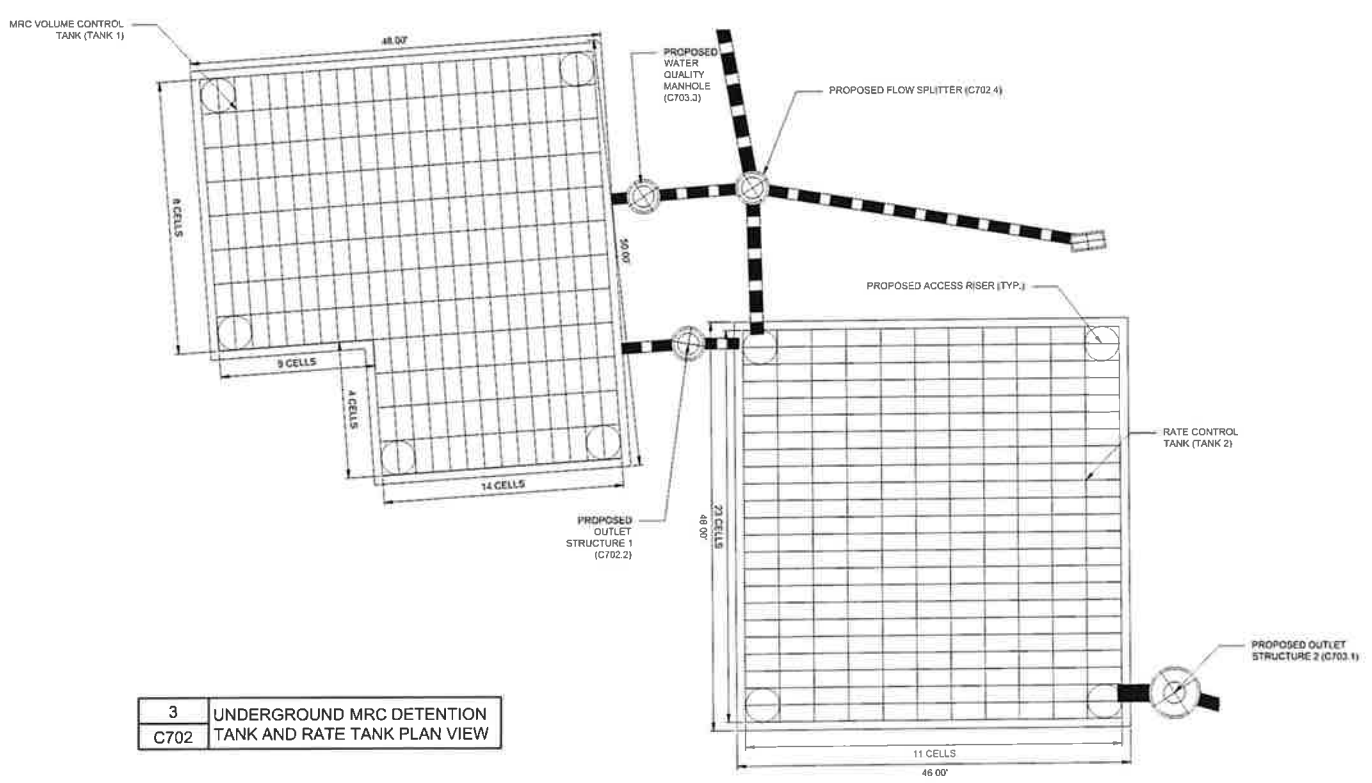
WWW.ACOBRIXX.COM

1 UNDERGROUND TANK 1 AND 2

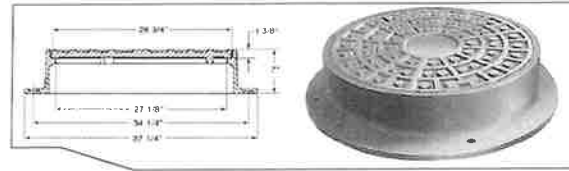
2 OUTLET STRUCTURE 1 (OS 1)

3 UNDERGROUND MRC DETENTION TANK AND RATE TANK PLAN VIEW

4 TYPICAL FLOW SPLITTER STRUCTURE

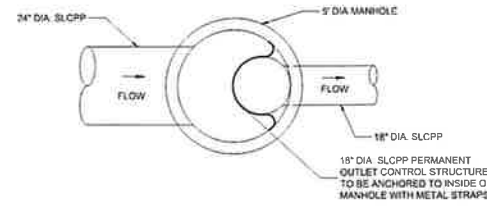


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 20

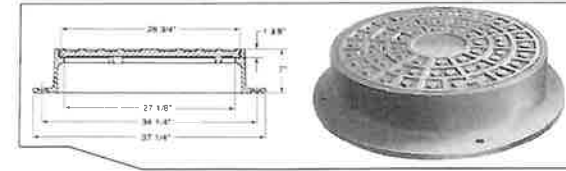


EXCEPT AS NOTED, ALL MANHOLE FRAMES AND COVERS SHALL BE NEENAH FOUNDRY FRAME AND COVER PATTERN NO. R-1753-A AND LABELED "PRIVATE STORM"

FRAME AND COVER

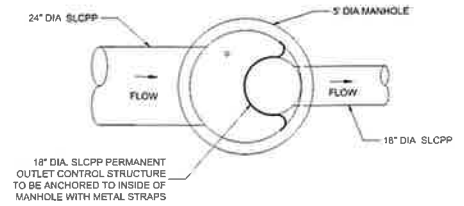


TOP VIEW

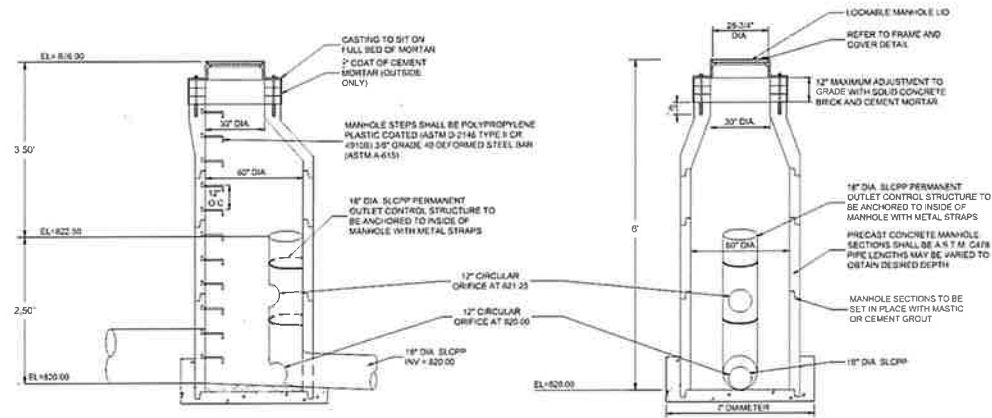


EXCEPT AS NOTED, ALL MANHOLE FRAMES AND COVERS SHALL BE NEENAH FOUNDRY FRAME AND COVER PATTERN NO. R-1753-A AND LABELED "PRIVATE STORM"

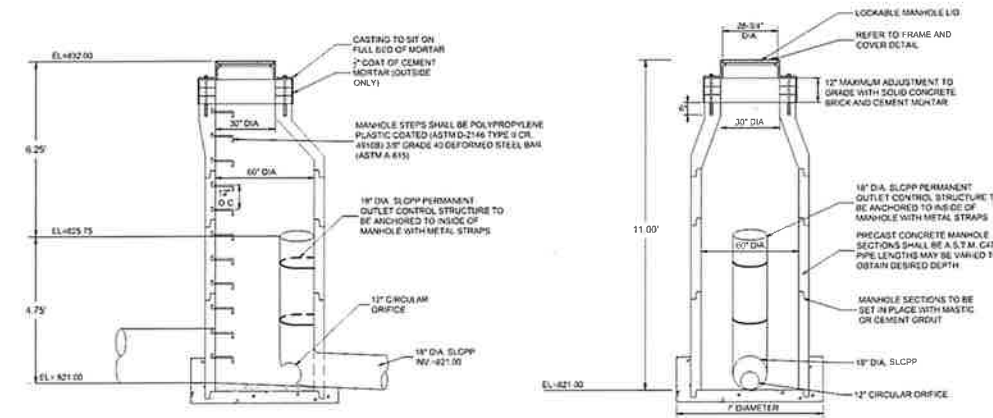
FRAME AND COVER



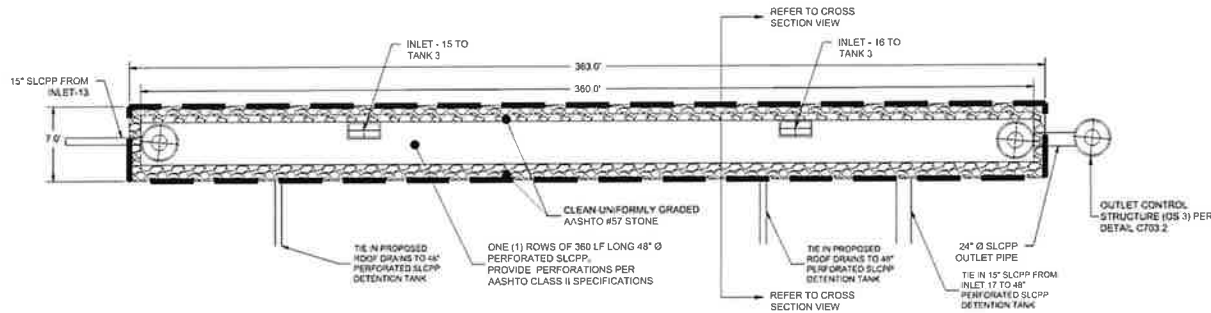
TOP VIEW



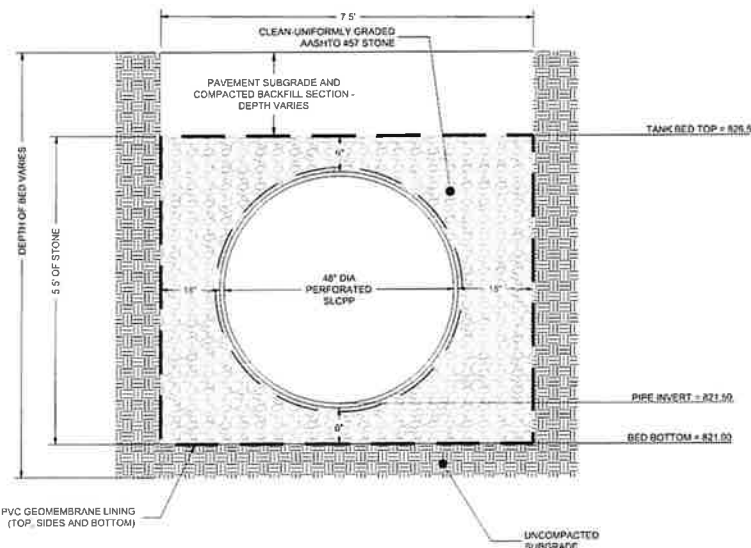
1
C703 OUTLET CONTROL STRUCTURE 2 (OCS 2)



2
C703 OUTLET STRUCTURE 3 (OS 3)



PLAN VIEW
NOT TO SCALE



3
C703 UNDERGROUND TANK 3

PLAN VIEW
TOP SLAB NOT SHOWN

SECTION A-A

STORMCEPTOR DESIGN NOTES

THE STANDARD STC450I CONFIGURATION WITH ROUND, SOLID FRAME AND COVER, AND INLET PIPE IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

CONFIGURATION DESCRIPTION

GRADED INLET ONLY AND INLET PIPE
GRADED INLET WITH INLET PIPE OR PIPES
CURB INLET ONLY AND INLET PIPE
CURB INLET WITH INLET PIPE OR PIPES

SITE SPECIFIC DATA REQUIREMENTS

STRUCTURE ID	WATER QUALITY FLOW RATE (ON LUS)	FRANK FLOOR/BE GRADING	SET POINT IN FEET OF FLOOR FLOWLINE	RAM ELEVATION	PIPE DATA	INLET PIPE 1	INLET PIPE 2	OUTLET PIPE	NOTES/SPECIFIC REQUIREMENTS

FRAME AND COVER
(MAY VARY)
NOT TO SCALE

FRAME AND GRATE
(MAY VARY)
NOT TO SCALE

GENERAL NOTES

- CONTRACTOR TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- FOR SITE SPECIFIC DRAWINGS WITH TABLED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE.
- STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFORM STRUCTURE AND ITS REQUIREMENTS OF PROJECT.
- STORMCEPTOR STRUCTURE SHALL MEET AASHTO H-20S-100 LOAD RATING ASSUMING EARTH COVER OF 0 - 2 FEET AND GROUNDWATER ELEVATION AT OR BELOW THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M240 AND BE CAST WITH THE CONTECH LOGO.
- STORMCEPTOR STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM DATA AND AASHTO LOAD FACTOR DESIGN METHOD.
- ALTERNATE UNITS ARE SHOWN IN MILLIMETERS (MM).

INSTALLATION NOTES

- ANY SURFACE, SHOULDER DEPTH AND/OR ANTI-FLOUNDER PROVISIONS ARE SITE SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMCEPTOR MANHOLE STRUCTURE.
- CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLY STRUCTURE.
- CONTRACTOR TO PROVIDE, INSTALL AND GROUT INLET AND OUTLET PIPES TO MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINE TO MATCH PIPES OFFERING CENTERLINES.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT - HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

CONTECH ENGINEERED SOLUTIONS LLC
www.contech.com
1000 Corporate Blvd., Suite 400, West Chester, OH 45380
614.398.1100 • 614.398.1101 • 614.398.1102 FAX

STC450I
STORMCEPTOR
STANDARD DETAIL

4
C703 STORMCEPTOR 1 (STC 450I)



REVISION RECORD

No.	Date	Description
01	2025-11-11	WPP Updates
02	2025-12-16	ACCD TECHNICAL NOTES RESPONSE
03		
04		
05		
06		
07		
08		

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARS, PA 16046

POST CONSTRUCTION
STORMWATER
MANAGEMENT DETAILS

Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

C703

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 User: jmg
 Plot Date: 12/16/2025 10:27 AM
 Plot User: jmg
 Plot Device: HP DesignJet T1300
 Plot Scale: 1:1
 Plot Range: All Pages
 Plot Color: Yes
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 Plot Zorder: Standard

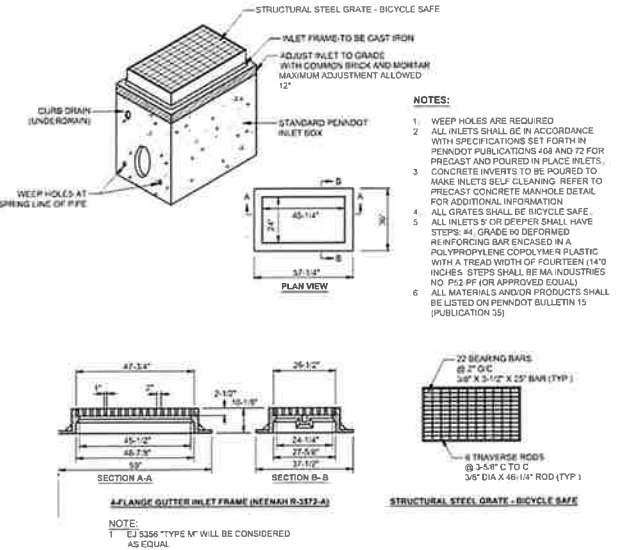


REVISION RECORD

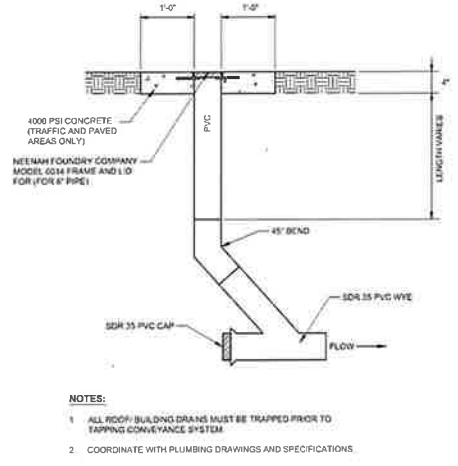
No.	Date	Description
01	2025-11-11	WPP Updates
02	2025-12-16	ACCD TECHNICAL NPDES RESPONSE
03		
04		
05		
06		
07		
08		

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARKS, PA 16846

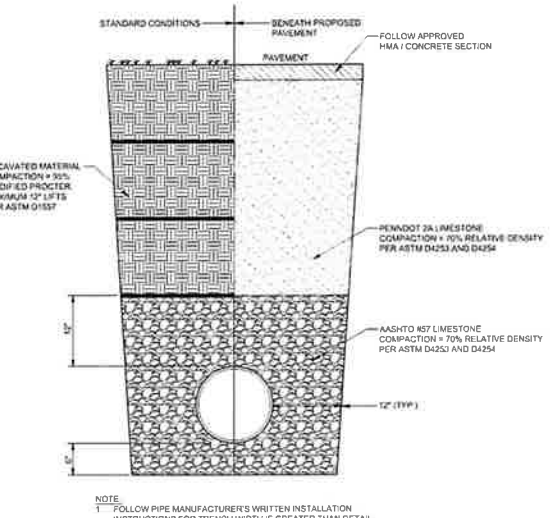
POST CONSTRUCTION
STORMWATER
MANAGEMENT DETAILS
Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number: -
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG



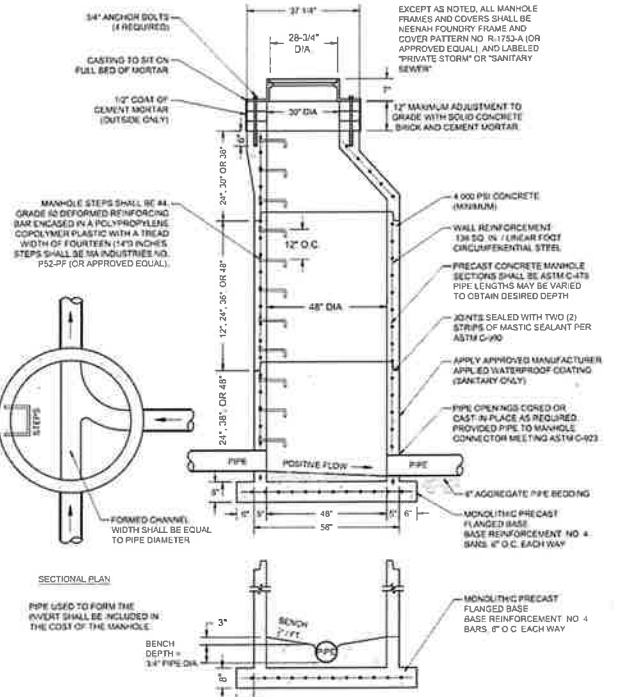
1 PENNDOT STANDARD INLET BOX WITH TYPE M FRAME AND BIKE SAFE GRATE



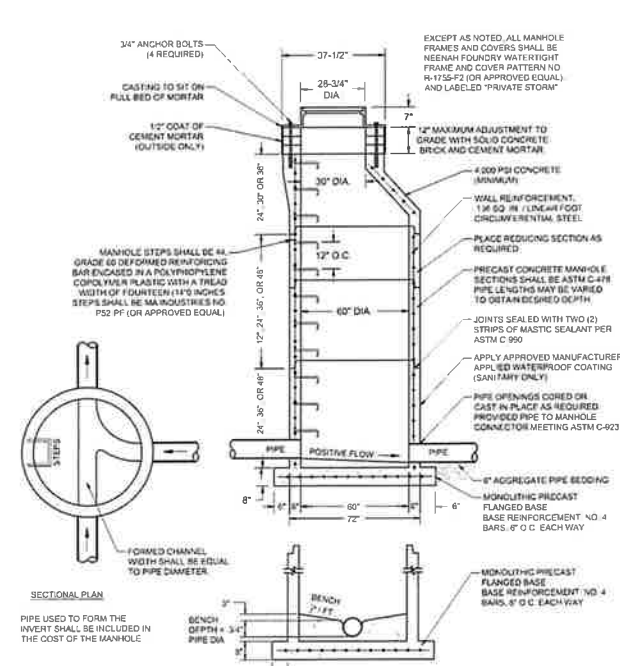
2 CLEANOUT



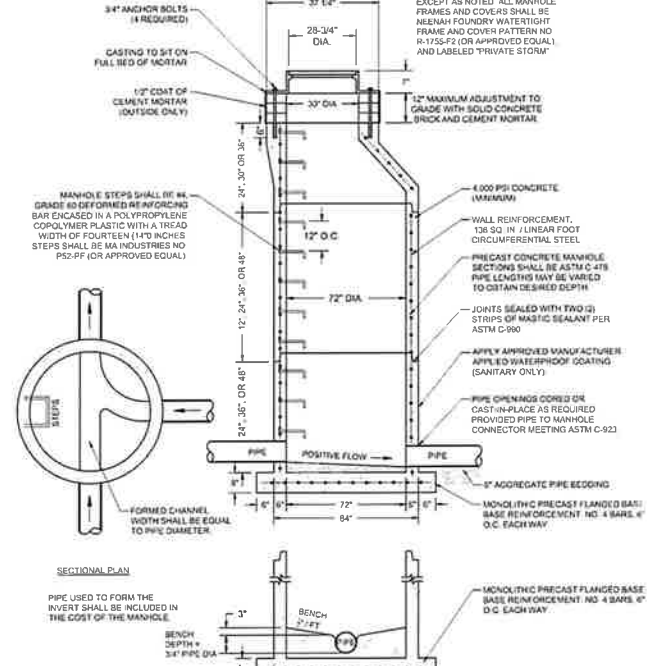
3 UNDERGROUND PIPE OR CONDUIT BACKFILL



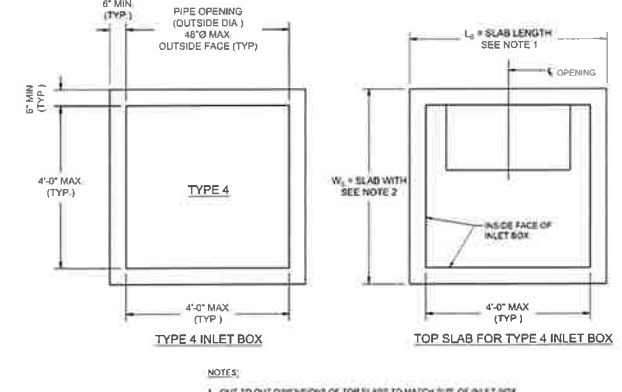
4 48" DIAMETER PRECAST CONCRETE MANHOLE



5 60" DIAMETER PRECAST CONCRETE MANHOLE (STORM)



6 72" DIAMETER PRECAST CONCRETE MANHOLE (STORM)

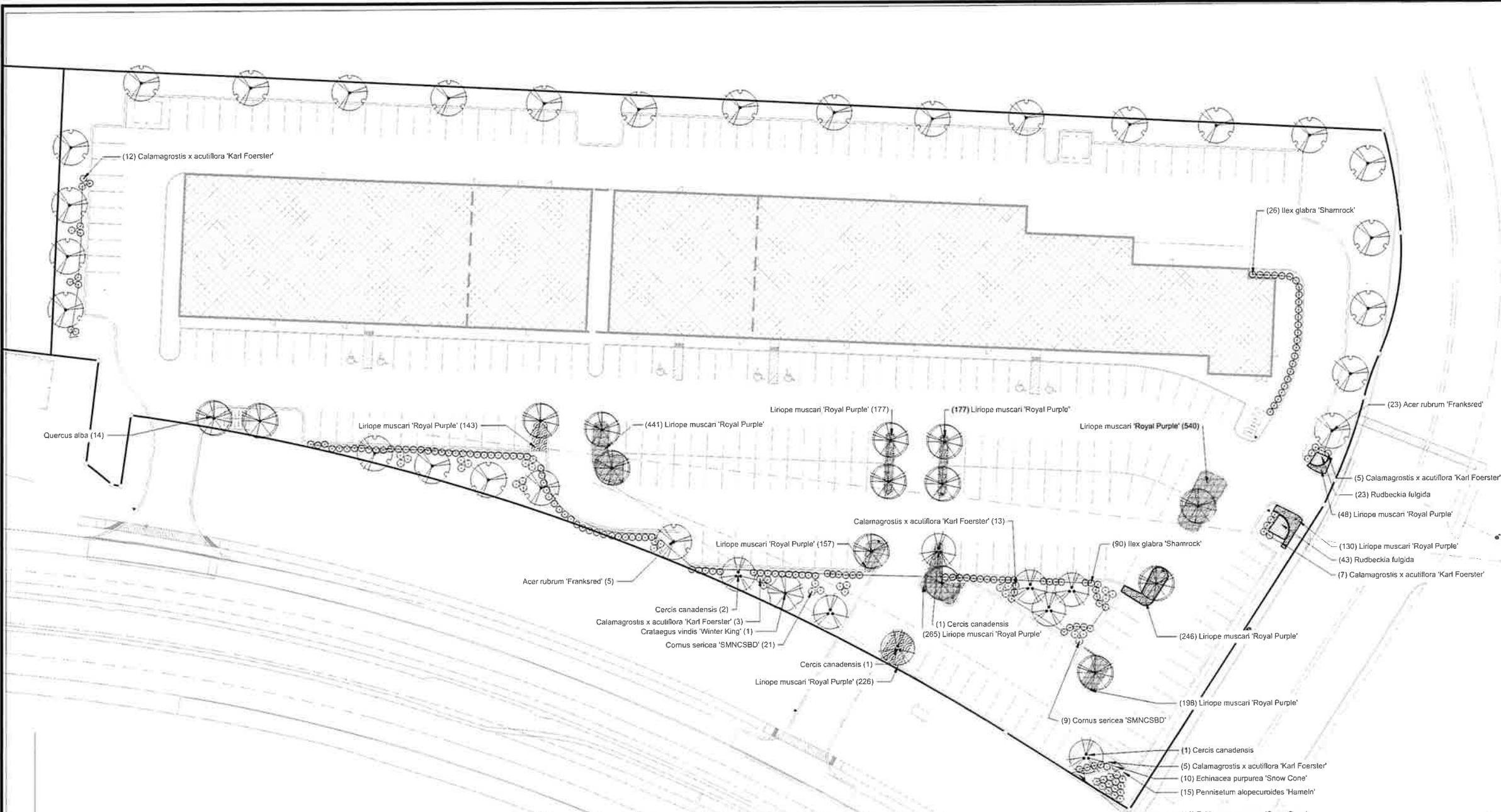


7 PENNDOT TYPE 4 INLET BOX



REVISION RECORD

No.	Date	By	Description
01	2025-07-11	JMG	ISSUE FOR PERMITS
02	2025-12-16	JMG	ACC'D TECHNICAL INFOS RESPONSE
03	2026-01-14	JMG	TOWNSHIP SUBMISSION
04			
05			
06			
07			
08			



LANDSCAPE NOTES:

- THE CONTRACTOR SHALL COMPLY AT ALL TIMES WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, PROVISIONS AND ORDINANCES GOVERNING SAFETY AND HEALTH INCLUDING THE FEDERAL CONSTRUCTION SAFETY ACT (PUBLIC LAW 91-54), FEDERAL REGISTER, CHAPTER XVII, PART 1926 OF TITLE 29 REGULATIONS, OCCUPATIONAL SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION AND SUBSEQUENT PUBLICATIONS UPDATING THESE REGULATIONS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXAMINING THE AREAS AND CONDITIONS UNDER WHICH THE PROJECT IS TO BE CONSTRUCTED PRIOR TO THE SUBMISSION OF THE BID. SUBMISSION OF A BID SHALL BE CONSIDERED TO MEAN THE CONTRACTOR HAS REVIEWED THE SITE AND IS FAMILIAR WITH CONDITIONS AND CONSTRAINTS OF THE SITE.
- PRIOR TO THE INSTALLATION OF PLANT MATERIAL, THE CONTRACTOR SHALL ACCURATELY LOCATE ALL EXISTING UNDERGROUND UTILITIES, INCLUDING ALL RECENTLY INSTALLED UTILITIES. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE LANDSCAPE ARCHITECT OF ANY CONFLICTS WITH THE PLANT MATERIAL. ANY UTILITIES DAMAGED AS A RESULT OF PLANTING ACTIVITIES SHALL BE REPAIRED AND/OR REPLACED BY THE CONTRACTOR AT NO ADDITIONAL EXPENSE TO THE OWNER.
- TREE PROTECTION FENCING MUST BE IN PLACE PRIOR TO ANY DEMOLITION OR LAND DISTURBANCE.
- NO CHANGES TO THE LANDSCAPE DESIGN SHALL BE MADE WITHOUT THE WRITTEN CONSENT OF THE LANDSCAPE ARCHITECT.**
- ALL PLANTS INDICATED ON PLANS SHALL BE GROWN IN CLIMATES SIMILAR TO PROJECT AREA.
- PLANT QUANTITIES ARE FOR REFERENCE ONLY.** THE CONTRACTOR IS REQUIRED TO FULFILL THE INTENT OF THE DRAWINGS. ANY DISCREPANCIES IN THE NUMBER OF PLANTS SHOWN ON THE DRAWING AND THE PLANT LIST SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER PRIOR TO SUBMITTING A BID OR BEGINNING INSTALLATION.
- ALL PLANT MATERIAL SHALL CONFORM TO OR EXCEED THE AMERICAN STANDARD FOR NURSERY STOCK (LATEST EDITION) AS PUBLISHED BY THE AMERICAN ASSOCIATION OF NURSERMEN. **DO NOT INSTALL DAMAGED, WILTING OR UNSOUND PLANTS.**
- IN THE EVENT THAT A PROPOSED PLANT CANNOT BE ACQUIRED, SUBSTITUTION FOR GENUS, SIZE OR SPECIES SPECIFIED SHALL ONLY BE SUBSTITUTED WITH AN EQUIVALENT PLANT WHICH MUST BE APPROVED BY THE LANDSCAPE ARCHITECT IN WRITING.
- PLANTING MAY ONLY OCCUR DURING THE FOLLOWING PERIODS: MARCH 15 - JUNE 15 AND SEPTEMBER 15 - NOVEMBER 15. EXCEPTIONS WILL BE MADE BUT MUST BE APPROVED IN WRITING PRIOR TO ANY INSTALLATION. PROCEED WITH PLANTING ONLY WHEN EXISTING AND FORECAST WEATHER CONDITIONS PERMIT PLANTING.
- A PRE-EMERGENT HERBICIDE SHALL BE APPLIED TO THE SHRUB BEDS AND MULCH AREAS PRIOR TO THE INSTALLATION OF PLANT MATERIAL. HERBICIDES TO BE APPLIED PER MANUFACTURER'S SPECIFICATIONS. PROPOSED HERBICIDE SHALL BE APPROVED BY THE LANDSCAPE ARCHITECT AND OWNER PRIOR TO APPLICATION.
- TOPSOIL SHALL MEET ASTM D5265. HAVE A PH RANGE OF 5.5-7.0, CONTAIN A MINIMUM OF 4% ORGANIC MATERIAL, AND BE FREE OF ANY STONES OVER 1" IN ANY DIRECTION AND FREE OF DEBRIS AND NON-ORGANIC MATERIAL.** A SOIL SAMPLE SHALL BE TAKEN BY THE CONTRACTOR PRIOR TO PLANTING AND SENT TO A SOIL TESTING LABORATORY APPROVED BY THE STATE DEPARTMENT OF AGRICULTURE. **IF REQUIRED, SOIL AMENDMENT TYPES AND QUANTITIES TO BE SUBMITTED FOR REVIEW.**
- PRIOR TO INSTALLATION OF ANY PLANT MATERIAL, THE CONTRACTOR SHALL CONFIRM ADEQUATE DRAINAGE IN ALL PITS AND PLANT BEDS. NOTIFY THE LANDSCAPE ARCHITECT IF BEDROCK IS ENCOUNTERED OR IF PROPER DRAINAGE CANNOT BE ACHIEVED.
- PLANT TREES AND SHRUBS AFTER FINISH GRADES ARE ESTABLISHED AND BEFORE PLANTING LAWNS UNLESS OTHERWISE APPROVED BY THE LANDSCAPE ARCHITECT.
- PLANTING MIXES: PLANTING MIX FOR BACKFILLING PLANT PITS OF DECIDUOUS TREES AND SHRUBS SHALL BE IN THE PROPORTION OF FOUR PARTS BY VOLUME OF TOPSOIL AND ONE PART BY VOLUME OF PEAT MOSS PLUS ONE POUND OF 10-4 FERTILIZER PER CUBIC YARD. PLANTING MIX FOR BACKFILLING PLANT PITS OF NEEDLED EVERGREEN PLANTS SHALL BE IN THE PROPORTION OF TWO PARTS BY VOLUME OF TOPSOIL AND ONE PART BY VOLUME PEAT MOSS PLUS ONE POUND OF 10-4 FERTILIZER PER CUBIC YARD. ALL AMENDMENTS SHALL BE THOROUGHLY MIXED.
- REFER TO EROSION AND SEDIMENTATION CONTROL PLAN FOR LIMITS OF DISTURBANCE. ALL DISTURBED AREAS SHALL BE FINE GRADED AND SEEDED WITH PERMANENT SEED MIX UNLESS OTHERWISE NOTED ON PLANS. PRIOR TO INSTALLATION OF SOIL OR SEED, ALL LAWN AREAS SHALL RECEIVE TOPSOIL TO A DEPTH OF 4" AFTER LIGHT ROLLING. DO NOT COMPACT. THE AREAS SHALL BE DISHED AND RAKED SMOOTH TO A UNIFORM GRADE AS SHOWN ON THE GRADING PLAN.
- A 3" THICK DOUBLE SHREDED HARDWOOD MULCH BED SHALL BE PROVIDED IN ALL AREAS CONTAINING TREES, SHRUBS AND UNDER BUILDING OVERHANGS.
- PLANT IDENTIFICATION TAGS SHALL BE LEFT ON UNTIL INSTALLED PLANTINGS ARE INSPECTED BY THE LANDSCAPE ARCHITECT. ALL TAGS SHALL BE REMOVED AFTER APPROVAL.
- WARRANTY PERIOD FOR TREES AND SHRUBS IS ONE YEAR. WARRANTY PERIOD FOR GROUNDCOVER, LAWNS, GRASSES AND PERENNIALS IS 6 MONTHS. WARRANTY PERIOD FOR REPLACEMENTS SHALL BE EQUAL TO THE ORIGINAL WARRANTY PERIOD.
- A SATISFACTORY LAWN IS DEFINED AS A HEALTHY, UNIFORM CLOSE STAND OF GRASS FREE OF WEEDS AND LARGE IRREGULARITIES WITH COVERAGE EXCEEDING 90% OVER 10 SQUARE FEET WITH NO BARE SPOTS EXCEEDING A 5"X5" SQUARE.
- REFER TO LANDSCAPE DETAIL SHEETS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION PERTAINING TO INSTALLATION OF LANDSCAPE INCLUDING BUT NOT LIMITED TO MULCHING, STAKING AND SETTING SPACING OF PLANTS.

SOUTH FAYETTE TOWNSHIP LANDSCAPE REQUIREMENTS:

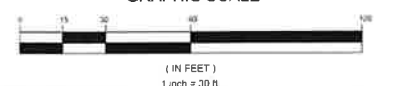
- §240-98 J (4) BUFFER AREAS AND LANDSCAPING**
IN ANY NONRESIDENTIAL DEVELOPMENT, DECIDUOUS TREES SHALL BE PLANTED IN ACCORDANCE OF THE FOLLOWING SCHEDULE. THESE TREES SHALL BE IN ADDITION TO THE TREES PROVIDED IN ANY REQUIRED BUFFER OR PARKING AREA.
BUILDING FOOTPRINT **REQUIREMENT**
30,001 - 75,000 sq ft A MINIMUM OF 30 TREES PLUS 1 TREE FOR EACH 3,000 sq ft OF BUILDING FOOTPRINT IN EXCESS OF 30,000 sq ft.
THE TREES SHALL BE PLANTED IN CLUSTERS ON THE SITE AND SHALL BE DISTRIBUTED THROUGHOUT THE SITE TO ENHANCE THE OPEN SPACE.
45,126 sq ft BUILDING: 35 TREES REQUIRED
35 TREES PROVIDED + 16 ADDITIONAL TREES PROVIDED
- §240-98 J (6)**
ALL TREES WHICH ARE REQUIRED TO BE PLANTED AS PER THE REGULATIONS OF THIS SUBSECTION J SHALL BE A MINIMUM OF TWO INCHES IN DIAMETER AT A POINT ONE FOOT ABOVE THE GROUND AT THE TIME OF PLANTING MEASURED ALONG THE TRUNK OF A PLANTED TREE, WHICH THE TREE SHALL BE PLANTED IN ACCORDANCE WITH ACCEPTED CONSERVATION PRACTICES.

PLANT SCHEDULE

TREES	QTY	BOTANICAL / COMMON NAME	CONDITION	SIZE
	27	Acer rubrum 'Franksred' / Red Sunburst Maple	B&B	2" CAL.
	8	Cercis canadensis / Eastern Redbud	B&B	2" CAL.
	1	Crataegus virdis 'Winter King' / Winter King Hawthorn	B&B	2" CAL.
	14	Quercus alba / White Oak	B&B	2" CAL.
SHRUBS	QTY	BOTANICAL / COMMON NAME	CONDITION	SIZE
	45	Calamagrostis x acutiflora 'Karl Foerster' / Karl Foerster Feather Reed Grass	#2 Cont.	18" HL.
	39	Cornus sericea 'SMNCSBD' / Arctic Fire® Yellow Dogwood	#5 Cont.	30" HT.
	115	Ilex glabra 'Shamrock' / Shamrock Holly	#5 Cont.	30" HT.
	15	Pennisetum alopecuroides 'Hameln' / Hameln Fountain Grass	#2 Cont.	
GROUND COVERS	QTY	BOTANICAL / COMMON NAME	CONT.	SIZE
	21	Echinacea purpurea 'Snow Cone' / Snow Cone Purple Coneflower	1 gal	16" dia.
	2,861	Liriope muscari 'Royal Purple' / Royal Purple Liriope	1 gal	12" dia.
	66	Rudbeckia fulgida / Coneflower	1 gal	16" dia.



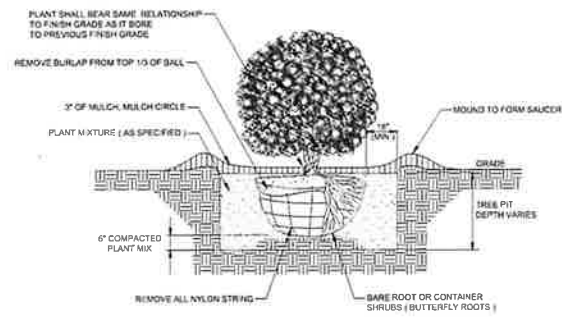
Know what's below.
Call before you dig.
Serial No. 20231760058



LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE-SF-LP
295 MYOMAR ROAD
IMARS, PA 16046

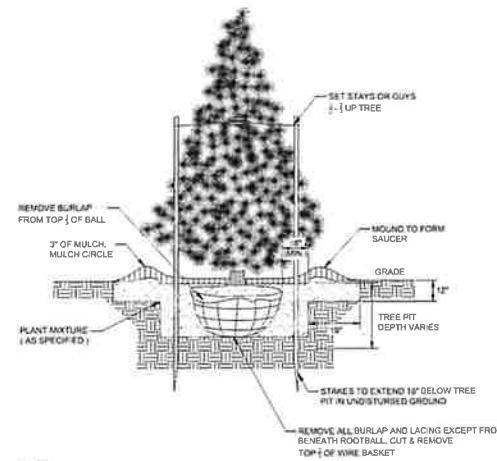
LANDSCAPE PLAN

Project Number: **C-12199-0025**
Drawing Scale: **1" = 30'**
Date Issued: **AUG 2025**
Index Number: _____
Drawn By: **MCL/RRR**
Checked By: **JMG**
Project Manager: **JMG**
L100



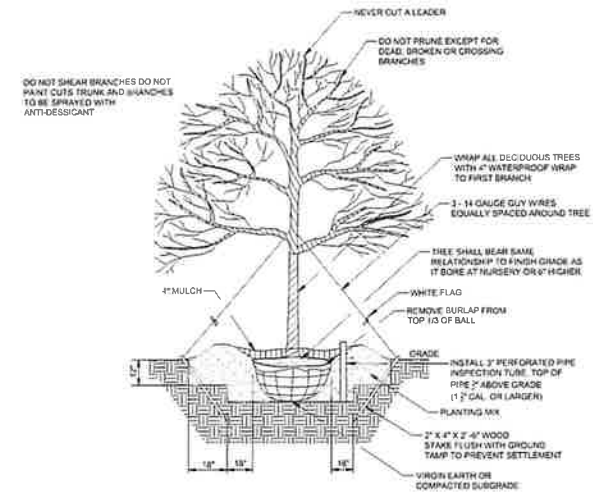
NOTES:
1. DO NOT PRUNE EVERGREENS EXCEPT TO REMOVE DEAD AND BROKEN BRANCHES

1	SHRUB PLANTING
L100	



NOTES:
1. STAKE ALL EVERGREEN TREES UNDER 12'
2. TREE SHALL BEAR SAME RELATION TO FINISHED GRADE AS IT BORE TO PREVIOUS GRADE
3. NEVER CUT LEADERS, 4. PRUNE ONLY TO REMOVE DAMAGED OR BROKEN BRANCHES

2	EVERGREEN TREE PLANTING
L100	



CUT AND REMOVE ALL BURLAP AND LACING EXCEPT FROM BENEATH ROOTBALL. CUT AND REMOVE TOP 1' OF WIRE BASKET. SLICE BURLAP WHERE BASKET REMAINS.

3	DECIDUOUS TREE PLANTING
L100	

Path: C:\ProgramData\Gateway Engineers\Projects\12199-0025\12199-0025-001-Landscape\12199-0025-001-Landscape-Planting.dwg
 Plot Date: 11/14/2025 1:02:17 PM
 Plot Device: HP DesignJet T1440PS3 (HP)

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REVISION RECORD

Date	No.	Description
2025-11-11	01	WPP Updates
2025-12-16	02	ACCD TECHNICAL NPDES RESPONSE
2026-01-14	03	TOWNSHIP SUBMISSION
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
 PREPARED FOR:
CE - SF, LP
 295 MYOMA ROAD
 MARKS, PA 16046

**LANDSCAPE PLAN
DETAIL**
 Project Number: C-12199-0025
 Drawing Scale: 1" = 30'
 Date Issued: AUG 2025
 Index Number: -
 Drawn By: MCL/RRR
 Checked By: JMG
 Project Manager: JMG
L101



Symbol	Qty	Label	Arrangement	Description	LLF	Luminaire Lumens	Luminaire Watts	Total Watts
	3	G2-5WQ	Single	GALN-SA2C-740-U-5WQ	0.912	14797	108	324
	2	GALN-T4FT	Single	GALN-SA2C-740-U-T4FT	0.912	14107	108	216
	6	GLN-SL3	Single	GALN-SA2C-740-U-SL3	0.912	14013	108	648

Calculation Summary								
Label	CalcType	Units	Avg	Max	Min	Avg/Min	Max/Min	
Parking Lot	Illuminance	Fc	0.79	3.4	0.2	3.95	17.00	

RP-8-21 Recommended Maintained Illuminance Values
Parking Lots (basic requirements, not for security lighting)

Application and Use	Grade	Description	Target Illuminance (fc) - Average Height	Target Illuminance (fc) - Minimum Height	Minimum & Vertical Spacing
All Parking Lots	0 Grade	Open Parking Facilities Type Four of Parking Garage All Activity Levels	0.30 FC @ 0.0'	0.10 FC @ 0.0'	30'
Drive Aides Parking Areas	0 Grade	Open Parking Facilities All Activity Levels	0.30 FC @ 0.0'	0.10 FC @ 0.0'	30'
Transaction Areas Pedestrian & Vehicle	0 Grade	Areas surrounding 1/2 hour apparel transaction area in all directions or curbs, periphery, bins, or structure	0.30 FC Min - Post-curve 0.20 FC Min - Post-curve	0.30 FC Min - Post-curve 0.20 FC Min - Post-curve	15'
Transaction Pedestrian General Areas	Varies	Vertical illuminance over entire front face of any structure. Coordinate with any machine display lighting requirements.	0.30 FC Min - Post-curve 0.20 FC Min - Post-curve	0.30 FC Min - Post-curve 0.20 FC Min - Post-curve	15'

Luminaire Location Summary					
LumNo	Label	Insertion Point			Orient
		X	Y	Z	
1	GLN-SL3	112	371	30	268.452
2	GLN-SL3	302	365	30	268.452
3	GLN-SL3	490	357	30	268.452
4	GALN-T4FT	633	343	30	268.409
5	GLN-SL3	20	222	30	0
6	GLN-SL3	186	169	30	87.436
7	G2-5WQ	356	168	30	90
8	GLN-SL3	697	168	30	151.699
9	G2-5WQ	537	161	30	90
10	G2-5WQ	566	63	30	242.103
11	GALN-T4FT	458	49	30	151.858

Applications, tasks, or viewing specific encountered on any given project may be different than these and may necessitate different criteria.
Table 17.2 RP-8-21

REVISION RECORD		
No	Date	Description
01	2025-11-11	WPP Updates
02	2025-12-16	ACCD TECHNICAL APDES RESPONSE
03	2026-01-14	TOWNSHIP SUBMISSION
04		
05		
06		
07		
08		

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031
PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARKS, PA 16046

LIGHTING PLAN
Project Number: C-12199-0025
Drawing Scale: 1" = 30'
Date Issued: AUG 2025
Index Number: --
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG
L200



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 Plot Orientation: Landscape
 Plot Range: All
 Plot Color: Black
 Plot Lineweight: 0.20
 Plot Font: Arial, 10
 Plot Style: Default.ctb
 Plot Device: HP DesignJet 500



REVISION RECORD

Date	No.	Description
2025-11-11	01	MPP Updates
2025-12-16	02	ACCD TECHNICAL INQUIRY RESPONSE
2026-01-14	03	TOWNSHIP SUBMISSION
	04	
	05	
	06	
	07	
	08	

LAFAYETTE 180
NEWBURY DRIVE
CUDDY, PA 15031

PREPARED FOR:
CE - SF, LP
295 MYOMA ROAD
MARS, PA 16846

LIGHTING DETAIL

Project Number: C-12199-0025
Drawing Scale: N/A
Date Issued: AUG 2025
Index Number:
Drawn By: MCL/RRR
Checked By: JMG
Project Manager: JMG

L201

McGraw-Edison GALN Galleon II

Optical Distributions

Product Specifications

Construction

- Die-cast aluminum housing and heat sink
- Three housing sizes, ranging 1 to 3 light sources

Optics

- High efficiency injection molded Acryl LED Optical Lens
- 17 beam distributions for area and roadway applications
- 3 beam type options include 155, 80 and 45°
- LED certified (2000 CCT and warmer only, beam mounting option)

Electrical

- Removable power entry terminal enclosure covers, large mounting and control modules for ease of maintenance and accessibility
- Standard with 120V wiring
- Standard with 120V surge module, optional 200V surge module

Typical Applications

- Outdoor parking lots, walkways, roadways, building areas
- Five year limited warranty

Lumen Maintenance Data

Temp	90% L70	95% L70	99% L70	90% L80	95% L80	99% L80	
25°C	98.4%	99.2%	98.5%	98.3%	+2.4kH	25°C	1.33
40°C	98.7%	98.2%	98.1%	97.4%	+1.9kH	40°C	1.31
50°C	98.7%	97.2%	96.6%	95.7%	+1.0kH	50°C	1.18
35°C	99.4%	99.0%	98.5%	98.3%	+2.4kH	40°C	1.19
40°C	99.1%	97.9%	97.7%	96.7%	+1.2kH	50°C	1.17

McGraw-Edison GALN Galleon II

Mounting Details

Adjustable Arm Range of Motion

- Includes the SP (SP) and PS (PS) beam type options
- Adjustable in increments of 5°
- Must maintain minimum lighting orientation

Wall Mount, Adjustable (WA)

2" Slighter, Adjustable (SP)

2-1/2" Slighter, Adjustable (SP)

Pole Mount, Adjustable Arm (PA)

Quick Mount Universal Arm (QU)

Quick Mount Mast Arm (QMA)

Pole Mount Arm with Quick Mount Adapter (QMA)

Mast Arm, Fixed (MA)

Spread Arm (SA)

Wall Mount, Fixed (WF)

Fiber Weights and DLAs

Beam Type	Number of Optics	1.4m	1.5m	1.6m	1.7m	1.8m	1.9m	2.0m
SP	1.4	33.9 lb (15.2 kg)	0.85	1.75	1.46	1.55	2.21	2.75
	1.6	43.9 lb (19.7 kg)	0.96	1.71	1.62	1.80	2.49	3.25
SP	1.4	33.9 lb (15.2 kg)	0.85	1.56	1.25	1.38	2.19	2.75
	1.6	43.9 lb (19.7 kg)	1.10	1.71	1.90	2.25	2.80	3.30
SP	1.4	33.9 lb (15.2 kg)	1.43	1.71	2.27	2.72	3.15	3.45
	1.6	43.9 lb (19.7 kg)	1.80	1.96	2.67	3.22	3.65	4.20
SP	1.4	33.9 lb (15.2 kg)	1.12	1.81	2.58	2.21	3.44	4.09
	1.6	43.9 lb (19.7 kg)	2.26	2.29	3.11	4.00	3.97	5.27
SP	1.4	33.9 lb (15.2 kg)	2.75	2.88	3.73	4.83	4.71	6.41
	1.6	43.9 lb (19.7 kg)	3.26	3.36	4.18	5.23	5.06	6.80
SP	1.4	33.9 lb (15.2 kg)	2.96	3.04	3.83	5.05	4.67	6.48
	1.6	43.9 lb (19.7 kg)	3.53	3.76	4.72	5.77	5.18	7.13
SP	1.4	33.9 lb (15.2 kg)	3.49	3.77	4.49	6.38	6.24	8.48
	1.6	43.9 lb (19.7 kg)	3.89	4.31	5.22	6.84	6.76	9.21
SP	1.4	33.9 lb (15.2 kg)	4.27	4.44	5.25	7.15	6.23	8.88
	1.6	43.9 lb (19.7 kg)	4.77	5.00	5.95	7.95	6.23	9.48

McGraw-Edison GALN Galleon II

Mounting Details

Pole Configuration Options

Pole Drilling Patterns

Type "N"

Type "M"

Quick Mount Universal Arm (QU)

Quick Mount Mast Arm (QMA)

Pole Mount Arm with Quick Mount Adapter (QMA)

Mast Arm, Fixed (MA)

Spread Arm (SA)

Wall Mount, Fixed (WF)

McGraw-Edison GALN Galleon II

Area / Site Luminaire

Product Features

- Interactive Menu
 - Ordering Information
 - Mounting Details
 - Optical Distributions
 - Product Specifications
 - Energy and Performance Data
 - Control Options
- Product Certifications: ETL, UL, IES, Energy Star, 30 Year, IP64

Quick Facts

- Lumen packages range from 3,300 - 73,500 (33W - 552W)
- 17 optical distributions
- Efficiency up to 159 lumens per watt

Connected Systems

- WaveLux Lite
- WaveLux

Dimensional Details

Standard Pole Mount Arm

Pole Drilling Pattern Type "N"

Beam Type	1.4m	1.5m	1.6m	1.7m	1.8m	1.9m	2.0m
SP	33.9 lb (15.2 kg)	0.85	1.75	1.46	1.55	2.21	2.75
SP	43.9 lb (19.7 kg)	0.96	1.71	1.62	1.80	2.49	3.25
SP	33.9 lb (15.2 kg)	0.85	1.56	1.25	1.38	2.19	2.75
SP	33.9 lb (15.2 kg)	1.10	1.71	1.90	2.25	2.80	3.30
SP	33.9 lb (15.2 kg)	1.43	1.71	2.27	2.72	3.15	3.45
SP	33.9 lb (15.2 kg)	1.80	1.96	2.67	3.22	3.65	4.20
SP	33.9 lb (15.2 kg)	1.12	1.81	2.58	2.21	3.44	4.09
SP	33.9 lb (15.2 kg)	2.26	2.29	3.11	4.00	3.97	5.27
SP	33.9 lb (15.2 kg)	2.75	2.88	3.73	4.83	4.71	6.41
SP	33.9 lb (15.2 kg)	3.26	3.36	4.18	5.23	5.06	6.80
SP	33.9 lb (15.2 kg)	2.96	3.04	3.83	5.05	4.67	6.48
SP	33.9 lb (15.2 kg)	3.53	3.76	4.72	5.77	5.18	7.13
SP	33.9 lb (15.2 kg)	3.49	3.77	4.49	6.38	6.24	8.48
SP	33.9 lb (15.2 kg)	3.89	4.31	5.22	6.84	6.76	9.21
SP	33.9 lb (15.2 kg)	4.27	4.44	5.25	7.15	6.23	8.88
SP	33.9 lb (15.2 kg)	4.77	5.00	5.95	7.95	6.23	9.48

McGraw-Edison GALN Galleon II

Control Options

Warranty: 5 Year Limited

Warranty: 5 Year Limited

Warranty

Warranty: 5 Year Limited

Warranty: 5 Year Limited

Warranty

Warranty: 5 Year Limited

Warranty: 5 Year Limited

Warranty

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Warranty: 5 Year Limited

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3D View 1
SCALE:



3D View 2
SCALE:



FRONT 3D VIEW 3
SCALE:



3D View 4
SCALE:



3D View 5
SCALE:



3D View 6
SCALE:



Project: New Retail Plaza - Shell Construction
LAFAYETTE 180
180 Miller's Run Road
(Bridgeville 15017)
South Fayette Township, PA

Title: 3D Perspective

No. 2025-15
Date: 12/19/2025
Revised:

A900



323 North Shore Drive
Suite 300
Pittsburgh, PA 15212

Ph: (412) 921 - 3303

January 14, 2026

Mr. John M. Barrett
Township Manager
South Fayette Township
100 Township Drive
South Fayette, PA 15017

Reference: TIS Addendum for the Proposed Retail Development in South Fayette Township, Allegheny County, PA (ePS No. 323385)

Dear Mr. Barrett:

David E. Wooster and Associates, LLC (Wooster) prepared a Transportation Impact Study (TIS) dated November 28, 2023 for a proposed retail development located on the northwest corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. By way of correspondence dated December 28, 2023, PennDOT Engineering District 11-0 (PennDOT) approved the TIS, which contemplated the construction of ~45,126 SF of retail space. Access to the site was (and is still) proposed to consist of three (3) site accesses:

- Two (2) right-in / right-out access site drives (Site Drives A and B) along the northern side of Millers Run Road (SR 0050).
- One (1) full access site drive (Site Drive C) along the western side of Newbury Drive.

Since that time, the applicant has modified the plan to include an additional ~13,000 SF of second-story office and warehousing/storage space. The overall building footprint and access to the site has not changed.

As such, the purpose of this correspondence is to demonstrate that the site plan change will not impact the results, findings, or conclusions of the approved TIS.

APPROVED STUDY CONDITIONS

The approved daily trips (ADT), morning (AM), evening (PM), and Saturday midday (SAT) peak hour trips associated with the ~45,126 SF of retail space are summarized in **Table 1** below. The data utilized to populate Table 1 is taken directly from Table 2 of the approved *Transportation Impact Study for the Proposed Retail Development* dated November 28, 2023 (2023 TIS) and is based on the Institute of Transportation Engineers’ (ITE’s) *Trip Generation Manual*, 11th Edition:

Table 1
Trip Generation Summary
Approved 2023 TIS Trips

TIME PERIOD	ANTICIPATED TRIP GENERATION		
	IN	OUT	TOTAL
LUC #821 – Shopping Plaza (40-150k) without Supermarket – 45,126 SF			
ADT	1,524	1,524	3,048
AM Peak Hour	48	30	78
PM Peak Hour	115	119	234
SAT Peak Hour	131	120	251

PROPOSED CONDITIONS

To evaluate traffic impacts associated with the site plan modification, Wooster utilized the Institute of Transportation Engineers’ (ITE’s) *Trip Generation Manual*, 12th Edition to re-evaluate the total trip generation potential of the site, as currently proposed. Specifically, Land Use Codes (LUCs) #821 – *40-150k Shopping Center without Supermarket*, #150 – *Warehouse*, and #710 – *General Office Building* were utilized. These trips are summarized in **Table 2** on the following page.

A copy of the trip generation calculations utilized to populate Table 2 can be found in the enclosures section at the end of this correspondence.

Table 2
Trip Generation Summary
Current Site Plan (2026)

TIME PERIOD	ANTICIPATED TRIP GENERATION		
	IN	OUT	TOTAL
LUC #821 – Shopping Plaza (40-150k) without Supermarket – 46,046 SF			
ADT	1,506	1,506	3,012
AM Peak Hour	45	28	73
PM Peak Hour	107	112	219
SAT Peak Hour	122	113	235
LUC #150 – Warehouse – 4,000 SF			
ADT	3	3	6
AM Peak Hour	0	0	0
PM Peak Hour	0	1	1
SAT Peak Hour	0	0	0
LUC #710 – General Office Building – 8,374 SF			
ADT	33	33	66
AM Peak Hour	9	1	10
PM Peak Hour	2	8	10
SAT Peak Hour	4	3	7
TOTAL			
ADT	1,542	1,542	3,084
AM Peak Hour	54	29	83
PM Peak Hour	109	121	230
SAT Peak Hour	126	116	242

TRIP GENERATION COMPARISON

Table 3 below depicts the net change in site-generated trips associated with the proposed site plan change.¹

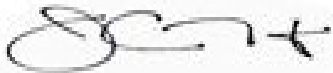
Table 3
Trip Generation Comparison

TIME PERIOD	ANTICIPATED TRIP GENERATION		
	IN	OUT	TOTAL
ADT	+18	+18	+36
AM Peak Hour	+6	-1	+5
PM Peak Hour	-6	+2	-4
SAT Peak Hour	-5	-4	-9

As demonstrated in the table, the change in projected daily and peak hour trips associated with the site plan change is *de minimis* and will therefore have no impact on the results, findings, or conclusions of the 2023 TIS.²

We trust that this correspondence has been submitted in the proper form. If you have any questions or require additional information, please feel free to contact me.

Sincerely,



Joshua A. Haydo, P.E., PTOE
Senior Traffic Engineer / Deputy COO

Enclosures: ITE Trip Generation Calculations

W:\4392 011426 TIS Addendum

¹ Trips depicted in Table 3 were derived by subtracting the approved TIS trips (Table 1) from the trips associated with the current plan (Table 2).

² Despite the increase in overall square footage, some analysis periods experience a *decrease* in projected site-generated trips. This is due to modifications to various trip generation rates in the 12th Edition of the ITE *Trip Generation Manual* (the 2023 TIS utilized the 11th Edition, which was the current version of the manual at that time).

Shopping Plaza (40-150k) - Supermarket - No (821)

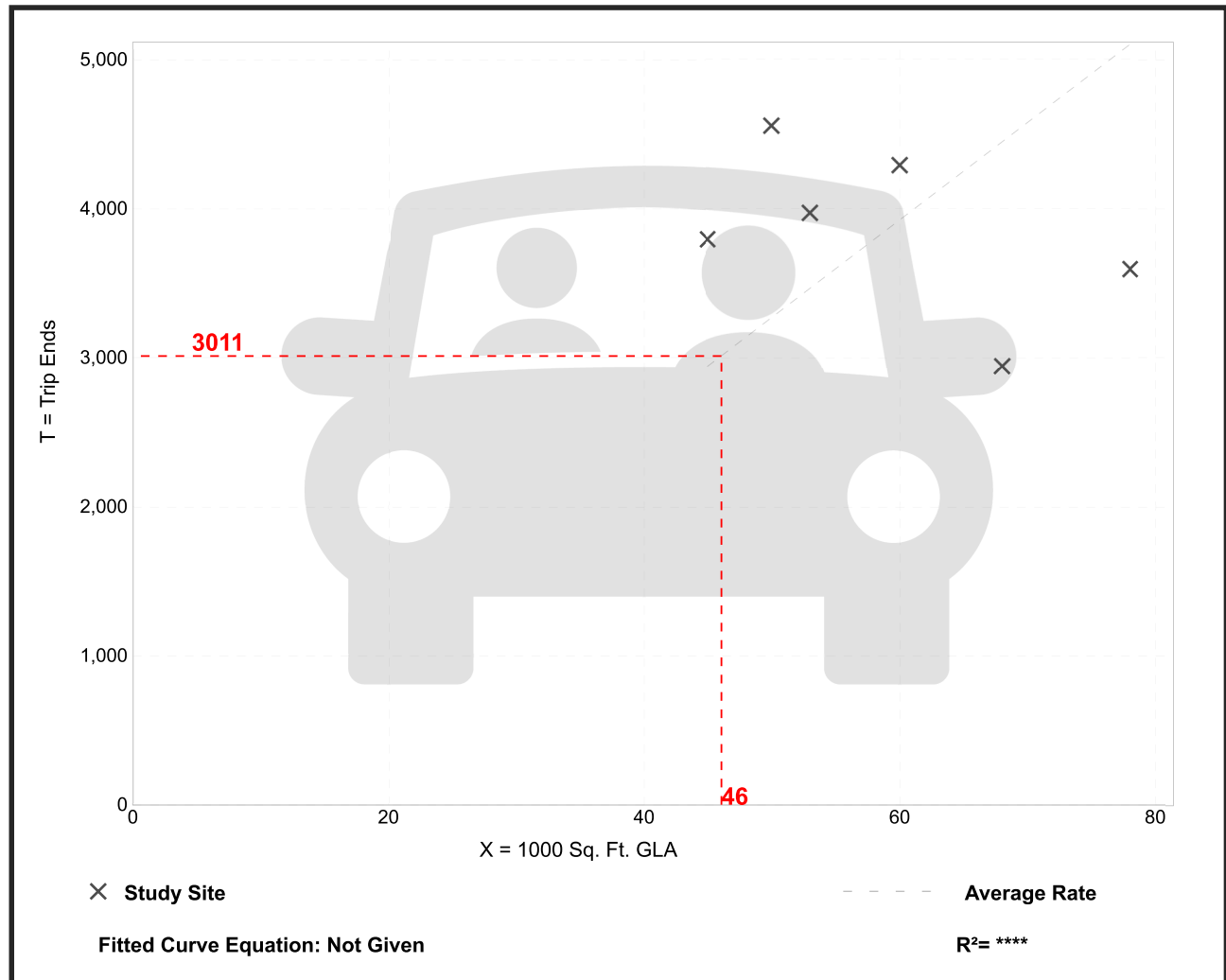
Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 6
Avg. 1000 Sq. Ft. GLA: 59
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
65.38	43.29 - 91.06	20.03

Data Plot and Equation



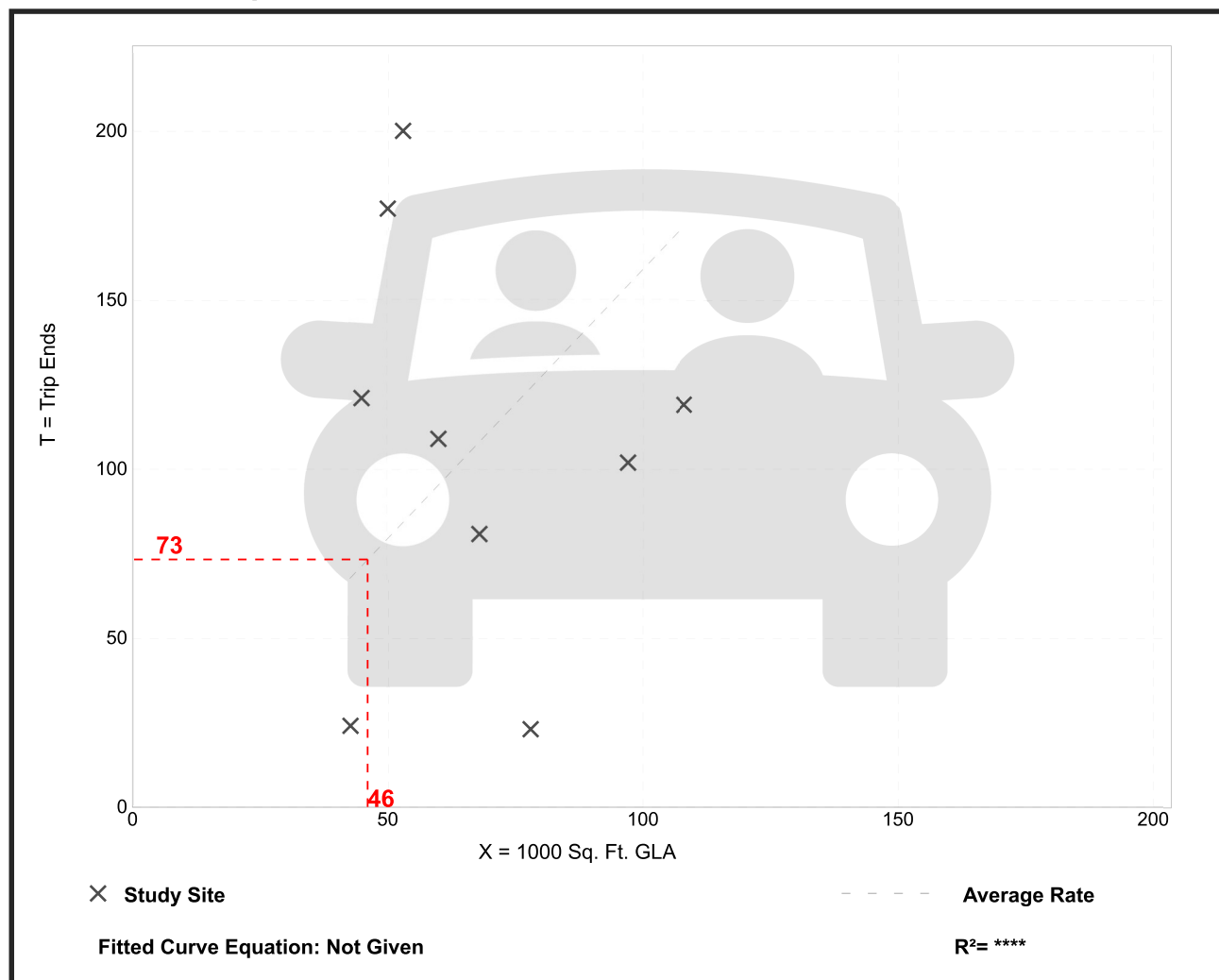
Shopping Plaza (40-150k) - Supermarket - No (821)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 9
 Avg. 1000 Sq. Ft. GLA: 67
 Directional Distribution: 62% entering, 38% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
1.59	0.29 - 3.77	1.18

Data Plot and Equation



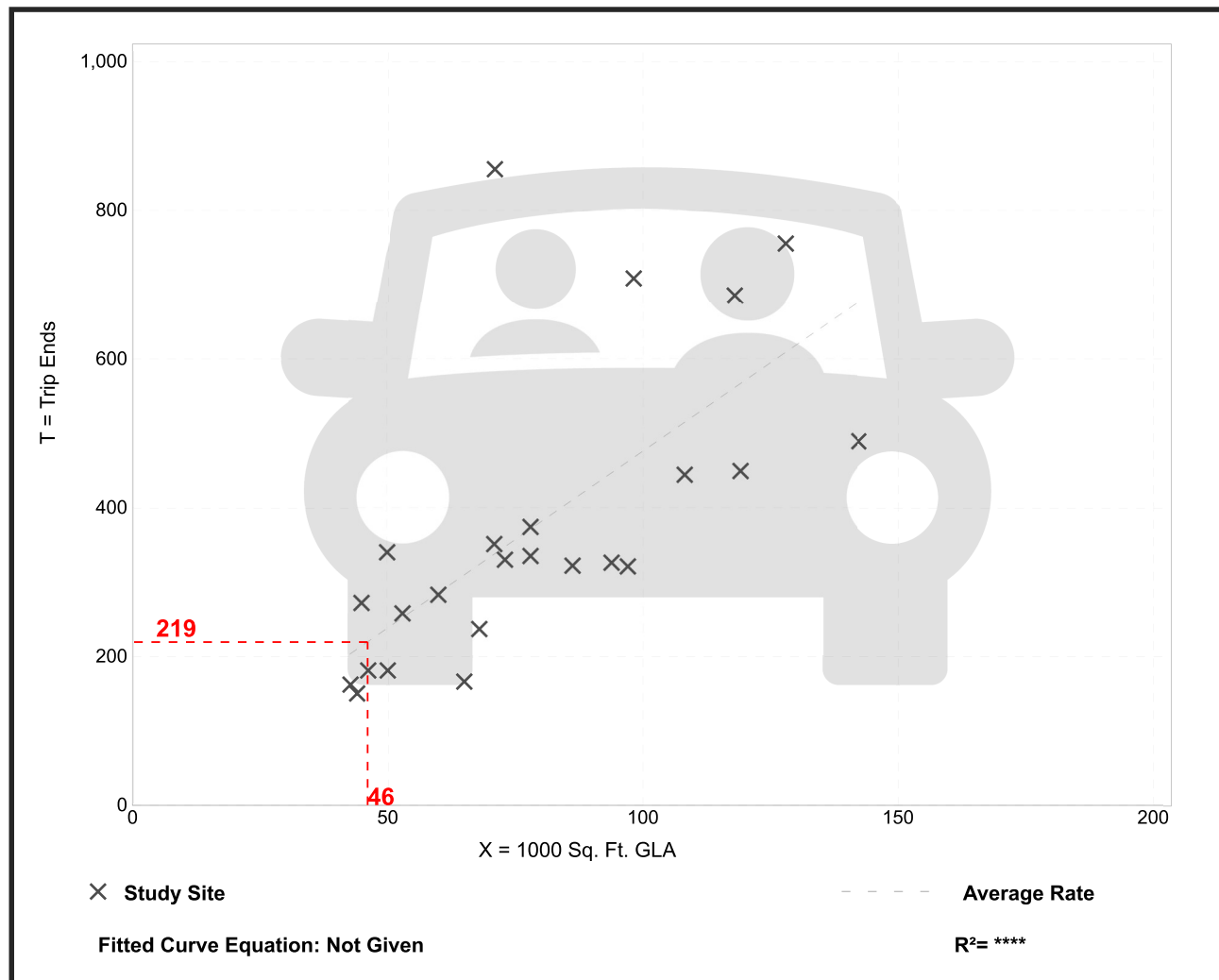
Shopping Plaza (40-150k) - Supermarket - No (821)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 24
 Avg. 1000 Sq. Ft. GLA: 79
 Directional Distribution: 49% entering, 51% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
4.76	2.55 - 12.04	1.89

Data Plot and Equation



Shopping Plaza (40-150k) - Supermarket - No (821)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
 On a: Saturday, Peak Hour of Generator

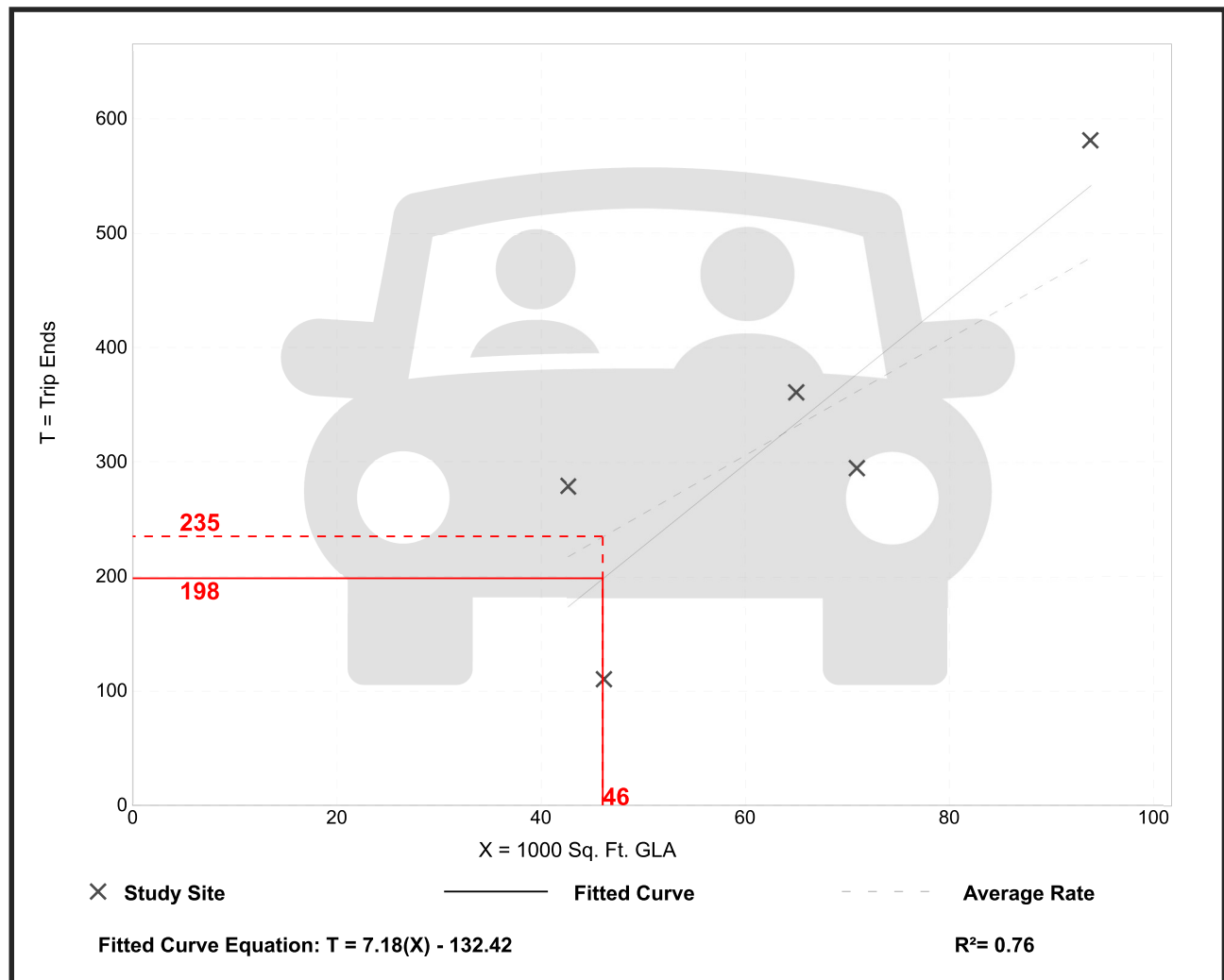
Setting/Location: General Urban/Suburban
 Number of Studies: 5
 Avg. 1000 Sq. Ft. GLA: 64
 Directional Distribution: 52% entering, 48% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
5.10	2.38 - 6.53	1.56

Data Plot and Equation

Caution – Small Sample Size



Warehouse (150)

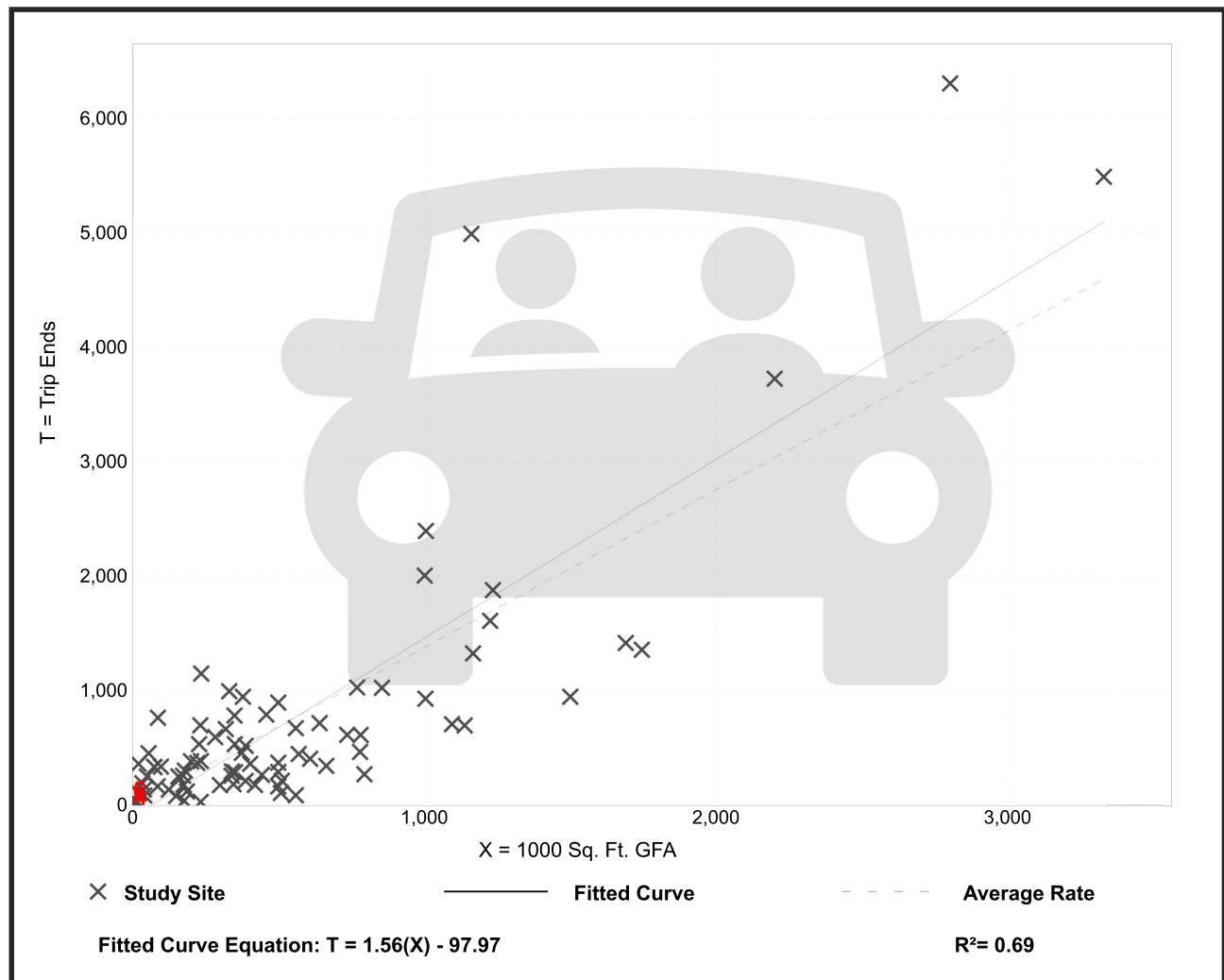
Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 81
Avg. 1000 Sq. Ft. GFA: 554
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.38	0.15 - 16.93	1.05

Data Plot and Equation



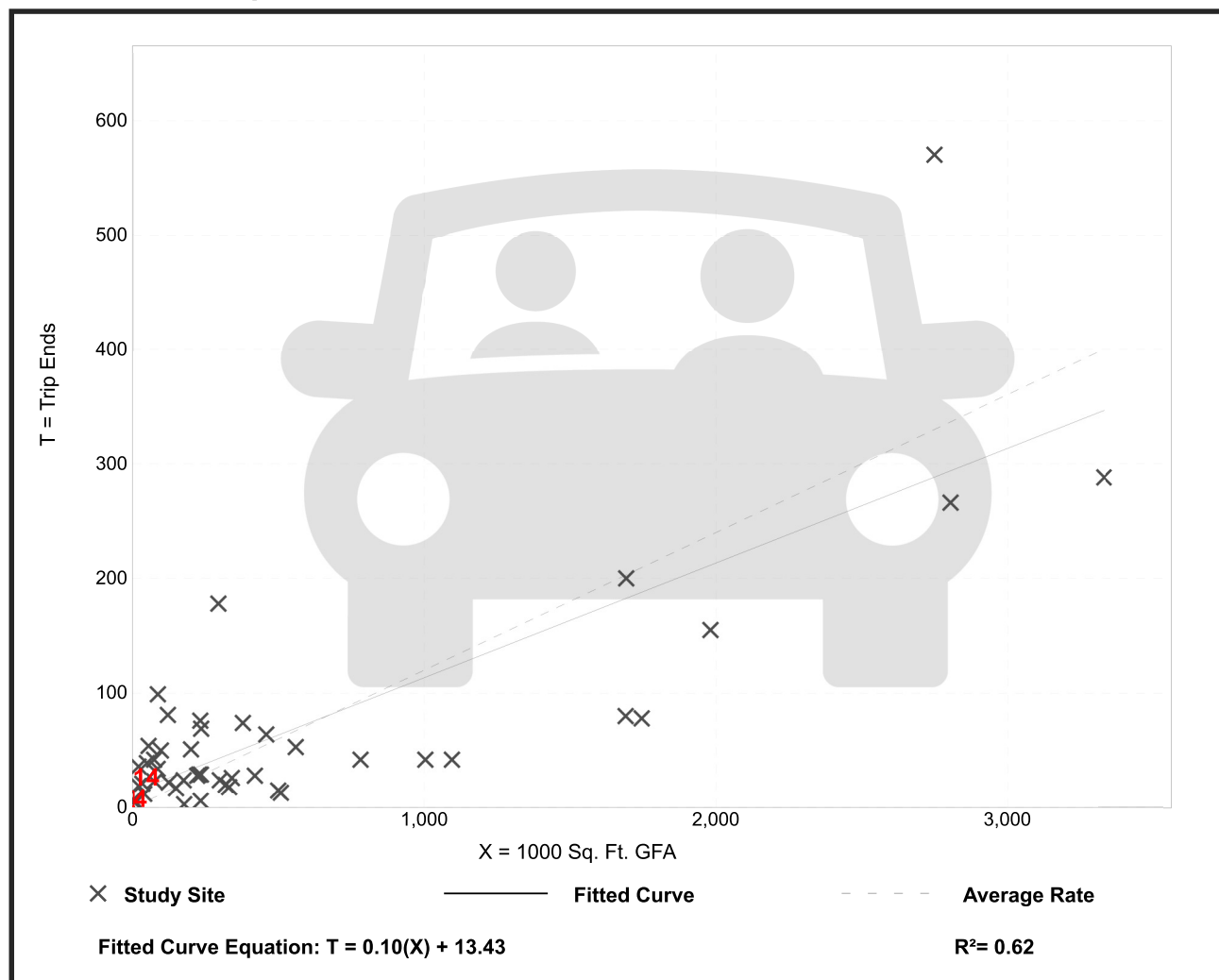
Warehouse (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 47
 Avg. 1000 Sq. Ft. GFA: 559
 Directional Distribution: 77% entering, 23% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.12	0.02 - 1.80	0.14

Data Plot and Equation



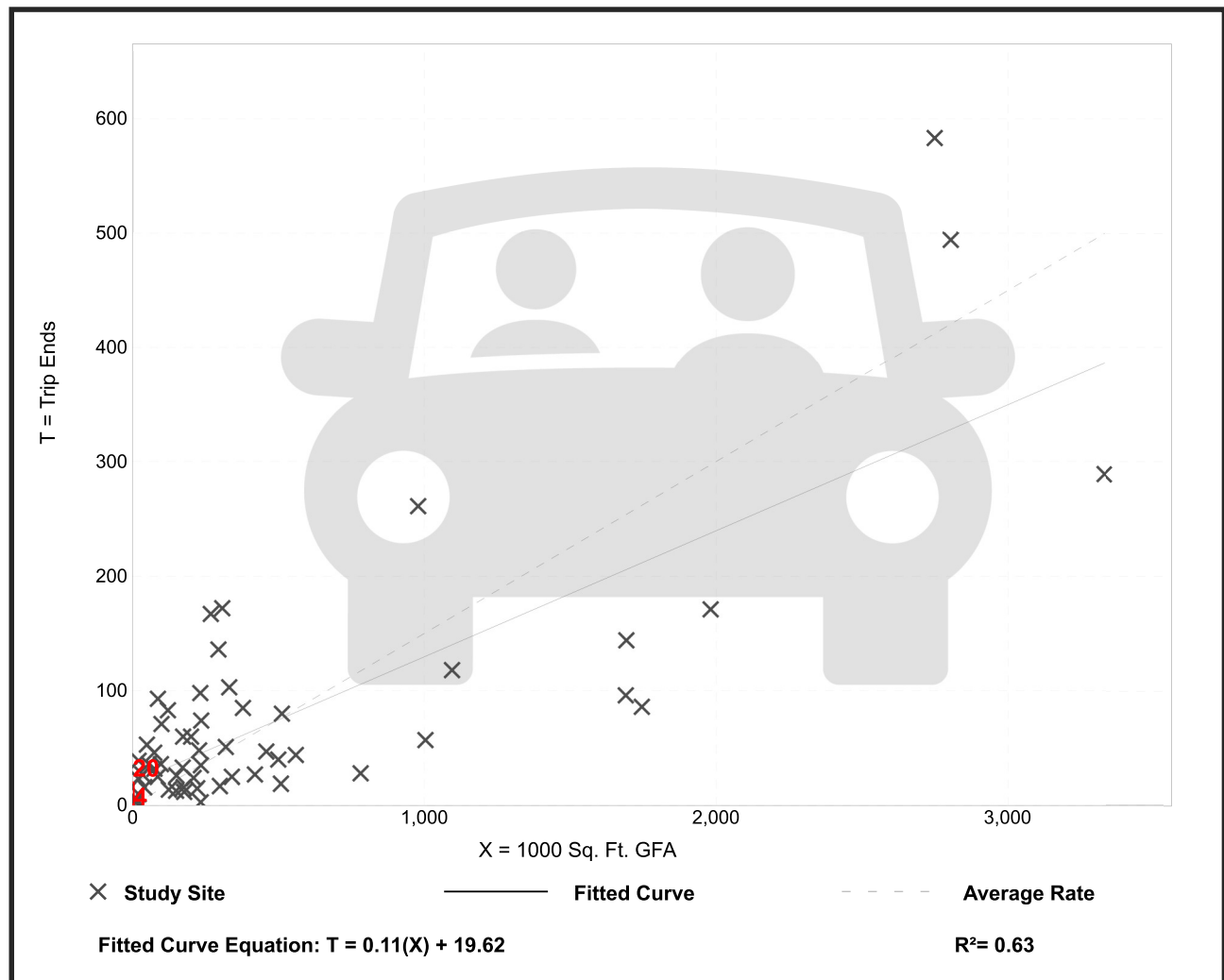
Warehouse (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 58
 Avg. 1000 Sq. Ft. GFA: 503
 Directional Distribution: 28% entering, 72% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.15	0.01 - 1.80	0.15

Data Plot and Equation



Warehouse (150)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Saturday, Peak Hour of Generator

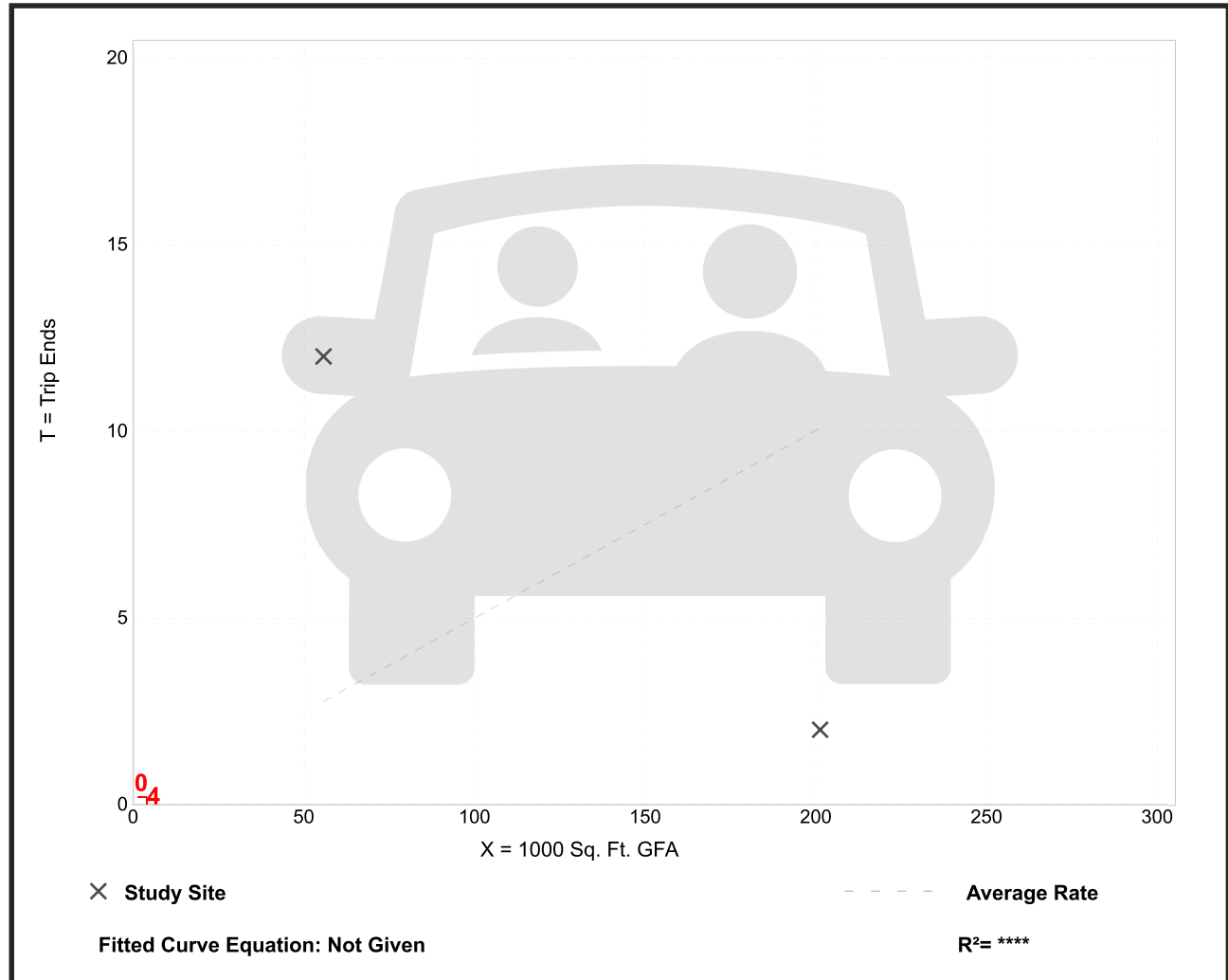
Setting/Location: General Urban/Suburban
Number of Studies: 2
Avg. 1000 Sq. Ft. GFA: 129
Directional Distribution: 64% entering, 36% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.05	0.01 - 0.22	*

Data Plot and Equation

Caution – Small Sample Size



General Office Building (710)

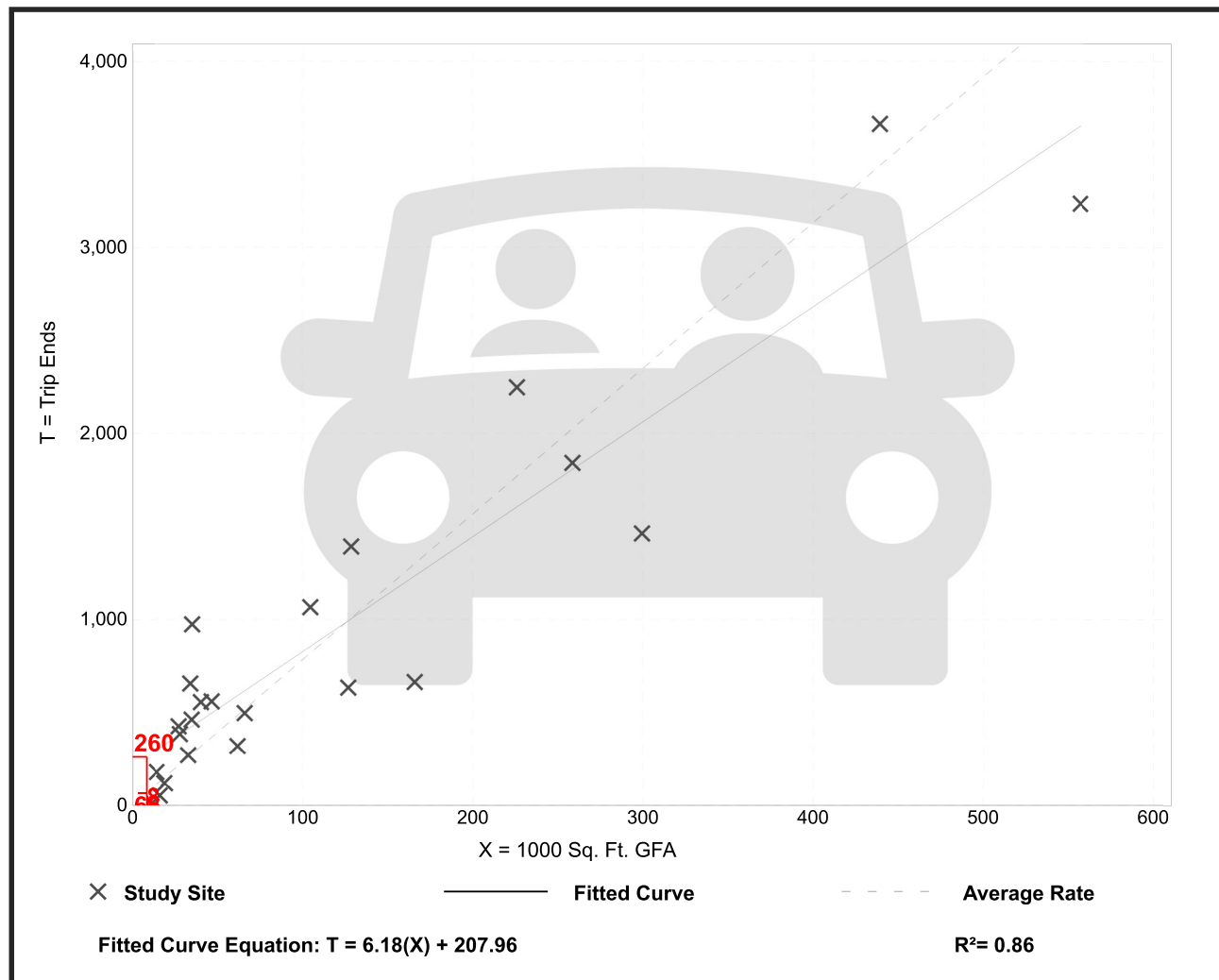
Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 22
Avg. 1000 Sq. Ft. GFA: 126
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
7.83	3.27 - 27.56	3.71

Data Plot and Equation



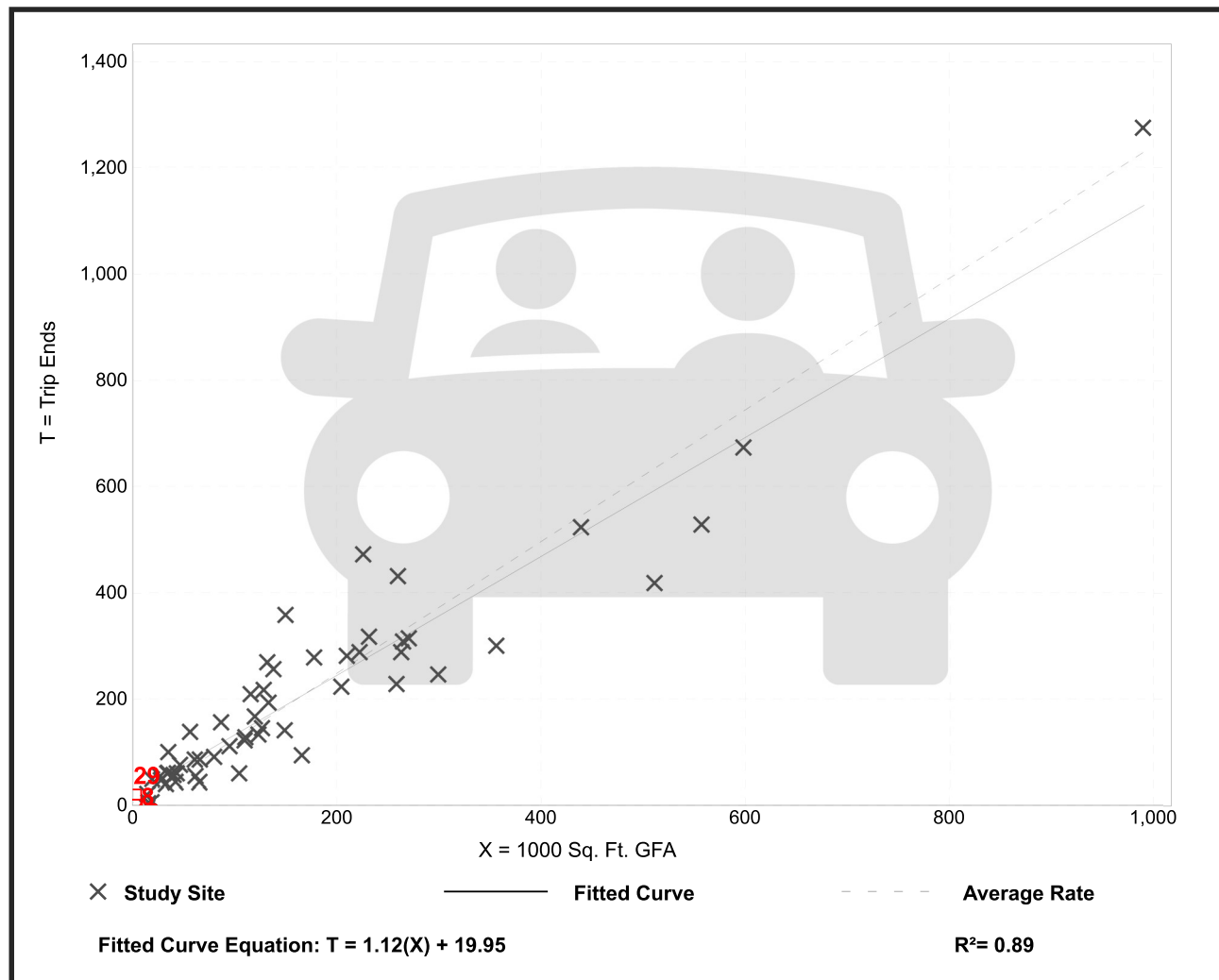
General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 54
 Avg. 1000 Sq. Ft. GFA: 170
 Directional Distribution: 88% entering, 12% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.24	0.32 - 2.83	0.40

Data Plot and Equation



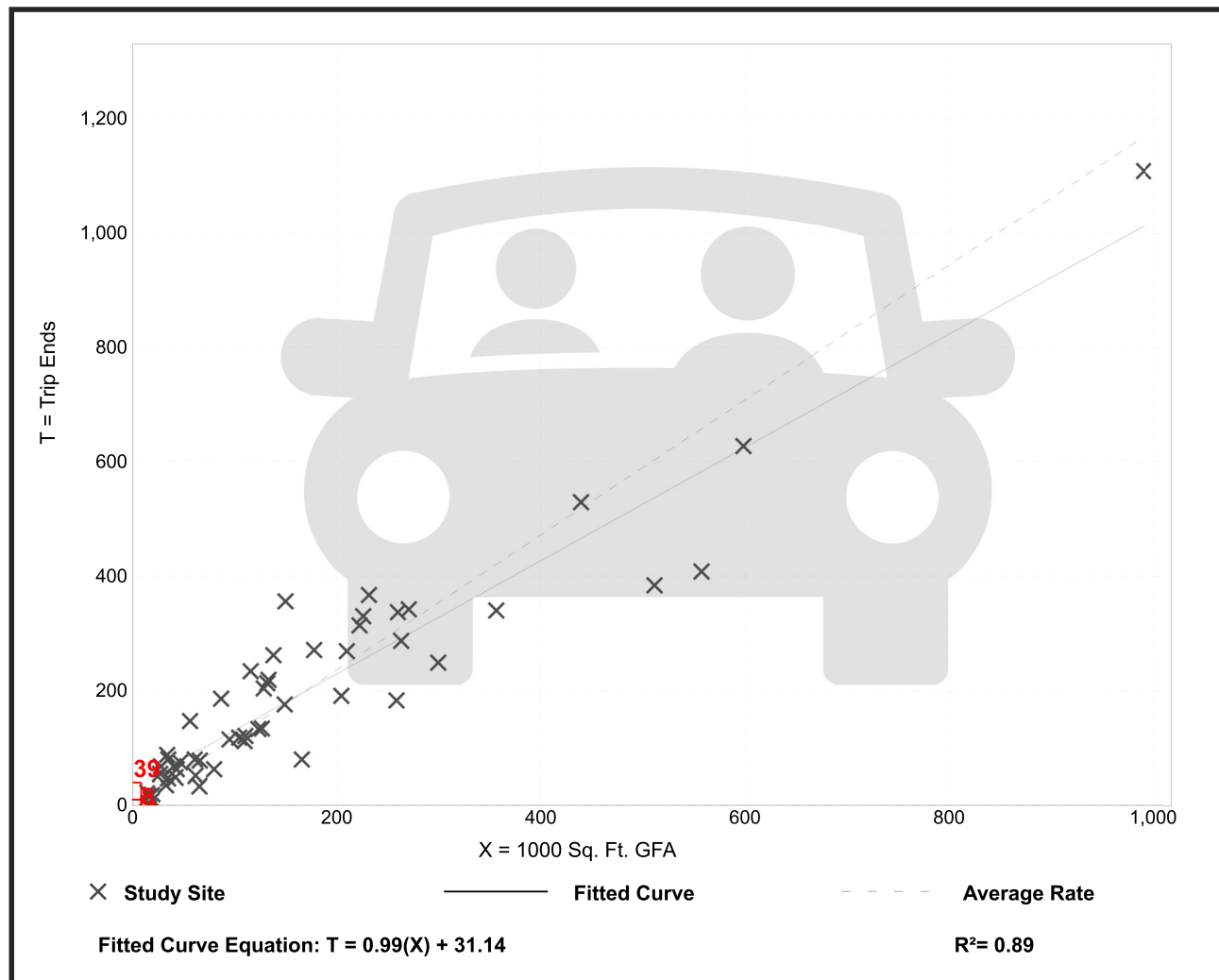
General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 53
 Avg. 1000 Sq. Ft. GFA: 166
 Directional Distribution: 16% entering, 84% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.18	0.26 - 2.59	0.41

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Saturday, Peak Hour of Generator

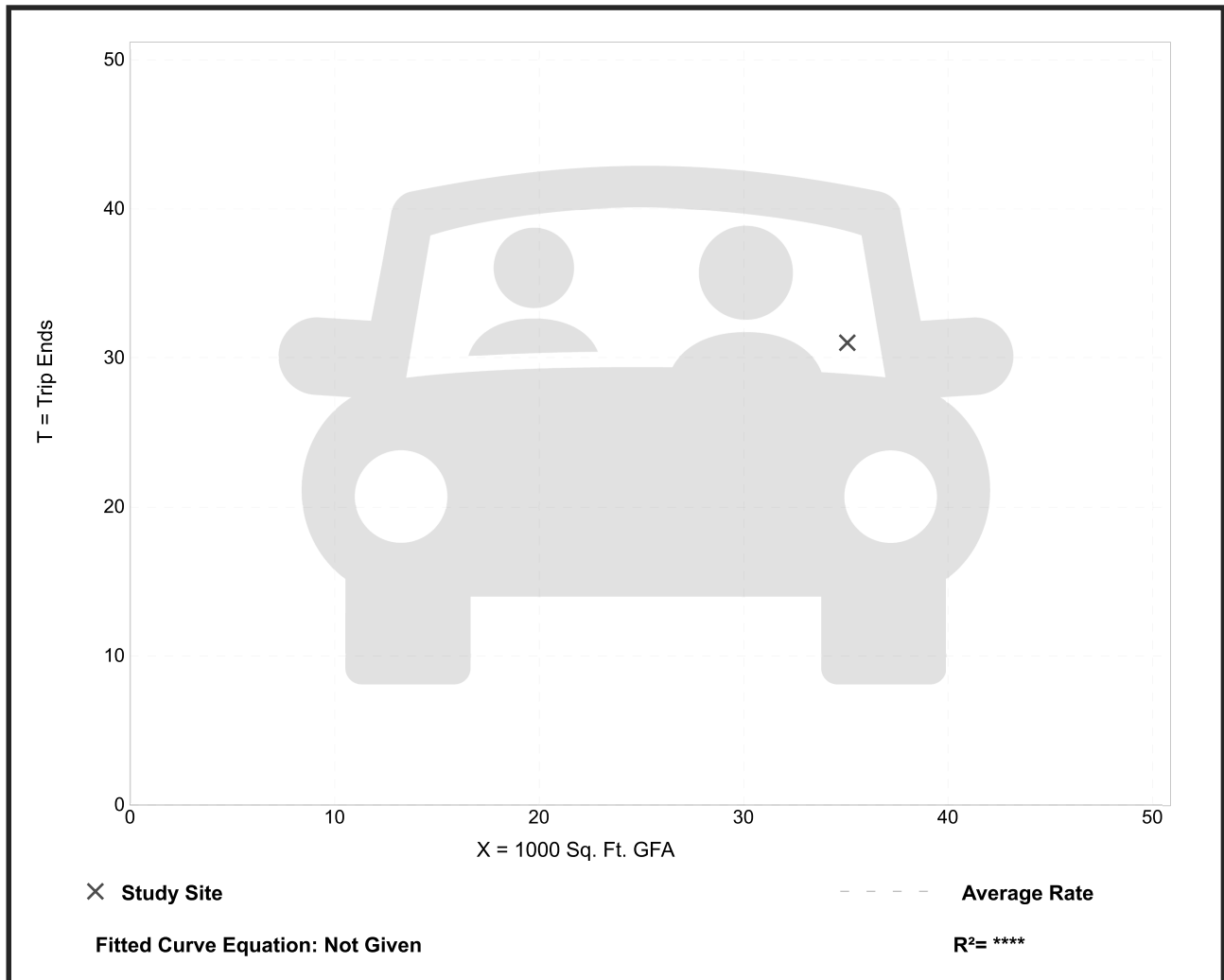
Setting/Location: General Urban/Suburban
 Number of Studies: 1
 Avg. 1000 Sq. Ft. GFA: 35
 Directional Distribution: 54% entering, 46% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.88	0.88 - 0.88	*

Data Plot and Equation

Caution – Small Sample Size



**TRANSPORTATION
IMPACT STUDY**

for the proposed

**RETAIL
DEVELOPMENT**

South Fayette Township, Allegheny County, Pennsylvania

October 17, 2023

**TRANSPORTATION
IMPACT STUDY**

for the proposed

**RETAIL
DEVELOPMENT**

South Fayette Township, Allegheny County, PA

October 17, 2023

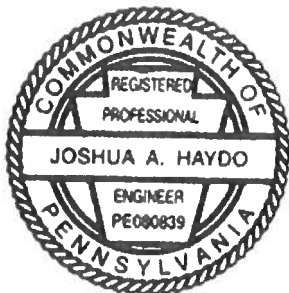
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Two East Crafton Avenue
Pittsburgh, PA 15205

Project Engineer(s): Jesse Nelson & Suleiman A. Swai, P.E.

Supervising Engineer: Joshua A. Haydo, P.E., PTOE

Seal



JCH

10/17/23

Date

TABLES

Table 1 – Level-of-Service Summaries

- 1A – Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
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- 1C – Millers Run Road (SR 0050) with Site Drive A
- 1D – Millers Run Road (SR 0050) with Site Drive B

Table 2 – Trip Generation Summary

Table 3 – Queuing Summaries

- 3A – Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- 3B – Newbury Drive with Plaza Access / Site Drive C
- 3C – Millers Run Road (SR 0050) with Site Drive A
- 3D – Millers Run Road (SR 0050) with Site Drive B

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Figure 1 – Site Location Graphic

Figure 2 – Preliminary Site Plan

Figure 3a – Existing Year 2023 Condition Peak Hour Traffic Volumes

Figure 3b – Existing Year 2023 Condition Peak Hour LOS

Figure 4 – Opening Year 2024 Base Condition Peak Hour Volumes

Figure 5 – Design Year 2029 Base Condition Peak Hour Volumes

Figure 6a – Primary Trip Distribution Percentages (The Piazza Retail Development)

Figure 6b – Primary Trips (The Piazza Retail Development)

Figure 7a – Pass-By Trip Distribution Percentages (The Piazza Retail Development)

Figure 7b – Pass-By Trips (The Piazza Retail Development)

Figure 8a – Primary Trip Distribution Percentages (South Fayette Commons Development)

Figure 8b – Primary Trips (South Fayette Commons Development)

Figure 9a – Primary Trip Distribution Percentages (Newbury: Cigar Lounge, Bar & Restaurant Development)

Figure 9b – Primary Trips (Newbury: Cigar Lounge, Bar & Restaurant Development)

Figure 10 – Total Background Trips

Figure 11a – Opening Year 2024 Without Development Condition Peak Hour Volumes

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Figure 12a – Design Year 2029 Without Development Condition Peak Hour Volumes

Figure 12b – Design Year 2029 Without Development Condition Peak Hour LOS

Figure 13a – Primary Trip Distribution Percentages

Figure 13b – Site-Generated Primary Trips

Figure 14 – Intersection Distribution Percentages

Figure 15a – Pass-By Trip Distribution Percentages (from North)

Figure 15b – Pass-By Trip Distribution Percentages (from South)

Figure 15c – Pass-By Trip Distribution Percentages (from East)

Figure 15d – Pass-By Trip Distribution Percentages (from West)

Figure 16a – Pass-By Trip Distribution Percentages (Total)

Figure 16b – Site-Generated Pass-By Trips

Figure 17 – Total Site-Generated Trips

Figure 18a – Opening Year 2024 With Development Condition Peak Hour Volumes

Figure 18b – Opening Year 2024 With Development Condition Peak Hour LOS

Figure 19a – Design Year 2029 With Development Condition Peak Hour Volumes

Figure 19b – Design Year 2029 With Development Condition Peak Hour LOS

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- A. TIS Scoping Checklist
- B. Turning Movement Count Data
- C. Photo Log of Existing Study Intersections
- D. Traffic Signal Permit Plans
- E. Level of Service (LOS) Criteria Summary
- F. Synchro Printouts – *Existing Year 2023 Condition*
- G. HCM Printouts – *Existing Year 2023 Condition*
- H. Source Data for Background Developments
- I. Synchro Printouts – *Opening Year 2024 Without Development Condition*
- J. HCM Printouts – *Opening Year 2024 Without Development Condition*
- K. Synchro Printouts – *Design Year 2029 Without Development Condition*
- L. HCM Printouts – *Design Year 2029 Without Development Condition*
- M. Trip Generation Calculations
- N. Synchro Printouts – *Opening Year 2024 With Development Condition*
- O. HCM Printouts – *Opening Year 2024 With Development Condition*
- P. Synchro Printouts – *Design Year 2029 With Development Condition*
- Q. HCM Printouts – *Design Year 2029 With Development Condition*
- R. Radar Speed Study
- S. Sight Distance Photo Log
- T. SimTraffic Printouts – *Design Year 2029 Without Development Condition*
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**Transportation Impact Study
Proposed Retail Development
South Fayette Township, Allegheny County, Pennsylvania**

EXECUTIVE SUMMARY

Project Description

The proposed project is located on the northwestern corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. The development is proposed to consist of ~45,126-square foot of retail space.

This report analyzes the impact of this project on the traffic operations of the adjacent roadway network under:

- Existing Year 2023 Condition
- Opening Year 2024 Without and With Development Conditions
- Design Year 2029 Without and With Development Conditions

Existing and Future Without Development Conditions

A linear growth rate of 1.00% was obtained from a representative of the Southwestern Pennsylvania Commission (SPC) for South Fayette Township. This rate was applied to the Existing Year 2023 Condition peak hour traffic volumes to develop the Opening Year 2024 and Design Year 2029 Base Condition traffic volumes.

Trip Generation and Distribution

Trip Generation

The Institute of Transportation Engineers' (ITE's) *Trip Generation Manual*, 11th Edition, was used to determine the trip generation rates associated with the proposed development and background development trips (specifically, Land Use Codes #821 – *Shopping Plaza 40k-150k without Supermarket*, #931 – *Fine Dining Restaurant*, #932 – *High-Turnover Sit-Down Restaurant*, and #934 – *Fast Food Restaurant with Drive-Through Window*). The rates for LUC #821 were utilized to determine the additional trips anticipated to be generated by the proposed development on a typical weekday and during the AM, PM, and SAT peak hours, which are summarized as follows:

- 3,048 additional vehicles during a typical weekday (1,524 entering and 1,524 existing)

- 78 additional vehicles during the AM peak hour (48 entering and 30 exiting)
- 234 additional vehicles during the PM peak hour (115 entering and 119 exiting)
- 251 additional vehicles during the SAT peak hour (131 entering and 120 exiting)

Trip Distribution

The trip generation data, the distribution of traffic at the existing study intersections, and the location of the proposed development were all used to assign the site-generated traffic to the study area roadway network.

List of Study Intersections

The study area for this project includes two (2) existing intersections and two (2) proposed intersections:

- Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- Newbury Drive with Plaza Access / Site Drive C
- Millers Run Road (SR 0050) with Site Drive A
- Millers Run Road (SR 0050) with Site Drive B

Conclusions and Recommendations

The proposed retail development in South Fayette Township, Allegheny County, Pennsylvania is not anticipated to impact traffic operations in the study area. The following is a summary of the results and recommended improvements at each of the study intersections:

Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 2.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- No roadway improvements are recommended.

Newbury Drive with Plaza Access / Site Drive C

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 10.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- Construct a full access site drive (Site Drive C) along the western side of Newbury Drive, as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive A

- Construct a right-in / right-out access site drive (Site Drive A) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive B

- Construct a right-in / right-out access site drive (Site Drive B) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

**Transportation Impact Study
Proposed Retail Development
South Fayette Township, Allegheny County, Pennsylvania**

1.0 INTRODUCTION / PROJECT SUMMARY

David E. Wooster and Associates (Wooster) has completed a Transportation Impact Study (TIS) to determine the impacts to traffic operations resulting from the construction of a proposed retail development located on the northwestern corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. The development is proposed to consist of ~45,126-square foot of retail space.

A site location graphic is shown on **Figure 1** and a preliminary Site Plan can be seen on **Figure 2** in the Figures section at the end of this report. The project is anticipated to open by the end of 2024. As such, this report analyzes the impact of this project on the traffic operations of the adjacent roadway network under:

- Existing Year 2023 Condition
- Opening Year 2024 Without and With Development Conditions
- Design Year 2029 Without and With Development Conditions

Traffic volumes have been developed for each scenario, capacity analyses have been performed, and the results have been reported in terms of both Level-of-Service (LOS) and average delay per vehicle. The capacity analyses contained in this report were performed using Synchro Software Version 11 (Synchro).

A virtual TIS Scoping Meeting was held on Thursday, September 14, 2023 and was attended by representatives of PennDOT Engineering District 11-0 (PennDOT), South Fayette Township, the applicant, and Wooster. A copy of the TIS Scoping Checklist has been included in **Appendix A** at the end of this report.

2.0 DATA COLLECTION

2.1 Turning Movement Counts

Turning movement counts were performed at the existing study intersections on a typical weekday (Tuesday through Thursday) between the hours of 7:00 a.m. and 9:00 a.m. and between the hours of 4:00 p.m. and 6:00 p.m. Counts were also performed on a typical Saturday between the hours of 11:00 a.m. and 2:00 p.m. These times were chosen because they typically reflect the morning (AM), evening (PM), and Saturday midday (SAT) peak hours for vehicular traffic. These counts were performed in September 2023.

Summaries of the turning movement counts can be found in **Appendix B** at the end of this report. This information was then utilized to determine the Existing Year 2023 Condition peak hour traffic volumes, which can be seen graphically on **Figure 3a**.

2.2 *Crash Data*

Copies of crash data summaries for the five (5) most recent calendar years were obtained from the Pennsylvania Department of Transportation (PennDOT) Crash Information Tool (PCIT) for the existing study intersections. As the crash data is property of PennDOT, a summary of the identified crashes and the corresponding crash reports are included in a separately-bound appendix to this report.

3.0 STUDY AREA CONDITIONS

3.1 *Study Area*

The study area for this project includes two (2) existing intersections and two (2) proposed intersections:

- Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- Newbury Drive with Plaza Access / Site Drive C
- Millers Run Road (SR 0050) with Site Drive A
- Millers Run Road (SR 0050) with Site Drive B

3.2 *Existing Study Intersection Descriptions*

3.2.1 *Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive*

The intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive is a signalized intersection with four (4) approaches. The eastbound approach (Millers Run Road) consists of an exclusive left turn lane, an exclusive through lane, and a shared through / right turn lane. The westbound approach (Millers Run Road) consists of an exclusive left turn lane, two (2) exclusive through lanes, and an exclusive, channelized right turn lane, which is yield controlled. The posted speed limit on Millers Run Road (SR 0050) is 40 mph. The northbound approach (Todd A. Miller Drive) consists of an exclusive left turn lane, an exclusive through lane, and an exclusive, channelized right turn lane, which is yield controlled. There is no posted speed limit on Todd A. Miller Drive.¹ The southbound approach (Newbury Drive) consists of two (2) exclusive left turn lanes and a shared through / right turn lane. There is no posted speed limit on Newbury Drive.

¹ HCM 6th Edition supports only speed limits within the range of 25-55 mph. As such, a speed limit of 25 mph was assumed for all roadways without a posted speed limit or a speed limit less than 25 mph.

3.2.2 Newbury Drive with Plaza Access / Site Drive C

The intersection of Newbury Drive with Plaza Access / Existing Site Access² is an unsignalized intersection with four (4) approaches. The eastbound approach (Existing Site Access) consists of a single lane that is used to perform all possible movements and is stop-controlled. There is no posted speed limit on the Existing Site Access. The westbound approach (Plaza Access) consists of a single lane that is used to perform all possible movements and is stop-controlled. There is no posted speed limit on the Plaza Access. The northbound approach (Newbury Drive) consists of a shared left turn / through lane and a shared through / right turn lane, which operate under free-flow conditions. The southbound approach (Newbury Drive) consists of a shared left turn / through lane and a shared through / right turn lane, which operate under free-flow conditions. There is no posted speed limit on Newbury Drive.

A complete photo log of the existing study intersections can be found in **Appendix C** at the end of this report.

3.3 Signal Permit Plans

Traffic signal permit plans were requested for the signalized study intersection from a representative of PennDOT. The signal permit plans were utilized to model the Existing Year 2023 Condition in Synchro.

Copies of the signal permit plans have been included in **Appendix D** at the end of this report.

3.4 Existing Condition Capacity Analysis

The capacity analyses contained in this report were performed using Synchro Software Version 11 (Synchro) HCM 6th Edition reports. Using the traffic volumes developed for each scenario, Synchro assigns a Level of Service (LOS) for each approach to each study intersection. These LOS range from “A” to “F”, similar to a school’s grading system, with LOS A being the best possible traffic operation conditions and LOS F being the worst. A summary of these guidelines has been included in **Appendix E** at the end of this report.

The Existing Year 2023 Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the existing study intersections during the AM, PM, and SAT peak hours.

² Existing Site Access will become the proposed Site Drive C.

Tables 1A and 1B in the Tables section at the end of this report show the Existing Year 2023 Condition AM, PM, and SAT peak hour LOS for the existing study intersection. The Existing Year 2023 Condition peak hour LOS can be seen graphically on **Figure 3b**.

Synchro and HCM printouts for the Existing Year 2023 Condition can be found in **Appendix F** and **Appendix G**, respectively, at the end of this report.

4.0 OPENING YEAR 2024 WITHOUT DEVELOPMENT CONDITION

4.1 Base Condition Traffic Volume Development

A linear growth rate of 1.00% was applied to the Existing Year 2023 Condition peak hour traffic volumes (Figure 3a) to develop the Opening Year 2024 and Design Year 2029 Base Condition traffic volumes, which can be seen graphically on **Figure 4** and **Figure 5**, respectively.

4.2 Background Developments (Piazza Retail, South Fayette Commons, and Newbury: Cigar Bar & Restaurant Developments)

Trips associated with several developments that have not yet been constructed (or fully constructed) were added to the Opening Year 2024 Base Condition to develop the Opening Year 2024 Without Development Condition. These developments are discussed in the following sections. Source data for these developments (i.e. email correspondence, development descriptions, site plans, TIA/TIS figures, etc.) can be found in **Appendix H** at the end of this report.

The background developments include the construction of the Piazza Retail, South Fayette Commons, and Newbury (specifically, the Cigar Lounge, Bar & Restaurant) developments.

- The Piazza Retail development consists of a 8,250-SF Fast Food Restaurant with a Drive-Through and a 6,800-SF High-Turnover (Sit-Down) Restaurant.
- The South Fayette Commons development consists of a 10,500-SF Specialty Retail Center and a 3,000-SF High-Turnover (Sit-Down) Restaurant.
- The Newbury Cigar Lounge, Bar, & Restaurant development consists of a 9,377-SF Fine Dining Restaurant.

Trips associated with these developments that anticipated to travel through the study area during the AM, PM, and SAT peak hours can be seen graphically on **Figure 6a** through **Figure 10**.

4.3 Without Development Condition Traffic Volume Development

The background development trips (Figure 10) were added to the Opening Year 2024 Base Condition traffic volumes (Figure 4) to develop the Opening Year 2024 Without Development Condition traffic volumes, which can be seen graphically on **Figure 11a**.

4.4 Capacity Analysis

The analysis performed for the Opening Year 2024 Without Development Condition assumed the same intersection geometry and traffic control used in the Existing Year 2023 Condition analysis.

The Opening Year 2024 Without Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A and **1B** in the Tables section at the end of this report show the Opening Year 2024 Without Development Condition AM, PM, and SAT peak hour LOS for the existing study intersection. The Opening Year 2024 Without Development Condition peak hour LOS can be seen graphically on **Figure 11b**.

Synchro and HCM printouts for the Opening Year 2024 Without Development Condition can be found in **Appendix I** and **Appendix J**, respectively, at the end of this report.

5.0 DESIGN YEAR 2029 WITHOUT DEVELOPMENT CONDITION

5.1 Traffic Volume Development

Once again, background development trips (Figure 10) were added to the Design Year 2029 Base Condition traffic volumes (Figure 5) to develop the Design Year 2029 Without Development Condition traffic volumes, which can be seen graphically on **Figure 12a**.

5.3 Capacity Analysis

The analysis performed for the Design Year 2029 Without Development Condition assumed the same intersection geometry and traffic control used in the Existing Year 2023 Condition analysis.

The Design Year 2029 Without Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A and 1B in the Tables section at the end of this report show the Design Year 2029 Without Development Condition AM, PM, and SAT peak hour LOS for the existing study intersection. The Design Year 2029 Without Development Condition peak hour LOS can be seen graphically on **Figure 12b**.

Synchro and HCM printouts for the Design Year 2029 Without Development Condition can be found in **Appendix K** and **Appendix L**, respectively, at the end of this report.

6.0 DEVELOPMENT DESCRIPTION

6.1 Proposed Development

The development is proposed to consist of ~45,126-square foot of retail space.

6.2 Proposed Access

Access to the site is proposed via three (3) site drives:

- Two (2) right-in / right-out access site drives (Site Drives A and B) along the northern side of Millers Run Road (SR 0050).
- One (1) full access site drive (Site Drive C) along the western side of Newbury Drive.

6.3 Trip Generation

The Institute of Transportation Engineers' (ITE's) *Trip Generation Manual*, 11th Edition, was used to determine the trip generation rates associated with the proposed development and background development trips (specifically, Land Use Codes #821 – *Shopping Plaza 40k-150k without Supermarket*, #931 – *Fine Dining Restaurant*, #932 – *High-Turnover Sit-Down Restaurant*, and #934 – *Fast Food Restaurant with Drive-Through Window*). The rates for LUC #821 were utilized to determine the additional trips anticipated to be generated by the proposed development on a typical weekday and during the AM, PM, and SAT peak hours, which are summarized as follows:

- 3,048 additional vehicles during a typical weekday (1,524 entering and 1,524 exiting)
- 78 additional vehicles during the AM peak hour (48 entering and 30 exiting)
- 234 additional vehicles during the PM peak hour (115 entering and 119 exiting)
- 251 additional vehicles during the SAT peak hour (131 entering and 120 exiting)

The site-generated trips were then split into primary trips and pass-by trips. The pass-by trip percentages were determined using the information contained in ITE's *Trip Gen Web-Based App* and are summarized as follows:

ITE Land Use Code #821 – Shopping Plaza

- AM peak hour – 30% reduction (PM-10%)
- PM peak hour – 40% reduction (2021 Pass-By Rates)
- SAT peak hour – 31% reduction (2021 Pass-By Rates)

Table 2 in the Tables section at the end of this report summarizes the traffic anticipated to be generated by the proposed development. Copies of the trip generation calculations can be found in **Appendix M** at the end of this report.

6.4 Trip Distribution

6.4.1 Primary Trips

The trip generation data, the distribution of traffic at the existing study intersections, and the location of the proposed development were all used to assign the site-generated traffic to the study area roadway network. Additionally, site drive selection was governed generally³ by the following assumptions:

- 30% of inbound trips originating from the east were assumed to utilize Site Drive A; 60% were assumed to utilize Site Drive B; the remaining 10% were assumed to utilize Site Drive C.
- 45% of outbound trips destined to the west were assumed to utilize Site Drive A; 45% were assumed to utilize Site Drive B; the remaining 10% were assumed to utilize Site Drive C.

The projected primary trip distribution and primary trips associated with the proposed GetGo can be seen graphically on **Figure 13a** and **Figure 13b**, respectively.

6.4.2 Pass-By Trips

In order to project the pass-by trip distribution for the site-generated traffic, the turning movement count data at the existing intersections were utilized. Additionally, site drive selection was once again governed generally by the same assumptions utilized for the primary trip distribution (Section 6.4.1).

³ Engineering judgment was also utilized in estimating the distribution of both primary and pass-by trips. As such, the percentages listed in Section 6.4.1 may not be reflected exactly in the report figures, and may differ somewhat between the primary and pass-by distributions based on motorists' origins and destinations, which differ depending on whether their trip is primary or pass-by in nature.

The existing intersection distribution percentages can be seen graphically on **Figure 14**. The pass-by trip distribution percentage for each approach to the site (from the east, west, north, and south) can be seen graphically on **Figures 15a** through **15d**. Each movement on each approach was evaluated separately.

The individual (i.e. directional) pass-by trip distribution percentages were then combined to develop the overall pass-by trip distribution, which can be seen graphically on **Figure 16a**. The projected site-generated pass-by trips associated with the proposed development can be seen graphically on **Figure 16b**.

The site-generated primary trips (Figure 13b) and pass-by trips (Figure 16b) were then combined onto **Figure 17**, which depicts the total site-generated traffic associated with the proposed development.

7.0 OPENING YEAR 2024 WITH DEVELOPMENT CONDITION

7.1 Traffic Volume Development

To develop the Opening Year 2024 With Development Condition traffic volumes, the proposed site-generated trips (Figure 17) were added to the Opening Year 2024 Without Development Condition traffic volumes (Figure 11a). The resulting Opening Year 2024 With Development Condition traffic volumes can be seen graphically on **Figure 18a**.

7.2 Capacity Analysis

The analysis performed for the Opening Year 2024 With Development Condition assumed the same intersection geometry and traffic control used in the Opening Year 2024 Without Development Condition analysis.

The Opening Year 2024 With Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the study intersections during the AM, PM, and SAT peak hours.

Additionally, the capacity analyses show minimal increases in average delay (less than 10.0 seconds per vehicle) under the Opening Year 2024 With Development Condition when compared to the Opening Year 2024 Without Development Condition at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A through **1D** in the Tables section at the end of this report show the Opening Year 2024 With Development Condition AM, PM, and SAT peak hour LOS for the study

intersections. The Opening Year 2024 With Development Condition peak hour LOS can be seen graphically on **Figure 18b**.

Synchro and HCM printouts for the Opening Year 2024 With Development Condition can be found in **Appendix N** and **Appendix O**, respectively, at the end of this report.

8.0 DESIGN YEAR 2029 WITH DEVELOPMENT CONDITION

8.1 *Traffic Volume Development*

Similar to the Opening Year 2024 With Development Condition, the proposed site-generated trips (Figure 17) were added to the Design Year 2029 Without Development Condition traffic volumes (Figure 12a) to determine the Design Year 2029 With Development Condition traffic volumes, which can be seen graphically on **Figure 19a**.

8.2 *Capacity Analysis*

The analysis performed for the Design Year 2029 With Development Condition assumed the same intersection geometry and traffic control used in the Design Year 2029 Without Development Condition analysis.

The Design Year 2029 With Development Condition capacity analyses show acceptable overall intersection and movement / lane group LOS (LOS D or better) at the study intersections during the AM, PM, and SAT peak hours.

Additionally, the capacity analyses show minimal increases in average delay (less than 10.0 seconds per vehicle) under the Design Year 2029 With Development Condition when compared to the Design Year 2029 Without Development Condition at the existing study intersections during the AM, PM, and SAT peak hours.

Tables 1A through **1D** in the Tables section at the end of this report show the Design Year 2029 With Development Condition AM, PM, and SAT peak hour LOS for the study intersections. The Design Year 2029 With Development Condition peak hour LOS can be seen graphically on **Figure 19b**.

Synchro and HCM printouts for the Design Year 2029 With Development Condition can be found in **Appendix P** and **Appendix Q**, respectively, at the end of this report.

9.0 SIGHT DISTANCE EVALUATION

Sight distance requirements were evaluated at the proposed site drives in accordance with PennDOT Title 67, Chapter 441, *Access To And Occupancy Of Highways by Driveways and Local Roads*.

Sight distance requirements were based on observed 85th percentile speeds or a design speed (posted speed limit plus 5 mph) on the abutting roadway(s), whichever is greater. The observed 85th percentile speeds along Newbury Drive were obtained via a radar speed study. The radar speed data can be found in **Appendix R** at the end of this report. Appropriate friction factors from Table B of PennDOT Publication 212 were also utilized, where appropriate, to determine the required minimum sight distances.

A complete photo log of the existing sight distance at the proposed site drives can be found in **Appendix S** at the end of this report. The results of the sight distance analyses are summarized in the table below:

Sight Line	Approach Grade (%)	Required Sight Distance (feet)	Available Sight Distance (feet)
Millers Run Road (SR 0050) with Site Drive A			
Speed Limit = 40 mph			
Corner Sight Distance Looking Left	N/A ⁴	383'	>1,000'
Millers Run Road (SR 0050) with Site Drive B			
Speed Limit = 40 mph			
Corner Sight Distance Looking Left	N/A	383'	>1,000'
Newbury Drive with Site Drive C			
Speed Limit = Not Posted; 85th Percentile Speed (NB/SB) = 24 mph / 28 mph			
Corner Sight Distance Looking Left	-2.2%	176'	~270'
Corner Sight Distance Looking Right	-1.8%	201'	~270'
Stopping Sight Distance Looking Ahead	-2.8%	177'	~290'
Stopping Sight Distance from Behind	-1.8%	201'	~250'

⁴ Because available sight line is greater than 1,000 feet, approach grade is inconsequential. Assumed 0.0% for calculations/table.

As demonstrated in the table, adequate sight distance is available at all of the proposed site drives.

***** UNLESS OTHERWISE NOTED, THE AVAILABLE SIGHT DISTANCES REPORTED IN THIS STUDY ASSUME THAT ON-SITE CLEARING AND/OR GRADING WILL BE PERFORMED, AS NECESSARY, IN ORDER TO MAXIMIZE SIGHT LINES TO THE EXTENT POSSIBLE *****

10.0 QUEUING ANALYSIS

Queuing analyses were performed to compare pre-development and post-development queuing at the study intersections. The Design Year 2029 Without and With Development Conditions were modeled in Synchro and transferred to SimTraffic. Five (5) separate 60-minute simulations (utilizing a ten-minute seeding interval) were performed for each individual peak hour.

Queue reports from SimTraffic for the Design Year 2029 Without and With Development Conditions can be found in **Appendix T** and **Appendix U**, respectively, at the end of this report. Spreadsheets summarizing the 95th percentile queue averages are also included. The results for the Design Year 2029 Without and With Development Condition queues are summarized in **Tables 3A** through **3D** for the AM, PM, and SAT peak hours.

As can be seen in the Tables, queue lengths are generally not anticipated to increase significantly (no more than 1-2 car lengths) under the Design Year 2029 With Development Condition when compared to the Design Year 2029 Without Development Condition.

In addition, queue lengths are not projected to exceed their respective storage capacities under the Design Year 2029 Without or With Development Conditions during the AM, PM, or SAT peak hours.

11.0 SUMMARY AND CONCLUSIONS

In conclusion, the proposed development in South Fayette Township, Allegheny County, Pennsylvania is not anticipated to impact traffic operations in the study area. The following is a summary of the results and recommended improvements at each of the study intersections:

Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 10.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- No roadway improvements are recommended.

Newbury Drive with Plaza Access / Site Drive C

- Impacts to this intersection will be minimal with increases in average overall intersection delay of less than 10.0 seconds per vehicle during the AM, PM, and SAT peak hours.
- Construct a full access site drive (Site Drive C) along the western side of Newbury Drive, as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive A

- Construct a right-in / right-out access site drive (Site Drive A) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

Millers Run Road (SR 0050) with Site Drive B

- Construct a right-in / right-out access site drive (Site Drive B) along the northern side of Millers Run Road (SR 0050), as depicted on the site plan.
- Maintain clear sight lines by way of on-site clearing and grading, as necessary.
- No additional roadway improvements recommended.

REPORT TABLES

Table 1A - Level-of-Service Summary

Intersection		AM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn	C (24.8)	C (26.9)	C (26.2)	NM	C (27.4)	C (26.9)	NM
	Through							
	Right Turn	B (14.4)	B (17.0)	B (17.1)	NM	B (17.2)	B (17.4)	NM
	Approach	B (14.8)	B (17.3)	B (17.7)	NM	B (17.6)	B (17.9)	NM
Westbound	Left Turn	C (23.5)	C (25.2)	C (25.3)	NM	C (25.8)	C (25.9)	NM
	Through	B (11.6)	B (11.8)	B (12.7)	NM	B (11.9)	B (12.8)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	B (13.2)	B (15.1)	B (15.7)	NM	B (15.2)	B (15.9)	NM
Newbury Drive / Todd A. Miller Drive		North/South Roadway						
Northbound	Left Turn	C (33.9)	C (25.1)	C (25.2)	NM	C (25.8)	C (25.9)	NM
	Through	C (28.3)	C (26.3)	C (26.5)	NM	C (27.0)	C (27.2)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	C (29.8)	C (25.5)	C (25.6)	NM	C (26.2)	C (26.3)	NM
Southbound	Left Turn	C (21.0)	C (22.9)	C (22.8)	NM	C (23.4)	C (23.3)	NM
	Through							
	Right Turn	B (19.2)	C (23.2)	C (22.9)	NM	C (23.6)	C (23.3)	NM
	Approach	C (20.8)	C (22.9)	C (22.8)	NM	C (23.4)	C (23.3)	NM
Overall Intersection		B (15.4)	B (18.0)	B (18.3)	NM	B (18.2)	B (18.6)	NM

NM = No Mitigation

Intersection		PM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn	C (24.5)	C (25.8)	C (26.4)	NM	C (26.0)	C (27.1)	NM
	Through							
	Right Turn	B (14.0)	B (16.0)	B (16.5)	NM	B (16.4)	B (16.7)	NM
	Approach	B (14.8)	B (16.8)	B (17.7)	NM	B (17.1)	B (18.0)	NM
Westbound	Left Turn	C (24.7)	C (26.7)	C (27.3)	NM	C (26.9)	C (27.9)	NM
	Through	B (14.9)	B (15.3)	B (17.0)	NM	B (15.8)	B (17.3)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	B (15.4)	B (16.8)	B (18.2)	NM	B (17.2)	B (18.6)	NM
Newbury Drive / Todd A. Miller Drive		North/South Roadway						
Northbound	Left Turn	C (27.2)	C (25.4)	C (25.9)	NM	C (25.5)	C (26.4)	NM
	Through	C (25.6)	C (25.7)	C (26.2)	NM	C (25.9)	C (26.7)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	C (26.2)	C (25.5)	C (26.0)	NM	C (25.7)	C (26.5)	NM
Southbound	Left Turn	C (20.8)	C (22.1)	C (22.2)	NM	C (22.1)	C (22.6)	NM
	Through							
	Right Turn	B (19.1)	C (22.0)	C (21.5)	NM	C (21.9)	C (21.8)	NM
	Approach	C (20.5)	C (22.1)	C (22.1)	NM	C (22.1)	C (22.4)	NM
Overall Intersection		B (16.5)	B (18.3)	B (19.3)	NM	B (18.6)	B (19.6)	NM

NM = No Mitigation

Intersection		SAT Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn	C (24.7)	C (26.6)	C (27.9)	NM	C (26.7)	C (27.8)	NM
	Through							
	Right Turn	B (13.9)	B (17.4)	B (17.9)	NM	B (17.8)	B (18.4)	NM
	Approach	B (14.7)	B (18.1)	B (19.3)	NM	B (18.6)	B (19.7)	NM
Westbound	Left Turn	C (25.5)	C (27.1)	C (27.7)	NM	C (27.3)	C (27.8)	NM
	Through	B (14.3)	B (15.3)	B (17.0)	NM	B (15.7)	B (17.6)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	B (14.8)	B (17.3)	B (18.7)	NM	B (17.6)	B (19.1)	NM
Newbury Drive / Todd A. Miller Drive		North/South Roadway						
Northbound	Left Turn	C (31.0)	C (26.1)	C (26.7)	NM	C (26.3)	C (26.9)	NM
	Through	C (27.7)	C (26.4)	C (26.9)	NM	C (26.6)	C (27.1)	NM
	Right Turn	YIELD	YIELD	YIELD	NM	YIELD	YIELD	NM
	Approach	C (28.9)	C (26.2)	C (26.8)	NM	C (26.4)	C (27.0)	NM
Southbound	Left Turn	C (20.6)	C (22.4)	C (22.5)	NM	C (22.4)	C (22.6)	NM
	Through							
	Right Turn	B (18.4)	C (22.1)	C (21.7)	NM	C (22.0)	C (21.6)	NM
	Approach	C (20.3)	C (22.3)	C (22.3)	NM	C (22.3)	C (22.4)	NM
Overall Intersection		B (16.3)	B (19.4)	C (20.3)	NM	B (19.6)	C (20.6)	NM

NM = No Mitigation

Table 1B - Level-of-Service Summary

Intersection		AM Peak Hour						
Plaza Access / Site Drive C		Newbury Drive with Plaza Access / Site Drive C						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn			B (10.1)	NM		B (10.2)	NM
	Through							
	Right Turn							
	Approach			B (10.1)	NM		B (10.2)	NM
Westbound	Left Turn	B (11.6)	B (11.9)	B (13.4)	NM	B (12.0)	B (13.7)	NM
	Through							
	Right Turn							
	Approach	B (11.6)	B (11.9)	B (13.4)	NM	B (12.0)	B (13.7)	NM
Newbury Drive		North/South Roadway						
Northbound	Left Turn			A (7.8)	NM		A (7.9)	NM
	Through	FREE	FREE	A (0.1)	NM	FREE	A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	FREE	FREE	A (0.7)	NM	FREE	A (0.7)	NM
Southbound	Left Turn	A (7.9)	A (7.9)	A (7.9)	NM	A (7.9)	A (7.9)	NM
	Through			A (0.1)	NM		A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	A (0.7)	A (0.7)	A (0.7)	NM	A (0.7)	A (0.6)	NM
Overall Intersection		A (1.3)	A (1.3)	A (2.0)	NM	A (1.3)	A (2.0)	NM

NM = No Mitigation

Intersection		PM Peak Hour						
Plaza Access / Site Drive C		Newbury Drive with Plaza Access / Site Drive C						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn			B (13.3)	NM		B (13.6)	NM
	Through							
	Right Turn							
	Approach			B (13.3)	NM		B (13.6)	NM
Westbound	Left Turn	B (13.7)	B (14.5)	C (19.6)	NM	B (15.0)	C (20.6)	NM
	Through							
	Right Turn							
	Approach	B (13.7)	B (14.5)	C (19.6)	NM	B (15.0)	C (20.6)	NM
Newbury Drive		North/South Roadway						
Northbound	Left Turn			A (8.3)	NM		A (8.4)	NM
	Through	FREE	FREE	A (0.2)	NM	FREE	A (0.2)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	FREE	FREE	A (1.0)	NM	FREE	A (0.9)	NM
Southbound	Left Turn	A (8.2)	A (8.3)	A (8.3)	NM	A (8.3)	A (8.3)	NM
	Through			A (0.1)	NM		A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	A (0.4)	A (0.4)	A (0.4)	NM	A (0.4)	A (0.4)	NM
Overall Intersection		A (1.6)	A (1.5)	A (3.3)	NM	A (1.5)	A (3.2)	NM

NM = No Mitigation

Intersection		SAT Peak Hour						
Plaza Access / Site Drive C		Newbury Drive with Plaza Access / Site Drive C						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn			C (15.9)	NM		C (16.4)	NM
	Through							
	Right Turn							
	Approach			C (15.9)	NM		C (16.4)	NM
Westbound	Left Turn	C (20.3)	C (24.5)	F (59.1)	NM	D (26.0)	F (67.5)	NM
	Through							
	Right Turn							
	Approach	C (20.3)	C (24.5)	F (59.1)	NM	D (26.0)	F (67.5)	NM
Newbury Drive		North/South Roadway						
Northbound	Left Turn			A (8.5)	NM		A (8.5)	NM
	Through	FREE	FREE	A (0.3)	NM	FREE	A (0.3)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	FREE	FREE	A (1.0)	NM	FREE	A (1.0)	NM
Southbound	Left Turn	A (8.6)	A (8.8)	A (8.8)	NM	A (8.9)	A (8.8)	NM
	Through			A (0.1)	NM		A (0.1)	NM
	Right Turn			A (0.0)	NM		A (0.0)	NM
	Approach	A (0.6)	A (0.5)	A (0.5)	NM	A (0.5)	A (0.5)	NM
Overall Intersection		A (3.1)	A (3.3)	A (8.3)	NM	A (3.3)	A (9.0)	NM

NM = No Mitigation

Table 1C - Level-of-Service Summary

Intersection		AM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive A						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through			A (9.8)	NM		A (9.9)	NM
	Right Turn			A (9.8)	NM		A (9.9)	NM
	Approach			A (0.0)	NM		A (0.0)	NM
Overall Intersection				A (0.0)	NM		A (0.0)	NM

NM = No Mitigation

Intersection		PM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive A						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through			B (11.7)	NM		B (11.9)	NM
	Right Turn			B (11.7)	NM		B (11.9)	NM
	Approach			A (0.1)	NM		A (0.1)	NM
Overall Intersection				A (0.1)	NM		A (0.1)	NM

NM = No Mitigation

Intersection		SAT Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive A						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through			B (10.8)	NM		B (11.0)	NM
	Right Turn			B (10.8)	NM		B (11.0)	NM
	Approach			A (0.1)	NM		A (0.1)	NM
Overall Intersection				A (0.1)	NM		A (0.1)	NM

NM = No Mitigation

Table 1D - Level-of-Service Summary

Intersection		AM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive B						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through			A (9.8)	NM		A (9.9)	NM
	Right Turn			A (9.8)	NM		A (9.9)	NM
	Approach			A (9.8)	NM		A (9.9)	NM
Overall Intersection				A (0.0)	NM		A (0.0)	NM

NM = No Mitigation

Intersection		PM Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive B						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through			B (11.8)	NM		B (12.0)	NM
	Right Turn			B (11.8)	NM		B (12.0)	NM
	Approach			B (11.8)	NM		B (12.0)	NM
Overall Intersection				A (0.2)	NM		A (0.2)	NM

NM = No Mitigation

Intersection		SAT Peak Hour						
Millers Run Road (SR 0050)		Millers Run Road (SR 0050) with Site Drive B						
		East/West Roadway						
Direction	Approach / Movement	Existing Year 2023	Opening Year 2024 - Without Development	Opening Year 2024 - With Development	Opening Year 2024 - With Development & Mitigation	Design Year 2029 - Without Development	Design Year 2029 - With Development	Design Year 2029 - With Development & Mitigation
Eastbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Westbound	Left Turn							
	Through			FREE	NM		FREE	NM
	Right Turn							
	Approach			FREE	NM		FREE	NM
Site Drive A		North/South Roadway						
Northbound	Left Turn							
	Through							
	Right Turn							
	Approach							
Southbound	Left Turn							
	Through			B (11.0)	NM		B (11.1)	NM
	Right Turn			B (11.0)	NM		B (11.1)	NM
	Approach			B (11.0)	NM		B (11.1)	NM
Overall Intersection				A (0.1)	NM		A (0.1)	NM

Table 2
Trip Generation Summary

TIME PERIOD	ANTICIPATED TRIP GENERATION		
	IN	OUT	TOTAL
LUC #821 – Shopping Plaza (40-150k) without Supermarket – 45,126 SF			
ADT	1,524	1,524	3,048
AM Peak Hour	48	30	59
<i>Primary Trips</i>	34	21	55
<i>Pass-By Trips (26%)</i>	14	9	13
PM Peak Hour	115	119	234
<i>Primary Trips</i>	69	71	140
<i>Pass-By Trips (36%)</i>	46	48	94
SAT Peak Hour	131	120	251
<i>Primary Trips</i>	90	83	173
<i>Pass-By Trips (26%)</i>	41	37	78

Table 3A - Queue Summary

Intersection		AM Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development 95th Percentile Queue Length	Design Year 2029 With Development 95th Percentile Queue Length
Eastbound	Left Turn	225'	50'	81'
	Through		194'	195'
	Through		171'	172'
	Right Turn			
Westbound	Left Turn	200'	113'	126'
	Through		125'	133'
	Through		74'	95'
	Right Turn	350'	11'	25'
Newbury Drive / Todd A. Miller Drive		North/South Roadway		
Northbound	Left Turn		74'	72'
	Through		64'	69'
	Right Turn		37'	29'
Southbound	Left Turn	375'	115'	110'
	Left Turn		142'	134'
	Through			
	Right Turn		91'	81'

Intersection		PM Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development 95th Percentile Queue Length	Design Year 2029 With Development 95th Percentile Queue Length
Eastbound	Left Turn	225	73'	99'
	Through		170'	163'
	Through		135'	148'
	Right Turn			
Westbound	Left Turn	200'	121'	112'
	Through		202'	217'
	Through		159'	178'
	Right Turn	350	32'	52'
Newbury Drive / Todd A. Miller Drive		North/South Roadway		
Northbound	Left Turn		87'	86'
	Through		69'	67'
	Right Turn		48'	43'
Southbound	Left Turn	375	154'	171'
	Left Turn		181'	198'
	Through			
	Right Turn		116'	124'

Intersection		SAT Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development 95th Percentile Queue Length	Design Year 2029 With Development 95th Percentile Queue Length
Eastbound	Left Turn	225	79'	109'
	Through		169'	163'
	Through		147'	142'
	Right Turn			
Westbound	Left Turn	200'	104'	103'
	Through		178'	188'
	Through		124'	146'
	Right Turn	350'	49'	58'
Newbury Drive / Todd A. Miller Drive		North/South Roadway		
Northbound	Left Turn		91'	91'
	Through		72'	70'
	Right Turn		34'	44'
Southbound	Left Turn	375	155'	176'
	Left Turn		180'	202'
	Through			
	Right Turn		116'	118'

Table 3B - Queue Summary

Intersection		AM Peak Hour		
Plaza Access / Site Drive C		Newbury Drive with Plaza Access / Site Drive C		
Plaza Access / Site Drive C		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 <u>Without</u> Development 95th Percentile Queue Length	Design Year 2029 <u>With</u> Development 95th Percentile Queue Length
Eastbound	Left Turn			17'
	Through			
	Right Turn			
Westbound	Left Turn		39'	49'
	Through			
	Right Turn			
Newbury Drive		North/South Roadway		
Northbound	Left Turn		0'	24'
	Through			
	Through			
	Right Turn			
Southbound	Left Turn		30'	20'
	Through			
	Through			
	Right Turn			

Intersection		PM Peak Hour		
Plaza Access / Site Drive C		Newbury Drive with Plaza Access / Site Drive C		
Plaza Access / Site Drive C		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 <u>Without</u> Development 95th Percentile Queue Length	Design Year 2029 <u>With</u> Development 95th Percentile Queue Length
Eastbound	Left Turn			39'
	Through			
	Right Turn			
Westbound	Left Turn		60'	68'
	Through			
	Right Turn			
Newbury Drive		North/South Roadway		
Northbound	Left Turn		0'	44'
	Through			
	Through			
	Right Turn			
Southbound	Left Turn		37'	23'
	Through			
	Through			
	Right Turn			

Intersection		SAT Peak Hour		
Plaza Access / Site Drive C		Newbury Drive with Plaza Access / Site Drive C		
Plaza Access / Site Drive C		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 <u>Without</u> Development 95th Percentile Queue Length	Design Year 2029 <u>With</u> Development 95th Percentile Queue Length
Eastbound	Left Turn			47'
	Through			
	Right Turn			
Westbound	Left Turn		92'	143'
	Through			
	Right Turn			
Newbury Drive		North/South Roadway		
Northbound	Left Turn		0'	51'
	Through			
	Through			
	Right Turn			
Southbound	Left Turn		51'	37'
	Through			
	Through			
	Right Turn			

Table 3C - Queue Summary

Intersection		AM Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development 95th Percentile Queue Length	Design Year 2029 With Development 95th Percentile Queue Length
Eastbound	Left Turn			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			0'
	Through			0'
	Right Turn			
Site Drive A		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			0'

Intersection		PM Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development 95th Percentile Queue Length	Design Year 2029 With Development 95th Percentile Queue Length
Eastbound	Left Turn			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			0'
	Through			0'
	Right Turn			
Site Drive A		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			7'

Intersection		SAT Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 Without Development 95th Percentile Queue Length	Design Year 2029 With Development 95th Percentile Queue Length
Eastbound	Left Turn			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			0'
	Through			0'
	Right Turn			
Site Drive A		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			5'

Table 3D - Queue Summary

Intersection		AM Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 <u>Without</u> Development 95th Percentile Queue Length	Design Year 2029 <u>With</u> Development 95th Percentile Queue Length
Eastbound	Left Turn			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			0'
	Through			0'
	Right Turn			
Site Drive B		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			0'

Intersection		PM Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 <u>Without</u> Development 95th Percentile Queue Length	Design Year 2029 <u>With</u> Development 95th Percentile Queue Length
Eastbound	Left Turn			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			0'
	Through			0'
	Right Turn			
Site Drive B		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			8'

Intersection		SAT Peak Hour		
Millers Run Road (SR 0050)		East/West Roadway		
Direction	Approach / Movement	Available Storage Length	Design Year 2029 <u>Without</u> Development 95th Percentile Queue Length	Design Year 2029 <u>With</u> Development 95th Percentile Queue Length
Eastbound	Left Turn			0'
	Through			0'
	Right Turn			
Westbound	Left Turn			0'
	Through			0'
	Right Turn			
Site Drive B		North/South Roadway		
Northbound	Left Turn			
	Through			
	Right Turn			
Southbound	Left Turn			
	Through			
	Right Turn			9'

REPORT FIGURES

Proposed Development

South Fayette Township, Allegheny County, Pennsylvania

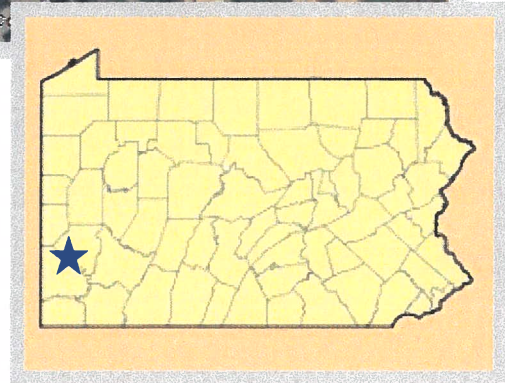
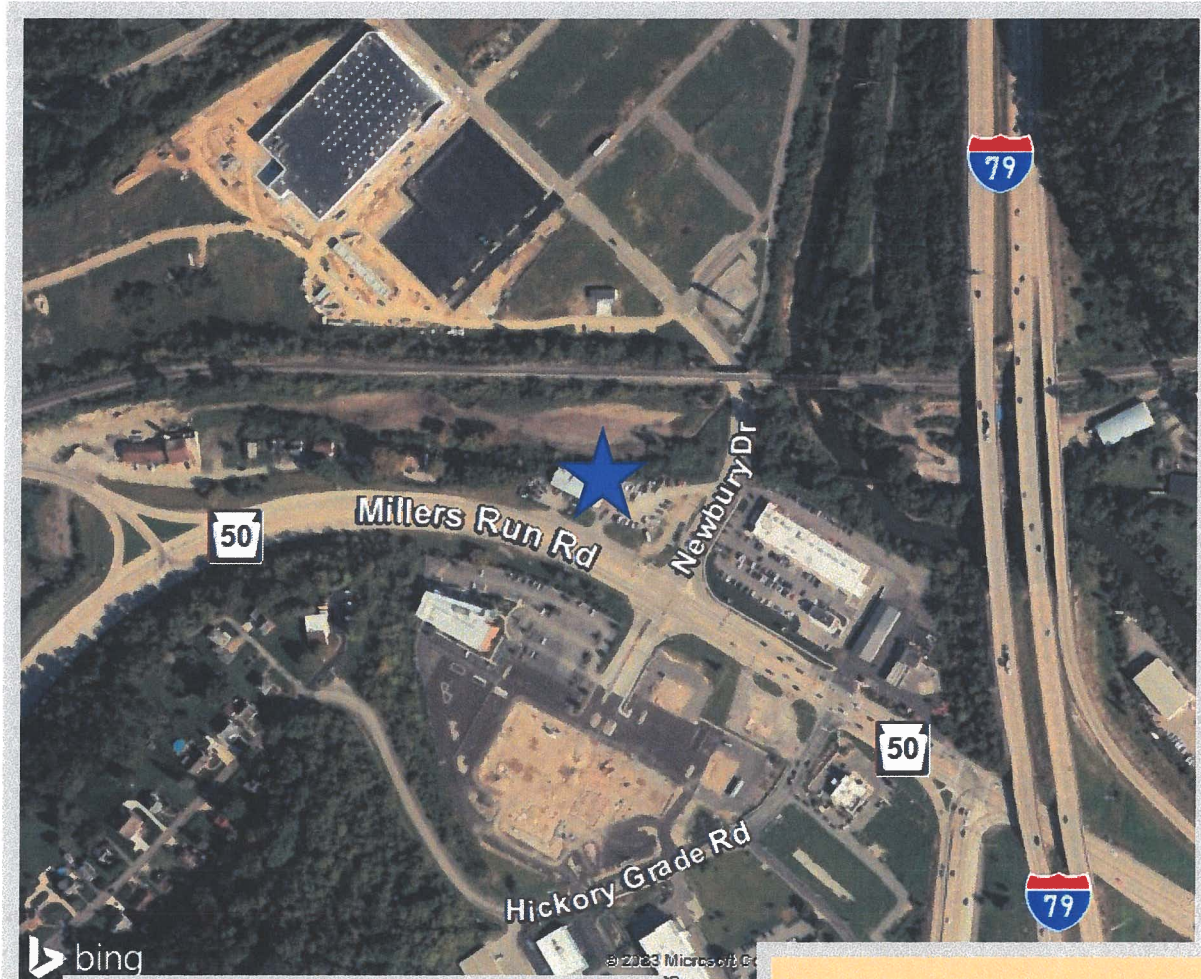
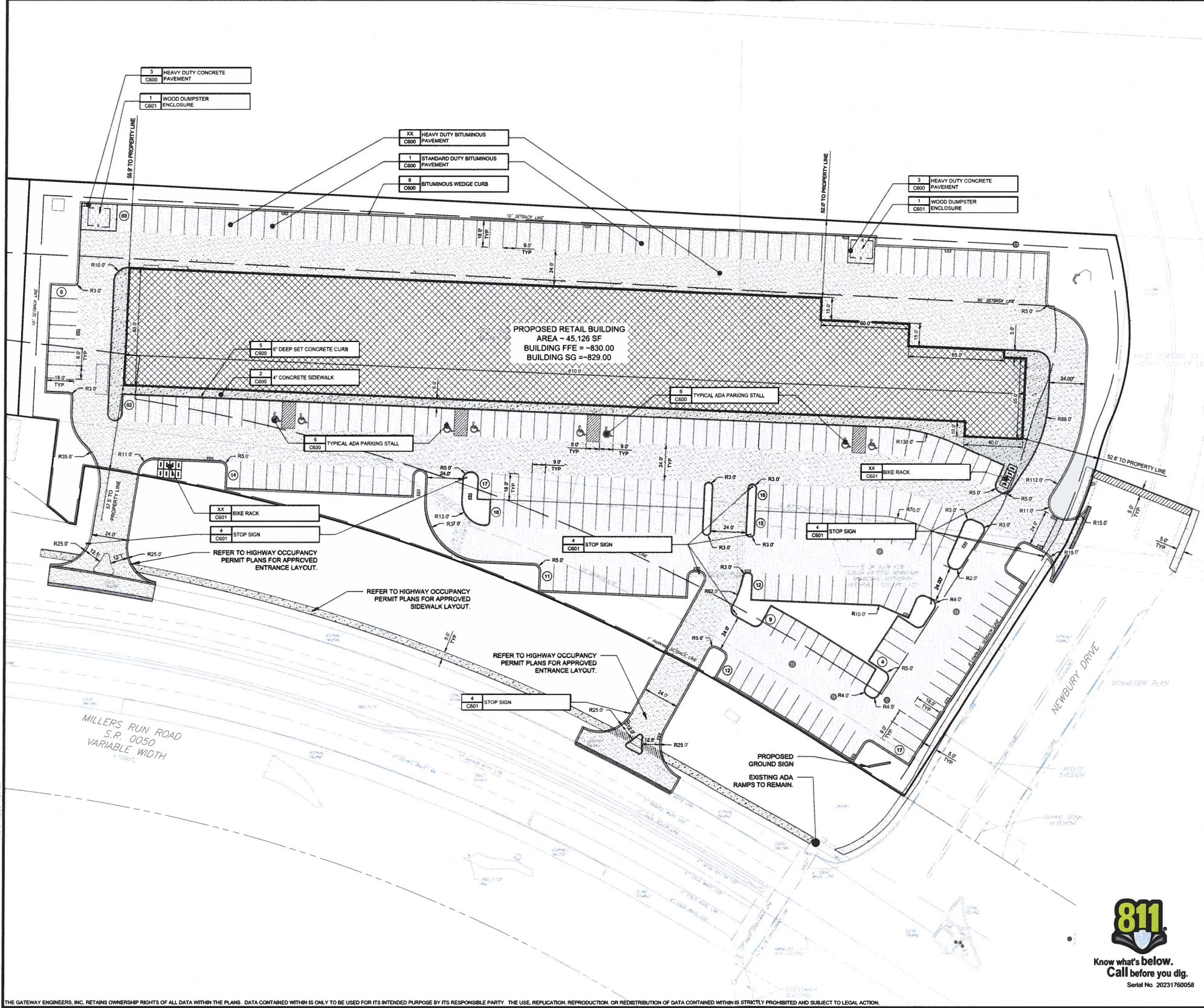


Figure 1



SITE LEGEND

●	5	PROPOSED 48" STORM MANHOLE
○	C703	
○	0	PROPOSED 48" SANITARY MANHOLE
○	C200	
○	3	PROPOSED CLEANOUT
○	C703	
○	1	PROPOSED TYPE 'M' INLET
○	C703	
○	7	PROPOSED HEADWALL/ENDWALL
○	C403	
○	15	PROPOSED HYDRANT
○	C601	
○	XX	PROPOSED LIGHT STANDARD
○	C200	
○	10	PROPOSED ADA SYMBOL
○	C600	
○	7	PROPOSED CONCRETE WHEEL STOP
○	C600	
○	14	PROPOSED BOLLARD
○	C601	
○	XX	PROPOSED WALL
○	C200	
○	0	PROPOSED DEPRESSED CURB
○	C600	
○	2	PROPOSED CONCRETE SIDEWALK
○	C600	
○	XX	PROPOSED HEAVY DUTY CONCRETE PAVEMENT
○	C200	
○	1	PROPOSED STANDARD DUTY BITUMINOUS PAVEMENT
○	C600	
○	XX	PROPOSED HEAVY DUTY BITUMINOUS PAVEMENT
○	C200	
○	XX	PROPOSED PAVEMENT PATCH
○	C600	
○	U	PROPOSED UTILITY POLE
○	○	PROPOSED PARKING COUNT
○	○	PROPOSED SIGN

- SITE PLAN NOTES:**
- CONTRACTOR SHALL REFER TO THE ARCHITECTURAL PLANS FOR THE EXACT LOCATION OF UTILITY ENTRANCES, BUILDING DIMENSIONS, ROOF LEADERS, EXIT DOORS, EXIT RAMPS AND PORCHES.
 - ALL DIMENSIONS ARE TO BUILDING FACE, FACE OF CURB OR EDGE OF SIDEWALK UNLESS NOTED OTHERWISE.
 - CONTRACTOR SHALL PROVIDE ALL LABOR AND MATERIALS FOR THE INSTALLATION OF BRIDGE AND PAVEMENT MARKINGS AS SHOWN ON THE CONSTRUCTION PLANS.
 - LABELLED DIMENSIONS OVERRIDE SCALED DIMENSIONS.
 - ALL NEW MATERIALS AND CONSTRUCTION METHODS MUST MEET PADDOT PUBLICATION 408 STANDARDS.
 - CONTRACTOR TO WORK AROUND EXISTING UTILITIES, IF ADJUSTMENTS ARE NECESSARY, WORK WILL BE CONSIDERED INCIDENTAL.
 - THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE NECESSARY SAFETY MEASURES TO SECURE THE SITE DURING CONSTRUCTION ACTIVITIES.
 - THE GATEWAY ENGINEERS, INC. IS NOT RESPONSIBLE TO INSPECT PROJECT SITES TO ENSURE COMPLIANCE WITH OCCUPATIONAL SAFETY HEALTH ADMINISTRATION (OSHA) STANDARDS.
 - TACK COAT SHALL BE REQUIRED IF PAVING LIFTS ARE NOT COMPLETED WITHIN 3 DAYS OF EACH OTHER. TACK COAT VERTICAL SURFACES ALONG SAW CUT LIMITS.
 - ALL NON-LANDSCAPED ISLANDS SHALL BE PAINTED WITH STRIPES 4" WIDE, AT 45° AND 2 FEET O.C. UNLESS OTHERWISE SPECIFIED. USE ALTO-RESIN TYPE, READY-USE COMPLYING WITH ANSI/ISO 1471 TYPE 1 COLOR WHITE.
 - WITHIN THE LIMITS OF THE PROPOSED DRIVEWAYS THE CONTRACTOR SHALL SAW CUT AND REMOVE THE EXISTING PAVEMENT AND THE EXISTING CURB AND SHOULDER. THE CURB SECTION OF THE PROPOSED DRIVEWAY SHALL BE DEPRESSED ACROSS THE LIMITS OF EACH DRIVEWAY. THE PROPOSED CURBS SHALL MATCH INTO THE EXISTING PAVEMENT AT ALL DRIVEWAY LOCATIONS. THE CONTRACTOR SHALL CLEAN CONTACT SURFACES OF ALL EXISTING PAVEMENTS BEFORE PLACEMENT OF PROPOSED ADJACENT PAVEMENT. ALL JOINTS SHALL BE SEALED WITH A 4-INCH STOP OF PS-44-52.
 - THE CONTRACTOR SHALL PROVIDE AND MAINTAIN TRAFFIC CONTROL MEASURES IN ACCORDANCE WITH PA DOT PUBLICATION 213 WORK ZONE TRAFFIC CONTROL (LATEST REVISION), AND AS REQUIRED BY LOCAL AGENCIES WHEN WORKING IN AND/OR ALONG STREETS, ROADS, HIGHWAYS, ETC. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN APPROVAL AND COORDINATE WITH LOCAL AND/OR STATE AGENCIES REGARDING THE NEED, EXTENT, AND LIMITATIONS ASSOCIATED WITH INSTALLING AND MAINTAINING TRAFFIC CONTROL MEASURES.

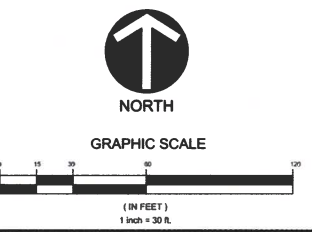


REVISION RECORD

No.	Date	Description
01	2023-06-05	GRADING PERMIT RESUBMISSION
02	2023-06-12	VARIANCE SUBMISSION
03	2023-06-14	LAND DEVELOPMENT SUBMISSION
04		
05		
06		
07		

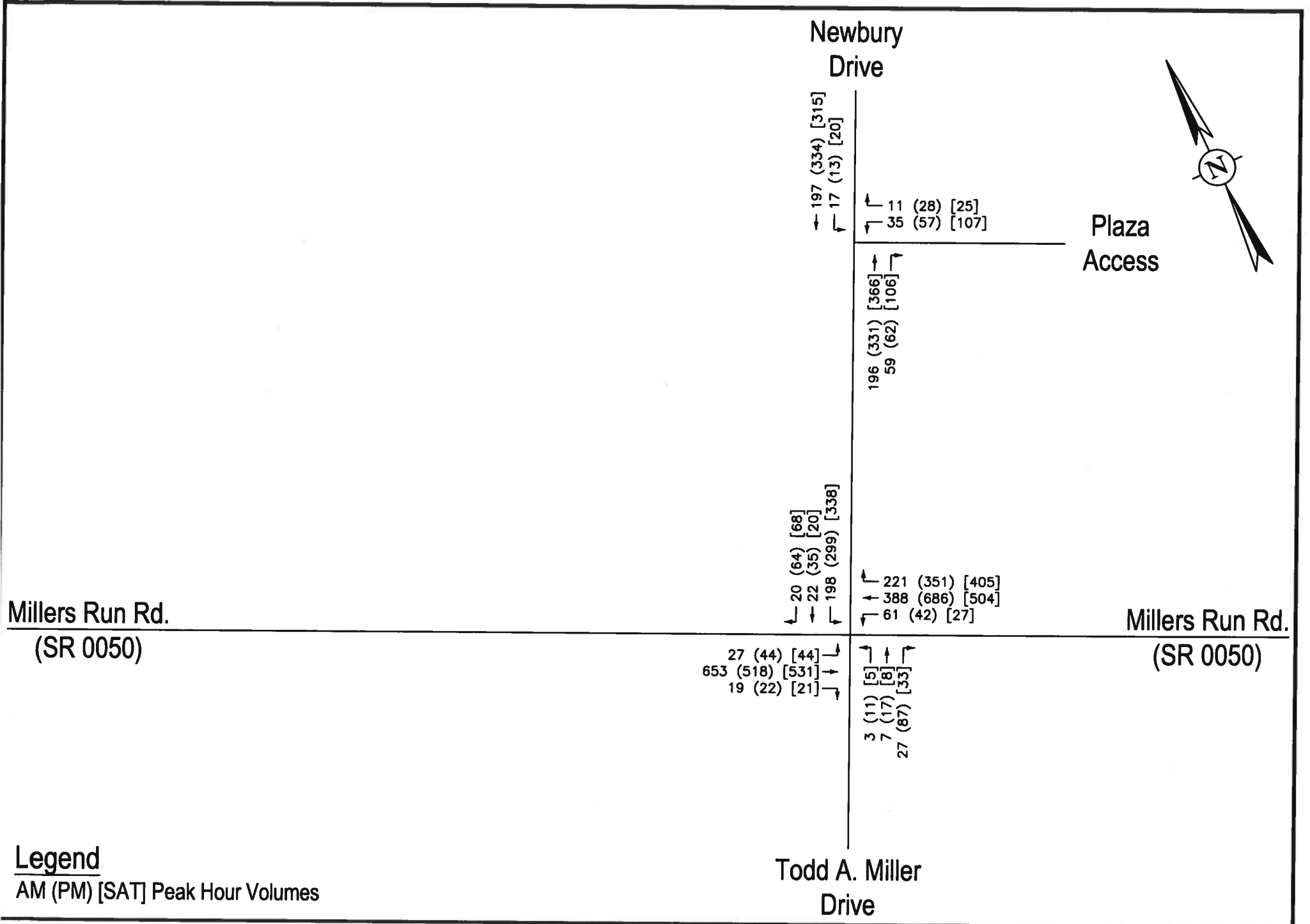
LAFAYETTE 180
 NEWBURY DRIVE
 CUDDY, PA 15031
 PREPARED FOR:
COZZA ENTERPRISES LLC
 1215 BRIGHTON ROAD
 PITTSBURGH, PA 15233

SITE PLAN
 Project Number: C-21199-0025
 Drawing Scale: 1"=30'
 Date Issued: AUGUST 2023
 Index Number: -
 Drawn By: LSR
 Checked By: JMG
 Project Manager: JMG
C100

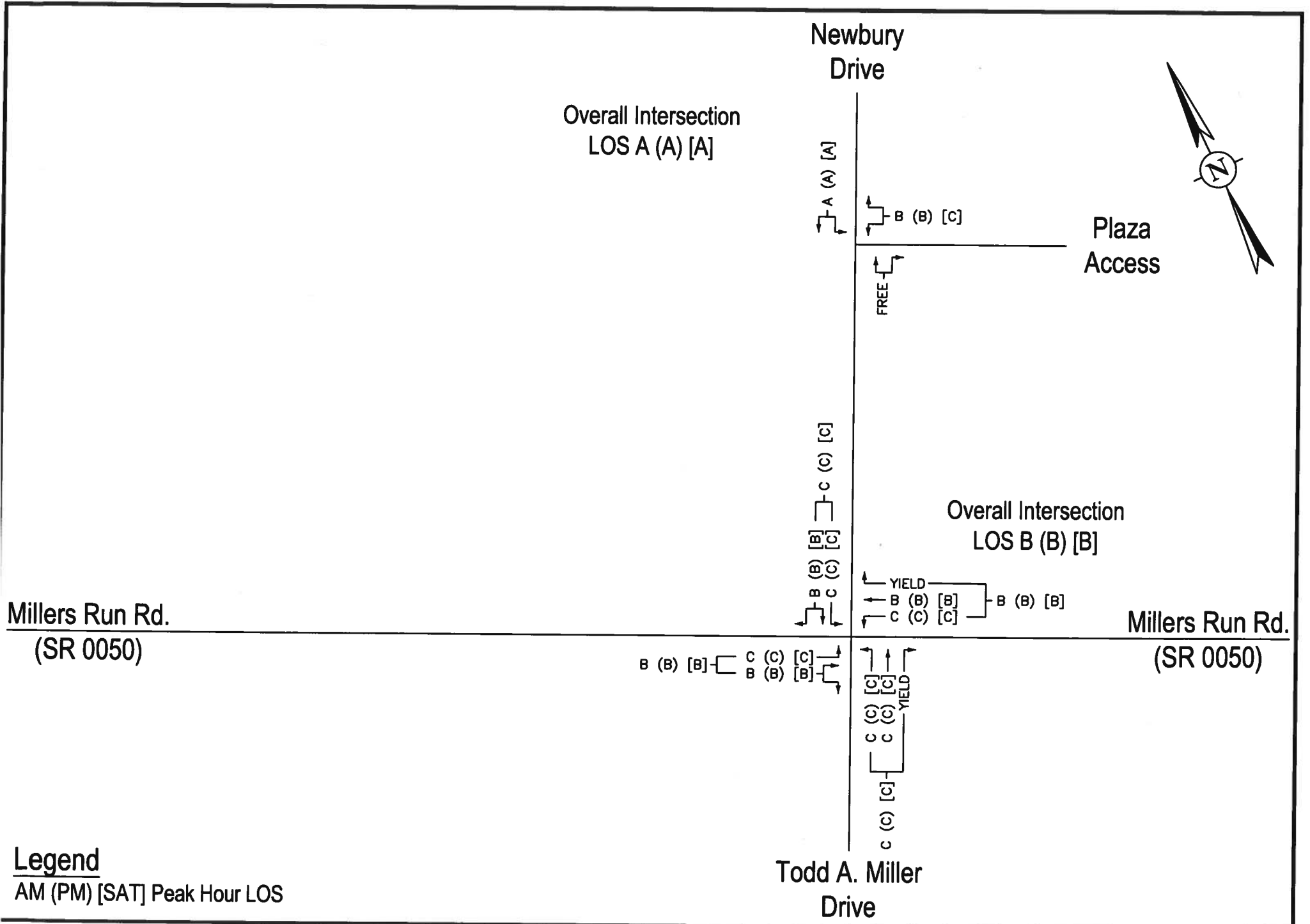


NOTE:
 NOT TO SCALE

**PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Preliminary Site Plan**

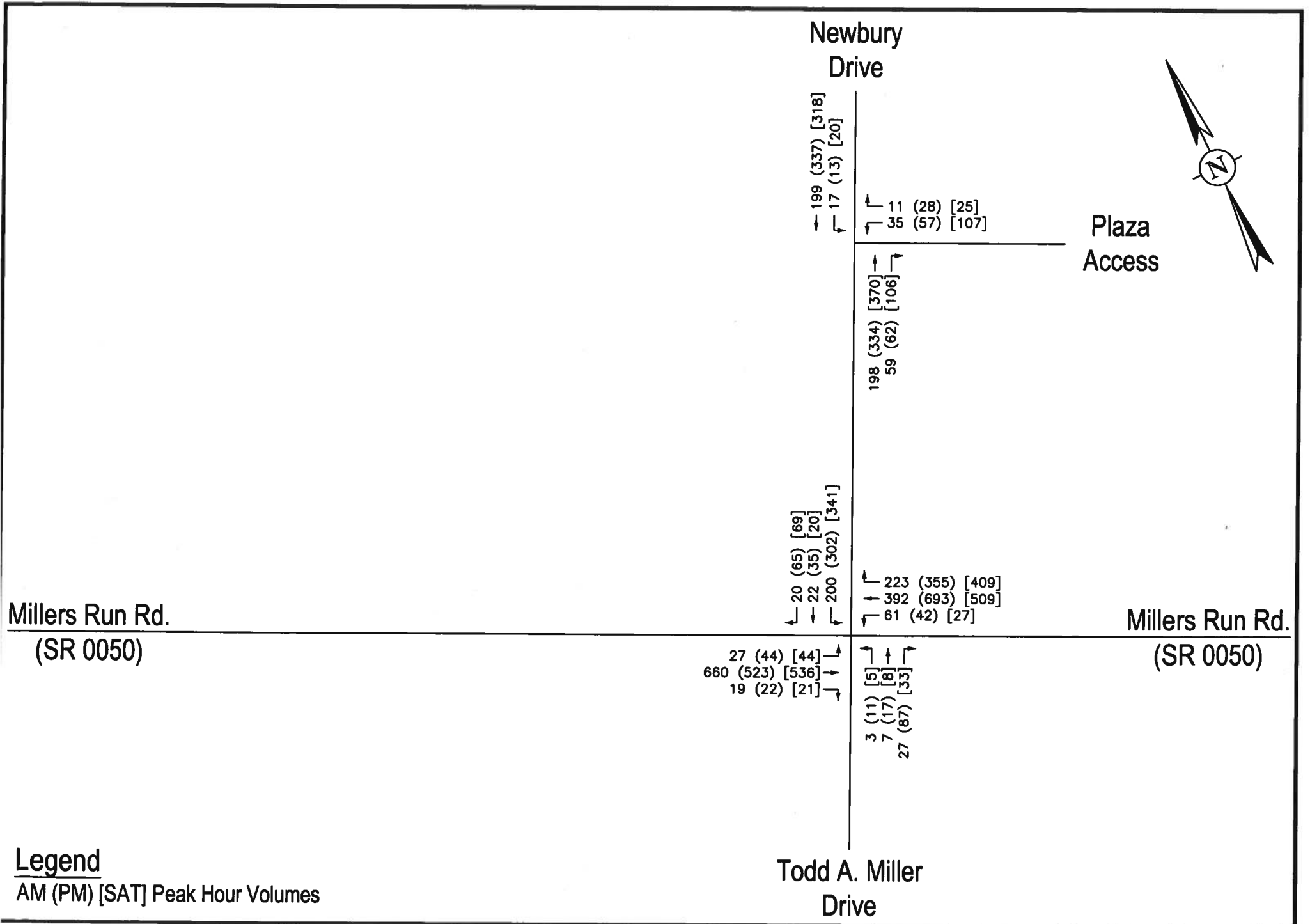


PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Existing Year 2023 Condition Peak Hour Traffic Volumes

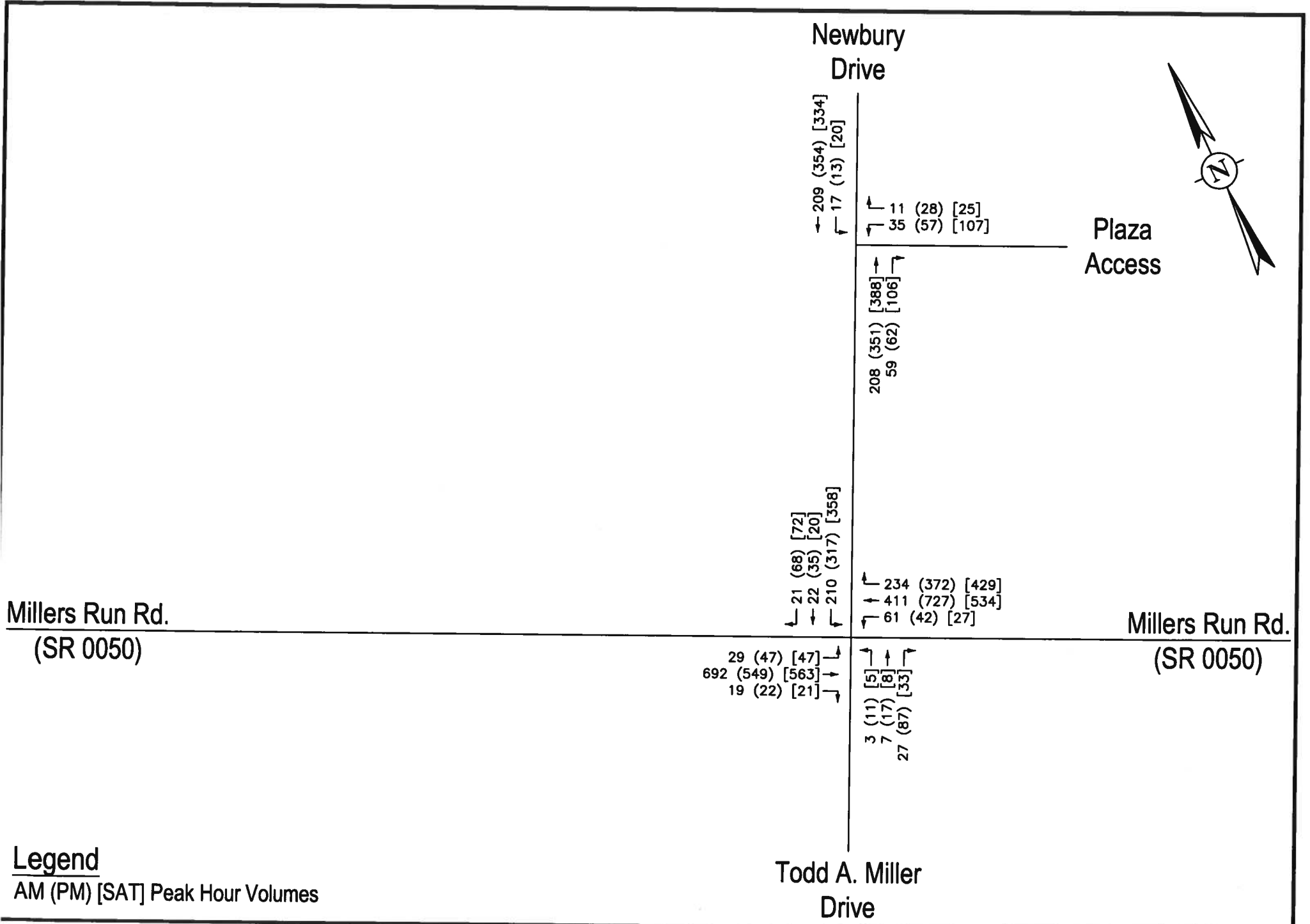


Legend
 AM (PM) [SAT] Peak Hour LOS

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Existing Year 2023 Condition Peak Hour LOS

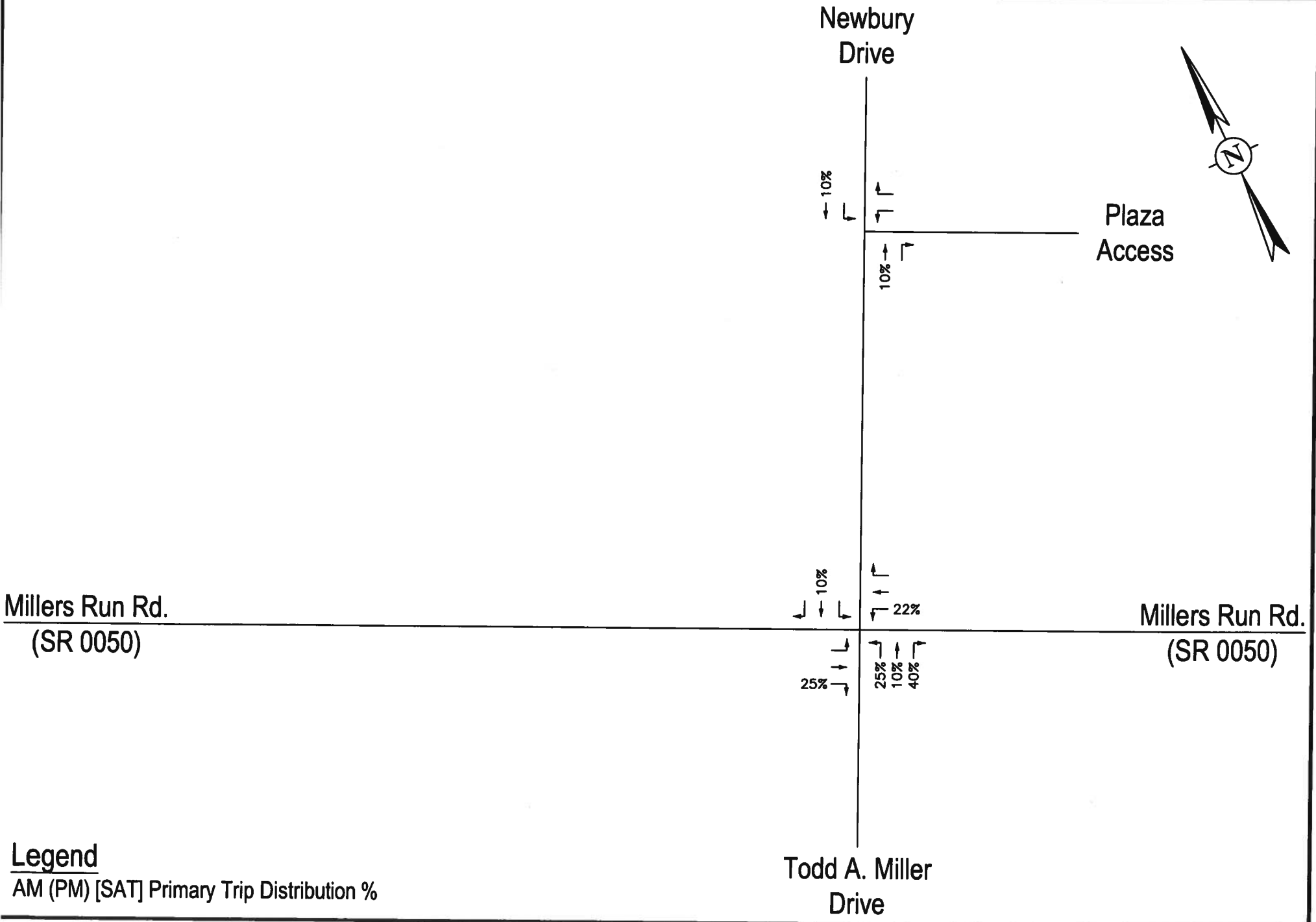
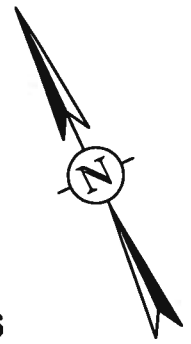


PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 Base Condition Peak Hour Traffic Volumes



Legend
 AM (PM) [SAT] Peak Hour Volumes

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Design Year 2029 Base Condition Peak Hour Traffic Volumes

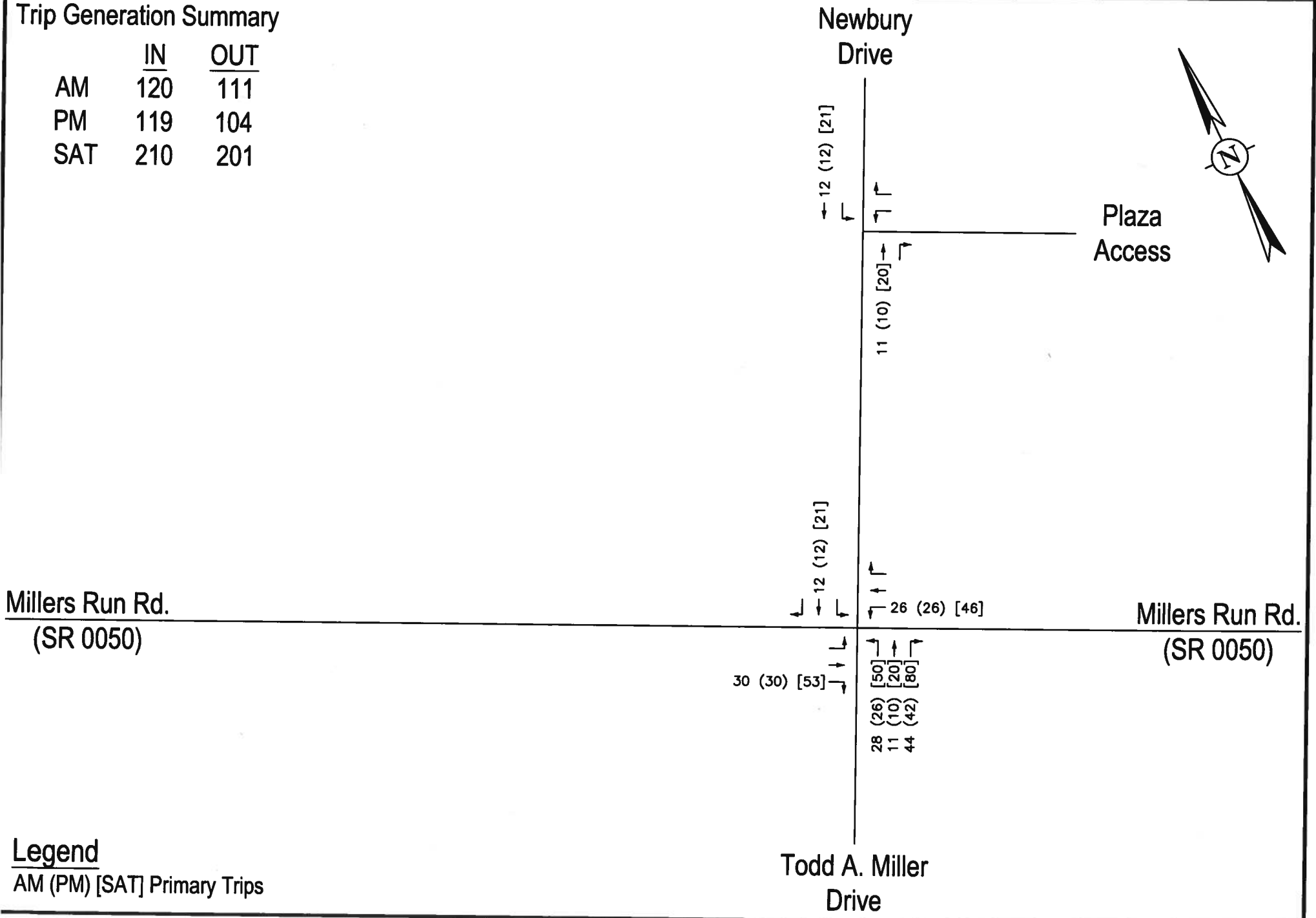
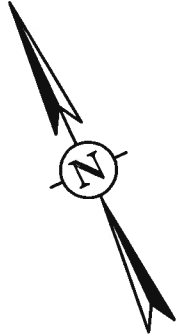


Legend
AM (PM) [SAT] Primary Trip Distribution %

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trip Distribution Percentages (The Piazza Retail Development)

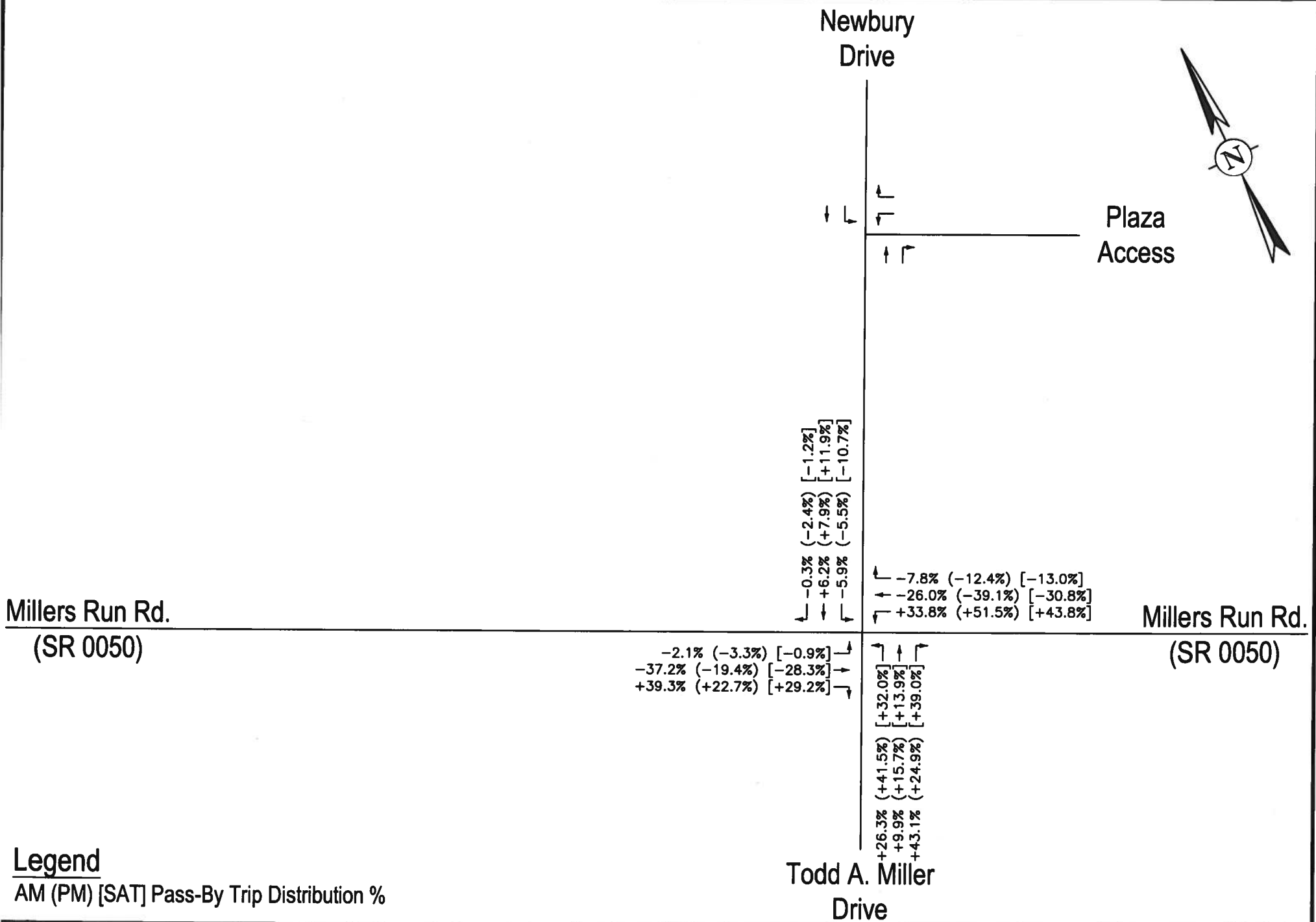
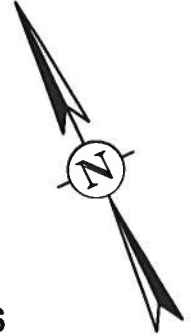
Trip Generation Summary

	<u>IN</u>	<u>OUT</u>
AM	120	111
PM	119	104
SAT	210	201



Legend
 AM (PM) [SAT] Primary Trips

**PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Primary Trips (The Piazza Retail Development)**



Legend

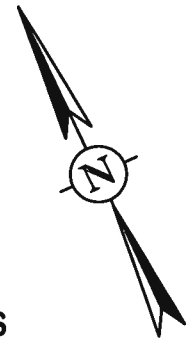
AM (PM) [SAT] Pass-By Trip Distribution %

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trip Distribution Percentages (The Piazza Retail Development)

Trip Generation Summary

	<u>IN</u>	<u>OUT</u>
AM	104	98
PM	60	51
SAT	62	59

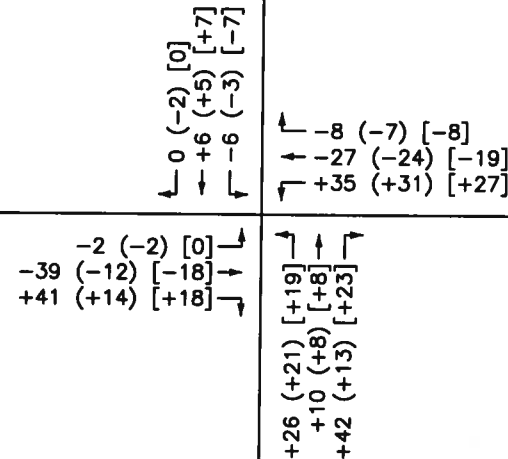
Newbury Drive



Plaza Access

Millers Run Rd.
(SR 0050)

Millers Run Rd.
(SR 0050)

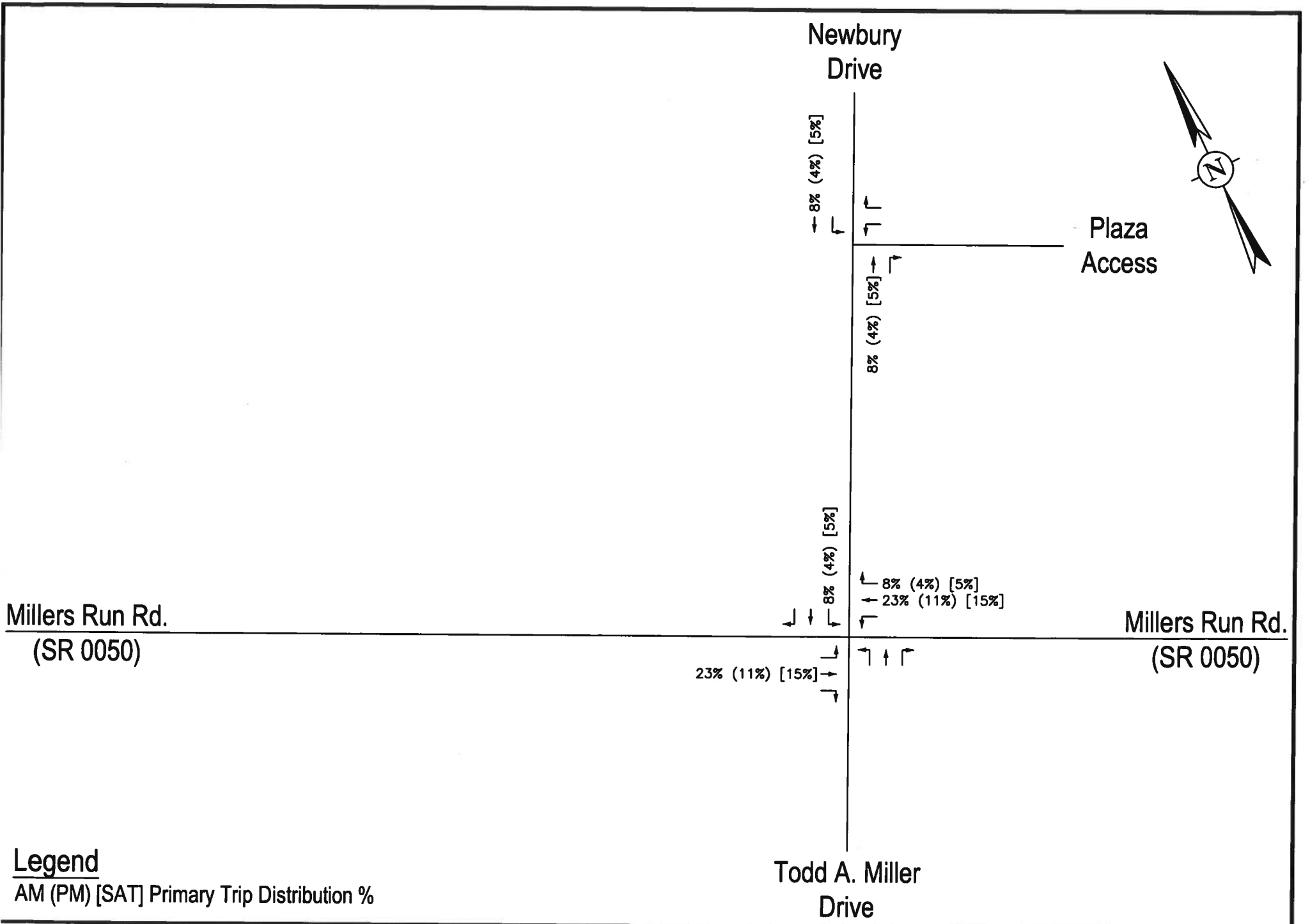


Legend

AM (PM) [SAT] Pass-By Trips

Todd A. Miller Drive

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trips (The Piazza Retail Development)



Legend

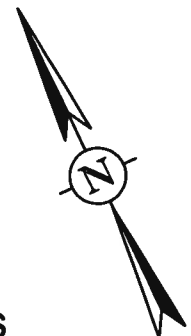
AM (PM) [SAT] Primary Trip Distribution %

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Primary Trip Distribution Percentages (South Fayette Commons Development)

Trip Generation Summary

	<u>IN</u>	<u>OUT</u>
AM	10	7
PM	19	20
SAT	18	16

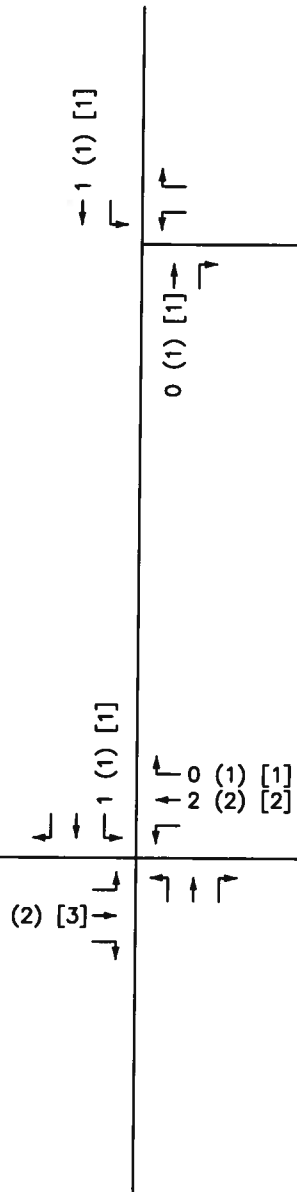
Newbury Drive



Plaza Access

Millers Run Rd.
(SR 0050)

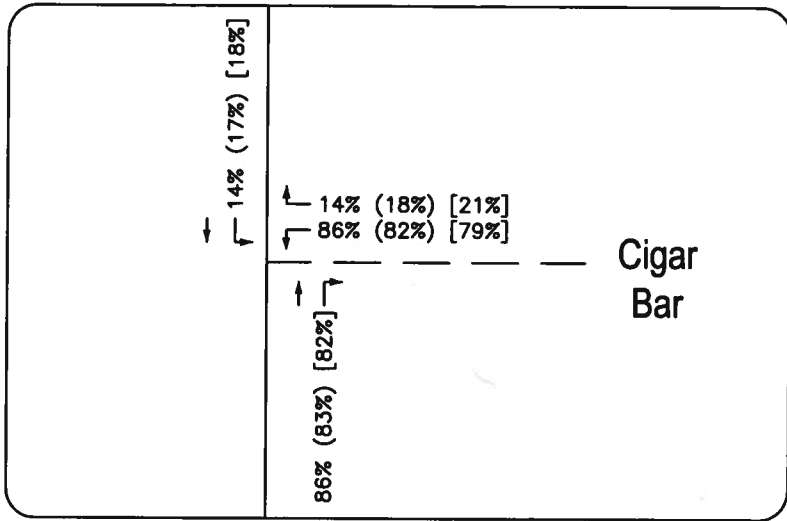
Millers Run Rd.
(SR 0050)



Legend

AM (PM) [SAT] Primary Trips

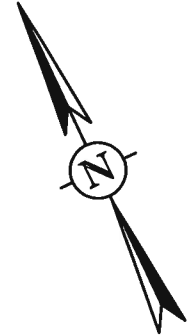
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Primary Trips (South Fayette Commons Development)



14% (17%) [18%]
214 (347) [335]

Newbury Drive

207 (359) [391]
14% (18%) [21%]



27% (37%) [31%]
411 (761) [577]

Millers Run Rd.
(SR 0050)

699 (584) [596]
44% (29%) [32%]

27% (37%) [31%]
59% (45%) [48%]

44% (29%) [32%]

Todd A. Miller Drive

86% (82%) [79%]

86% (83%) [82%]
42% (54%) [50%]

Plaza Access

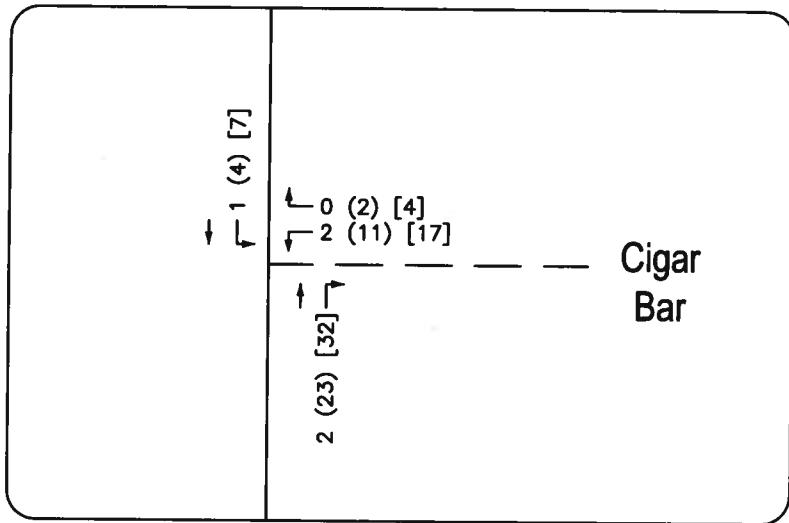
42% (54%) [50%]
670 (1079) [936]

Millers Run Rd.
(SR 0050)

878 (904) [902]
59% (45%) [48%]

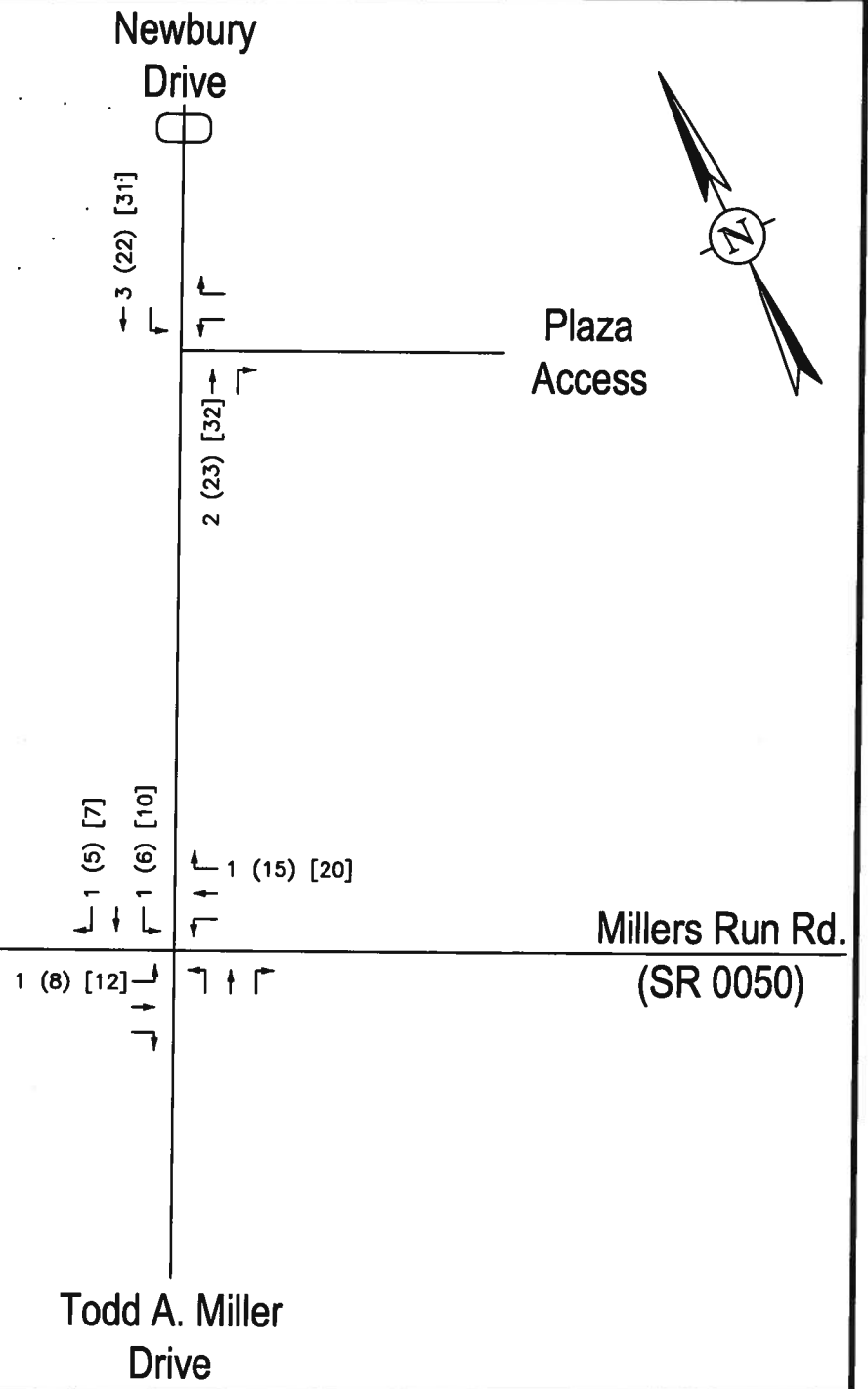
Legend
AM (PM) [SAT] Primary Trip Distribution %

PROPOSED RETAIL DEVELOPMENT - South Fayette Township
Primary Trip Distribution Percentages (Cigar Lounge, Bar & Restaurant Development)



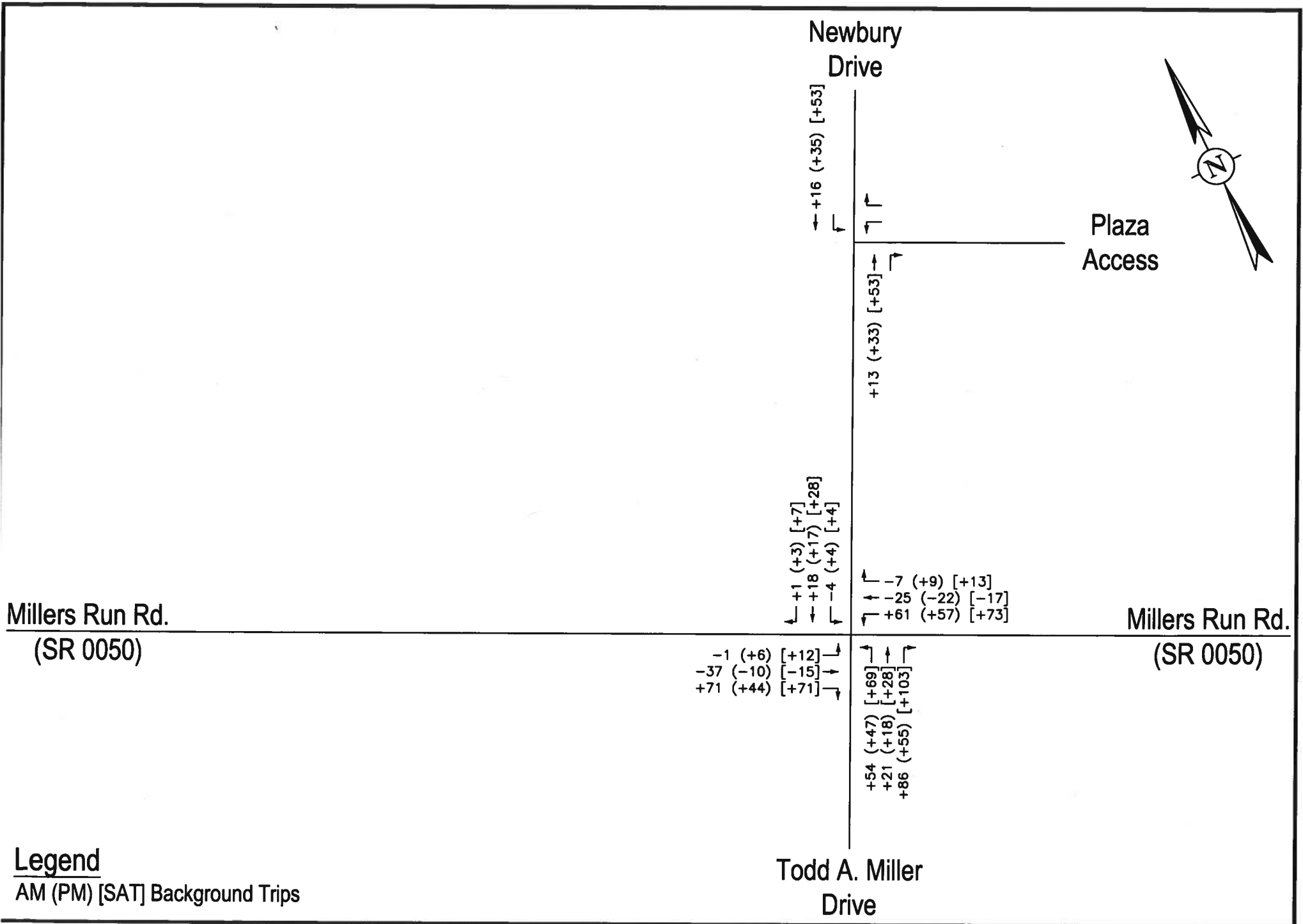
Trip Generation Summary

	<u>IN</u>	<u>OUT</u>
AM	3	2
PM	27	13
SAT	39	21



Legend
 AM (PM) [SAT] Primary Trips

**PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Primary Trips (Cigar Lounge, Bar & Restaurant Development)**



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Total Background Trips



Newbury Drive

← 215 (372) [371]
↓ 17 (13) [20]

↑ 11 (28) [25]
↘ 35 (57) [107]

Plaza Access

211 (367) [423]
59 (62) [106]

↓ 21 (68) [76]
↓ 40 (52) [48]
↓ 196 (306) [345]

↑ 216 (364) [422]
↑ 367 (671) [492]
↘ 122 (99) [100]

Millers Run Rd.
(SR 0050)

Millers Run Rd.
(SR 0050)

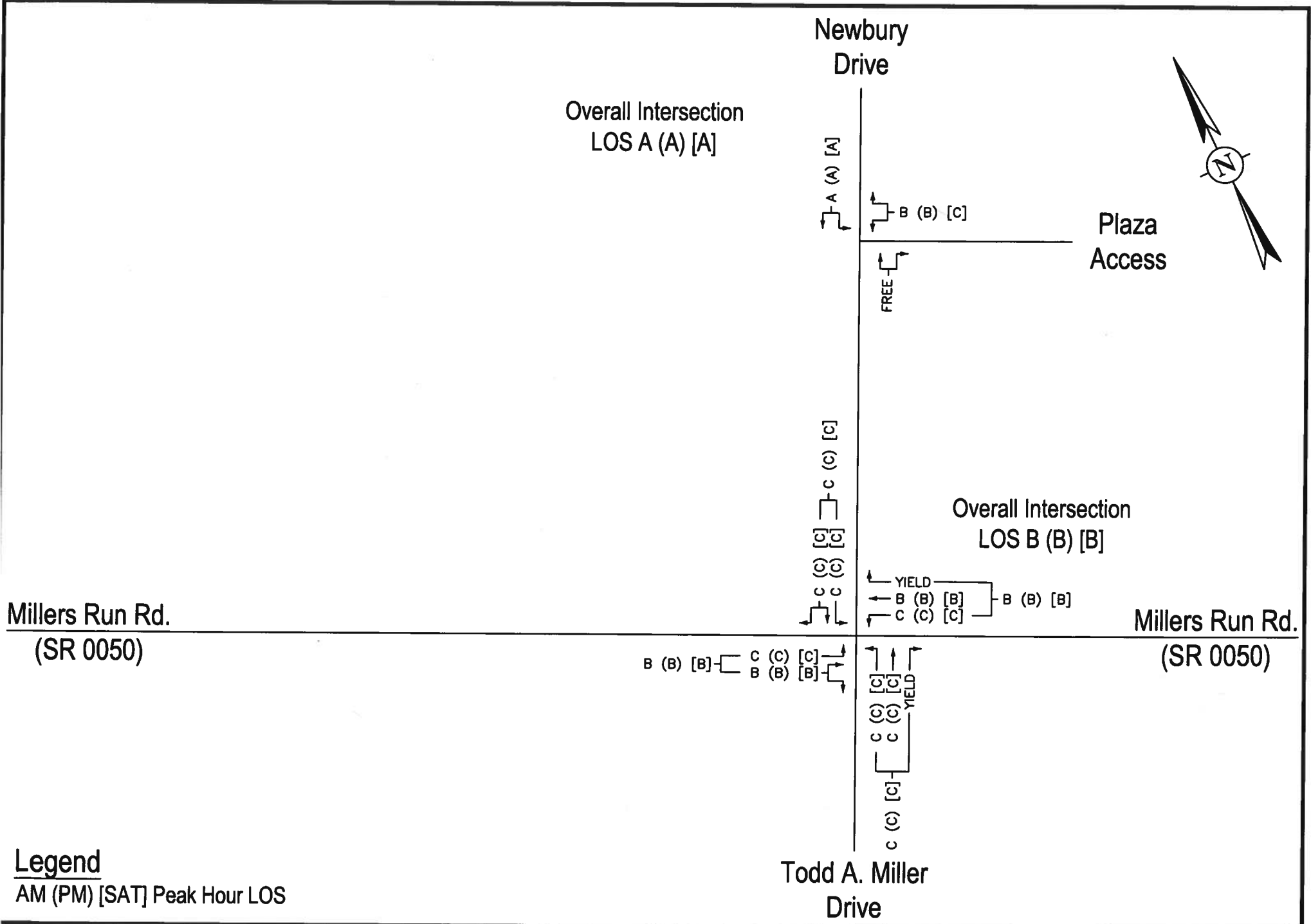
26 (50) [56]
623 (513) [521]
90 (66) [92]

↑ 57 (58) [74]
↑ 28 (35) [36]
↑ 113 (142) [136]

Todd A. Miller Drive

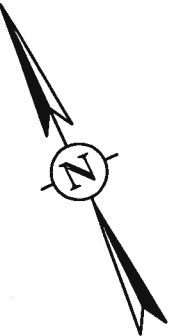
Legend
AM (PM) [SAT] Peak Hour Volumes

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Opening Year 2024 Without Development Condition Peak Hour Traffic Volumes



Legend
 AM (PM) [SAT] Peak Hour LOS

PROPOSED RETAIL DEVELOPMENT - South Fayette Township
 Opening Year 2024 Without Development Condition Peak Hour LOS



Newbury Drive

← 225 (389) [387]
 ↓ 17 (13) [20]
 ↑ 11 (28) [25]
 ↓ 35 (57) [107]

Plaza Access

↑ 221 (384) [441]
 ↓ 59 (62) [106]

↓ 22 (71) [79]
 ↓ 40 (52) [48]
 ↓ 206 (321) [362]

↑ 227 (381) [442]
 ↓ 386 (705) [517]
 ↓ 122 (99) [100]

Millers Run Rd.
(SR 0050)

Millers Run Rd.
(SR 0050)

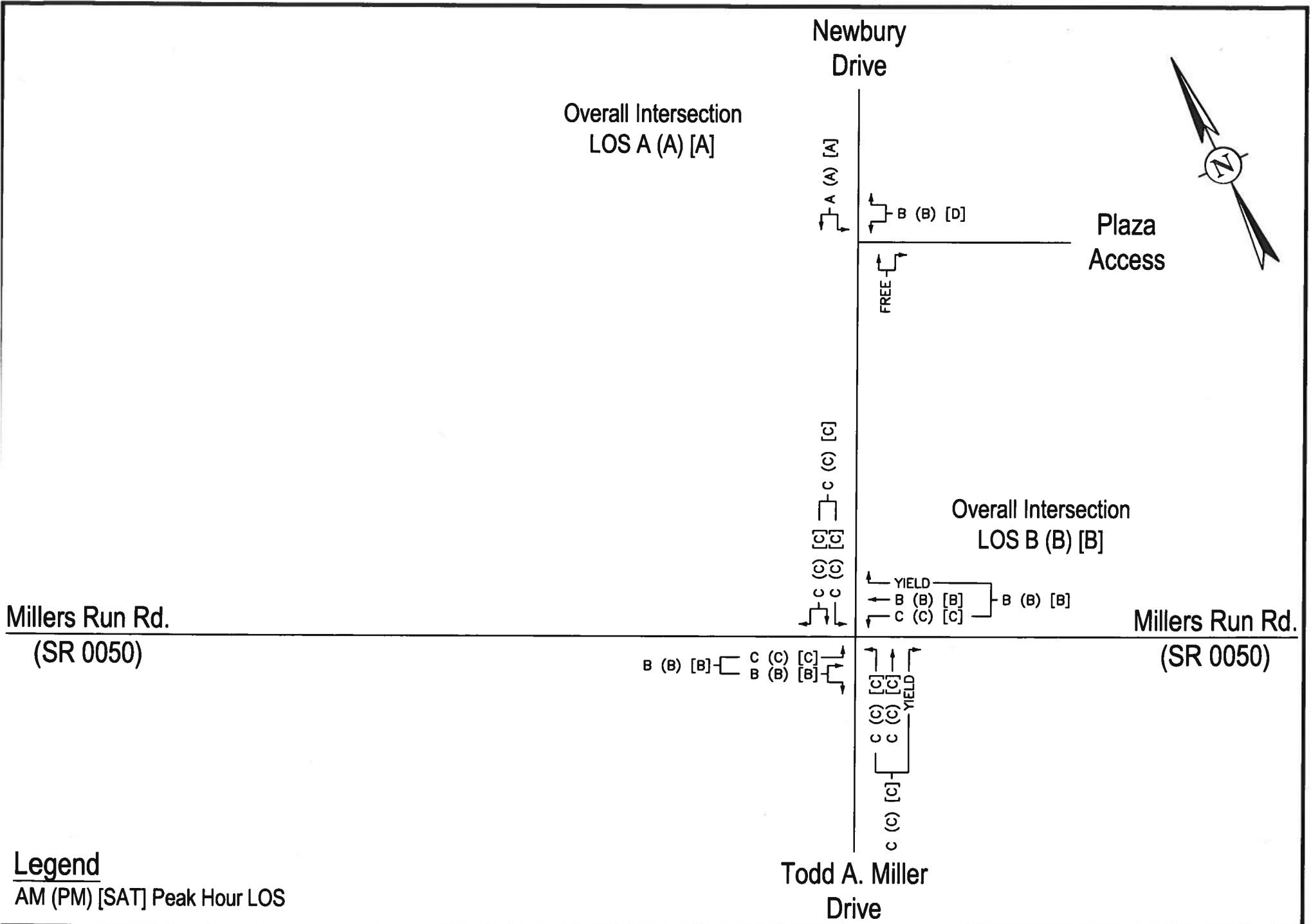
↓ 28 (53) [59]
 ↓ 655 (539) [548]
 ↓ 90 (66) [92]

↑ 57 (98) [74]
 ↓ 28 (35) [36]
 ↓ 113 (142) [136]

Todd A. Miller Drive

Legend
AM (PM) [SAT] Peak Hour Volumes

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 Without Development Condition Peak Hour Traffic Volumes

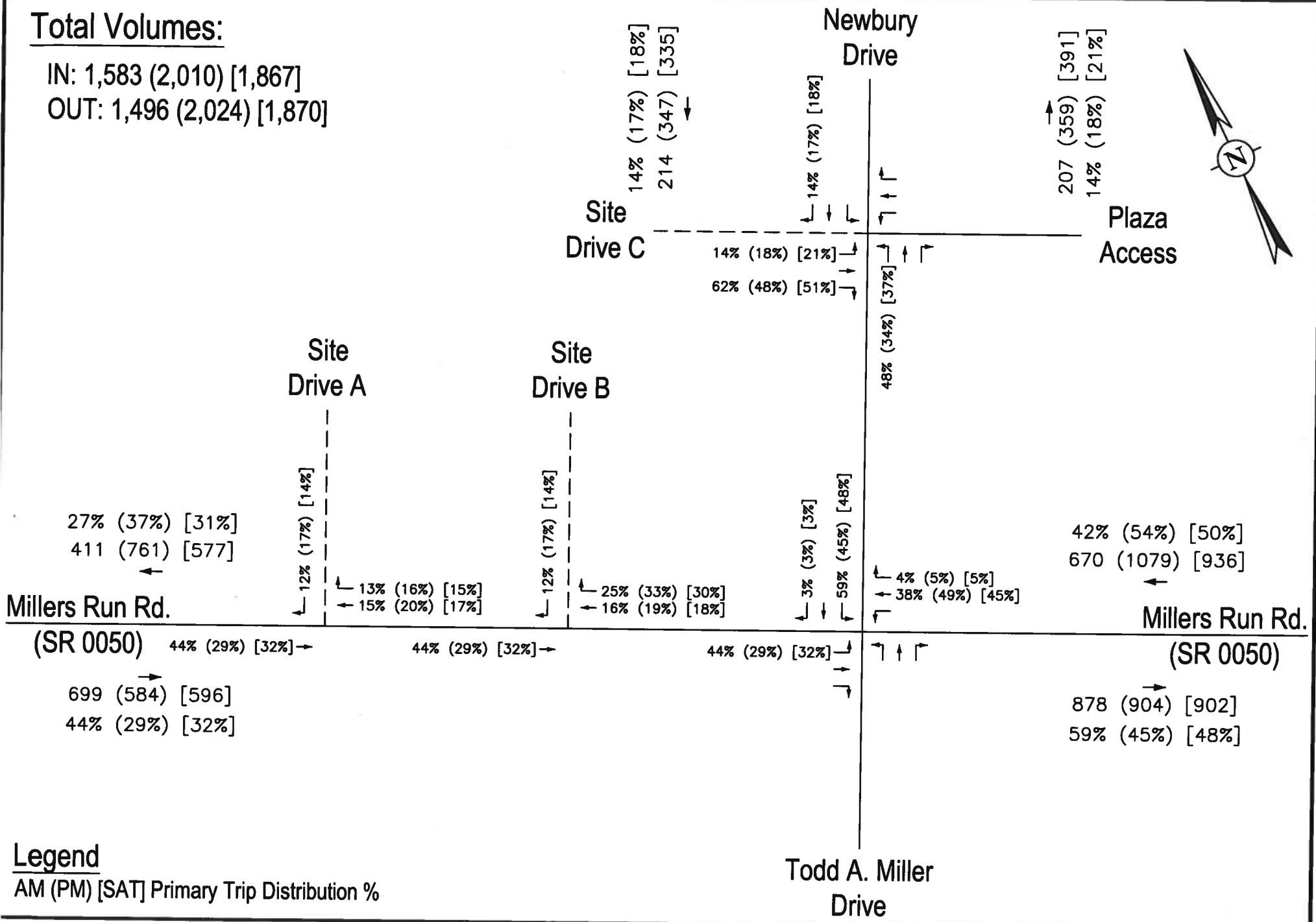
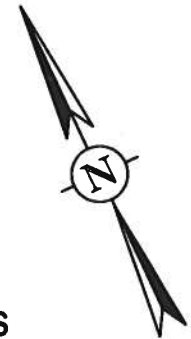


Legend
 AM (PM) [SAT] Peak Hour LOS

PROPOSED RETAIL DEVELOPMENT - South Fayette Township
 Design Year 2029 Without Development Condition Peak Hour LOS

Total Volumes:

IN: 1,583 (2,010) [1,867]
 OUT: 1,496 (2,024) [1,870]



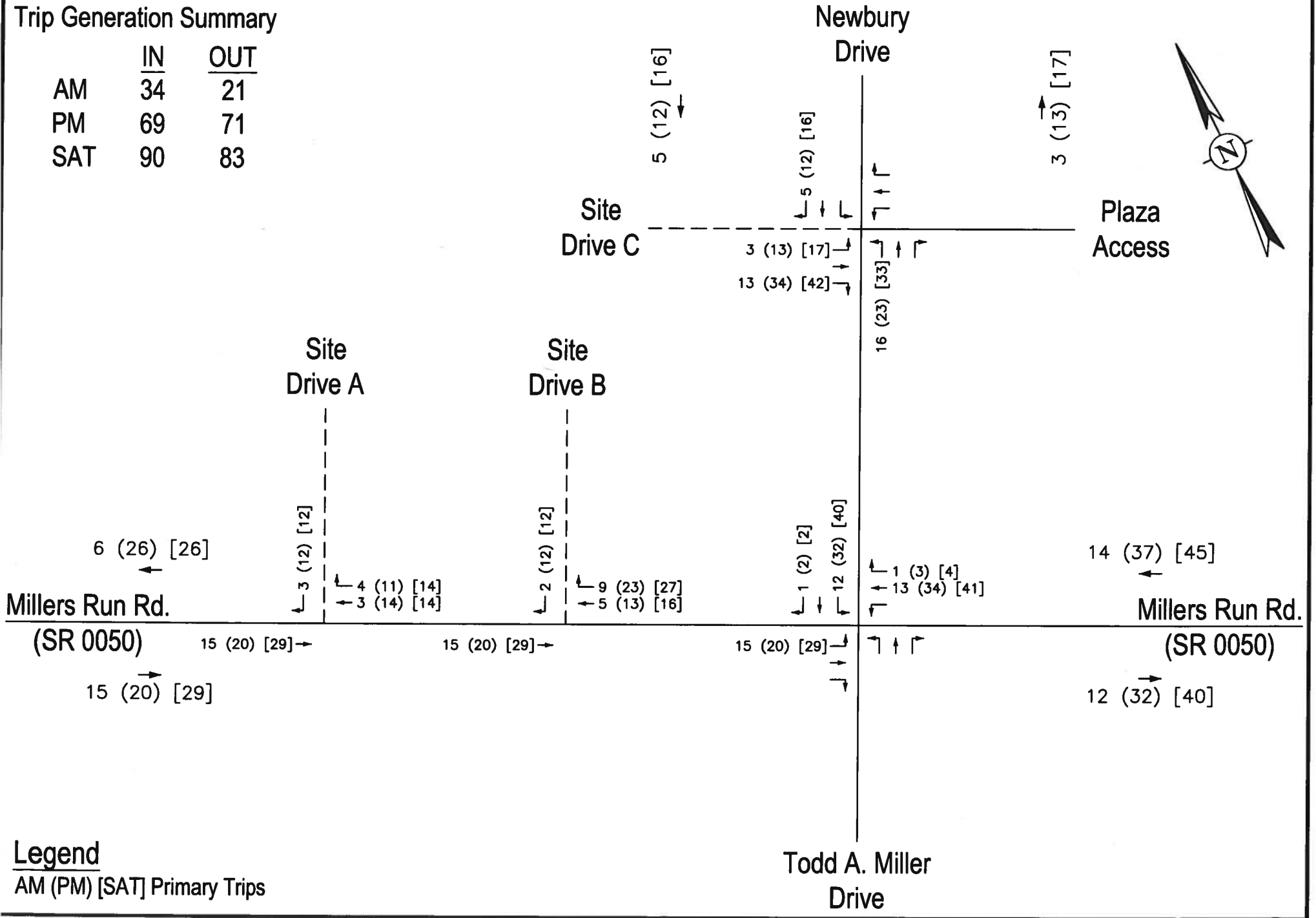
Legend

AM (PM) [SAT] Primary Trip Distribution %

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Primary Trip Distribution Percentages

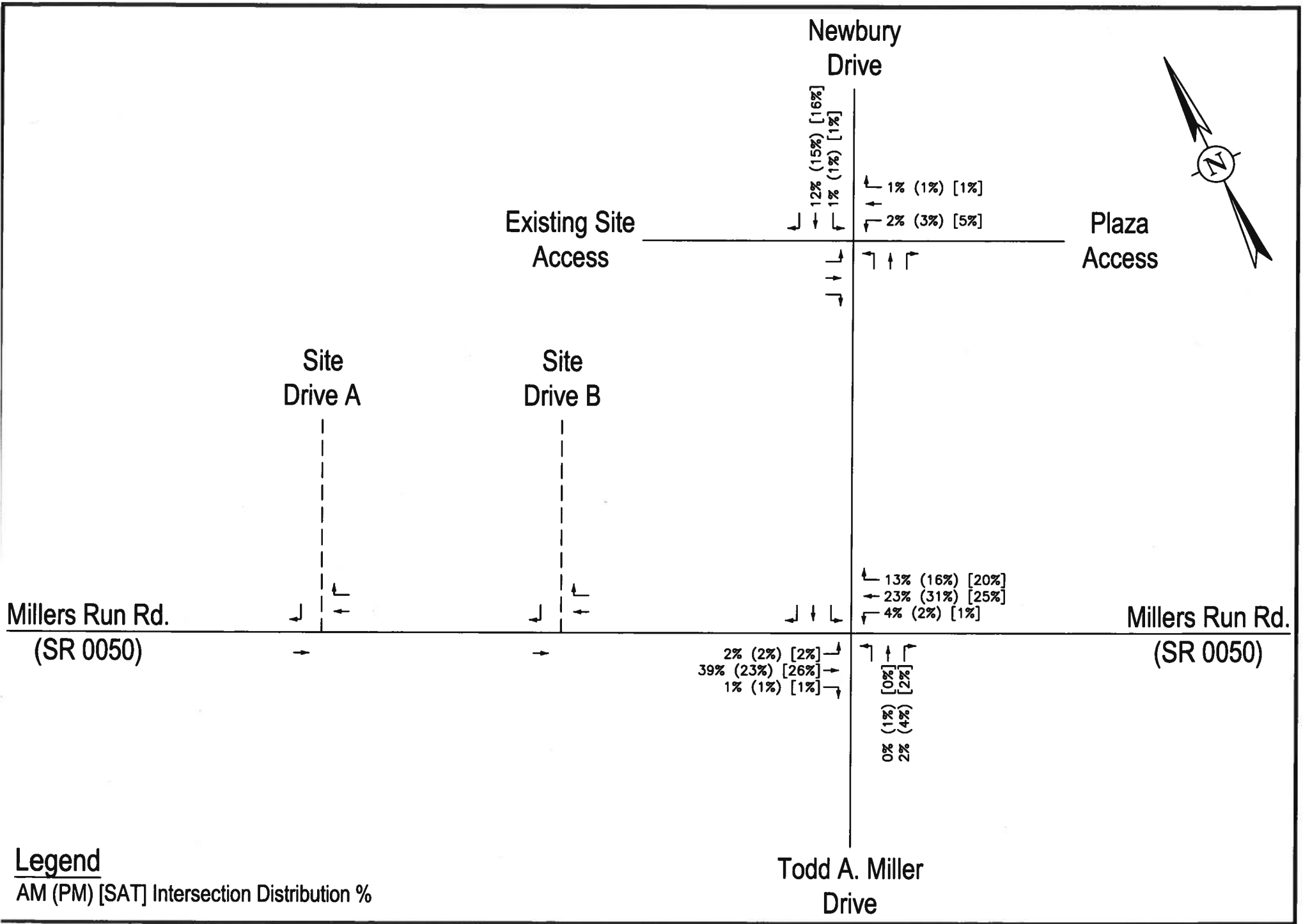
Trip Generation Summary

	<u>IN</u>	<u>OUT</u>
AM	34	21
PM	69	71
SAT	90	83

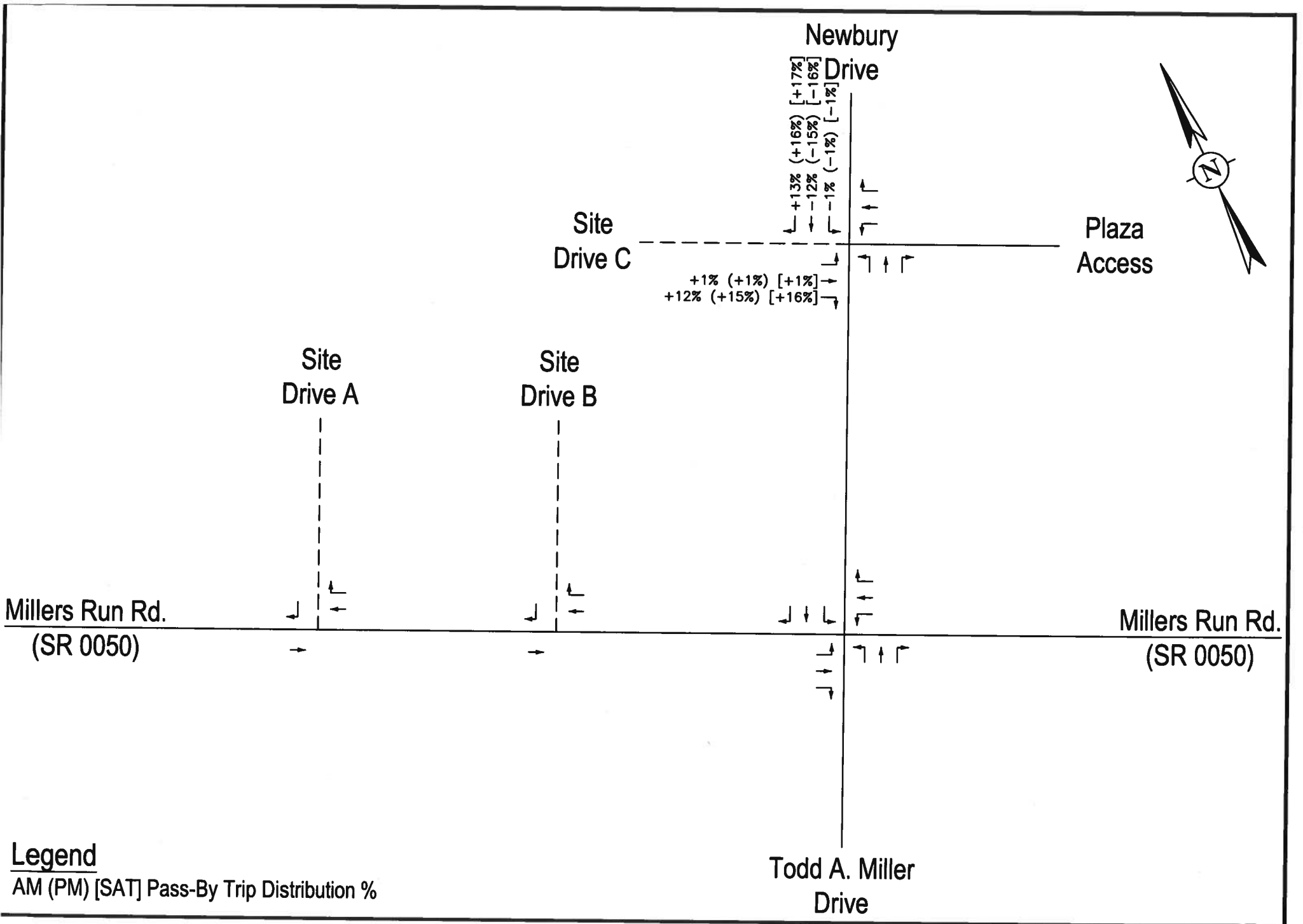


Legend
 AM (PM) [SAT] Primary Trips

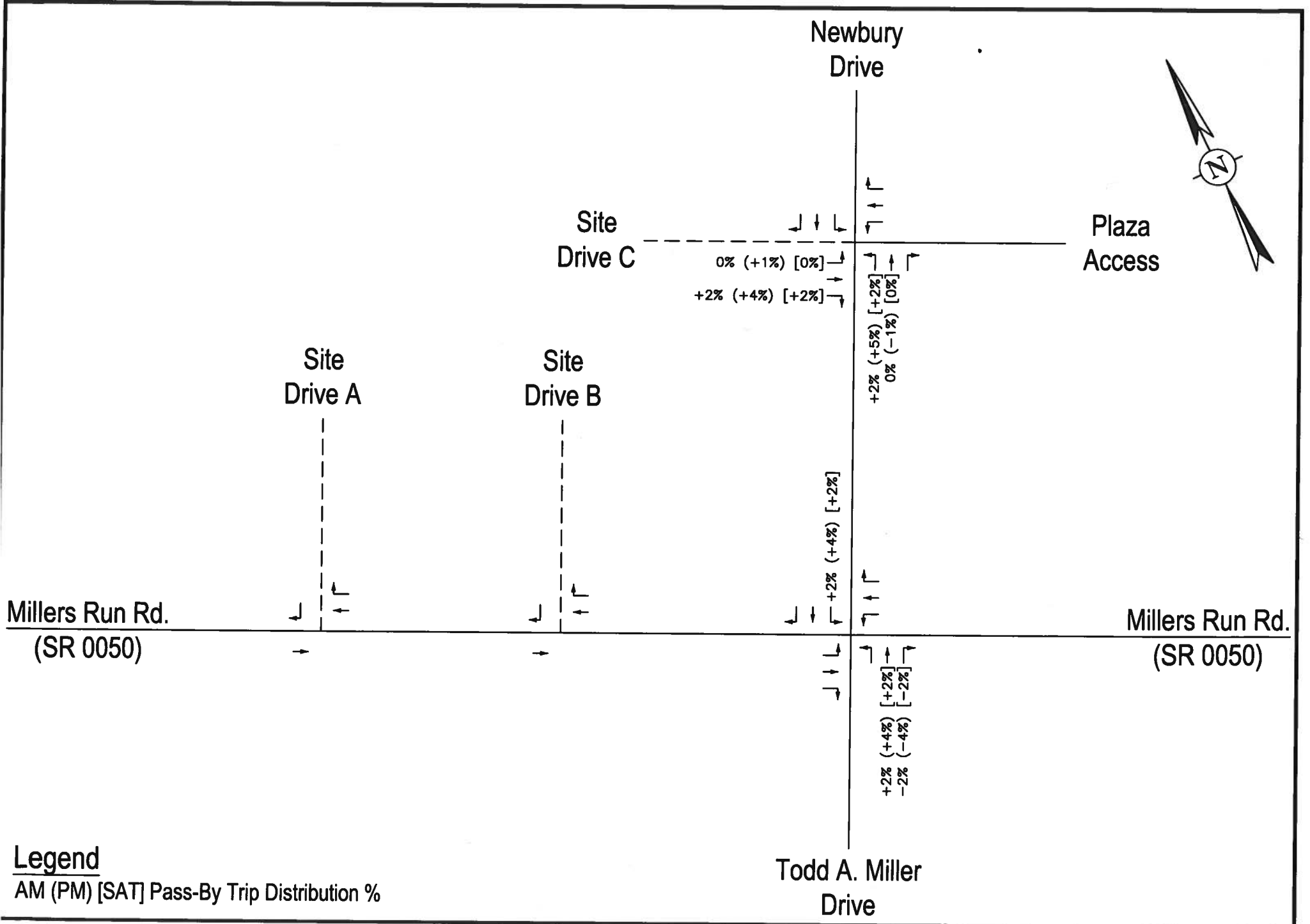
**PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Site-Generated Primary Trips**



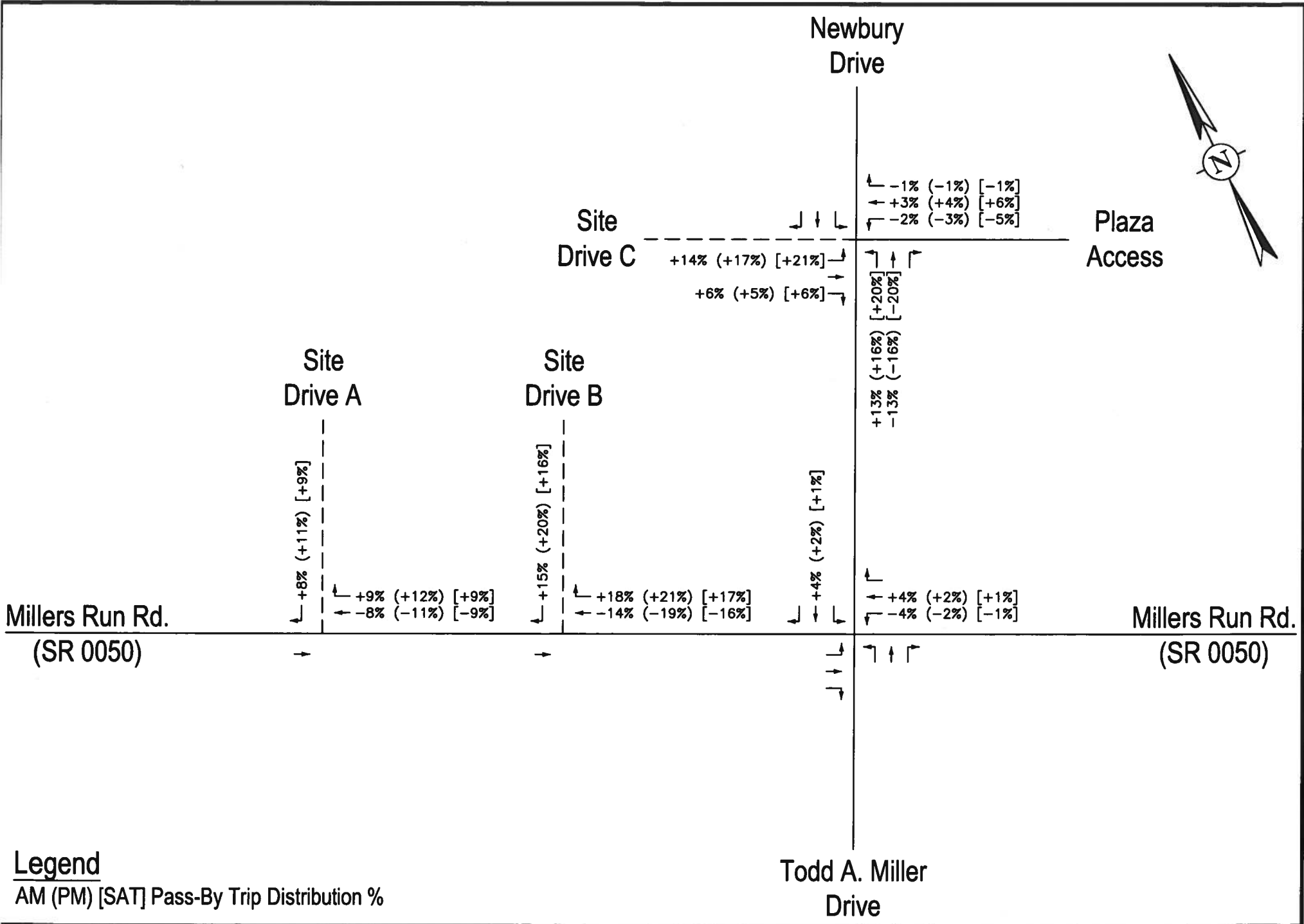
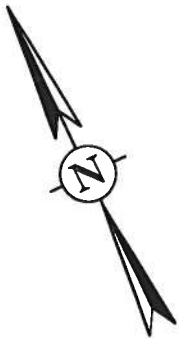
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Intersection Distribution Percentages



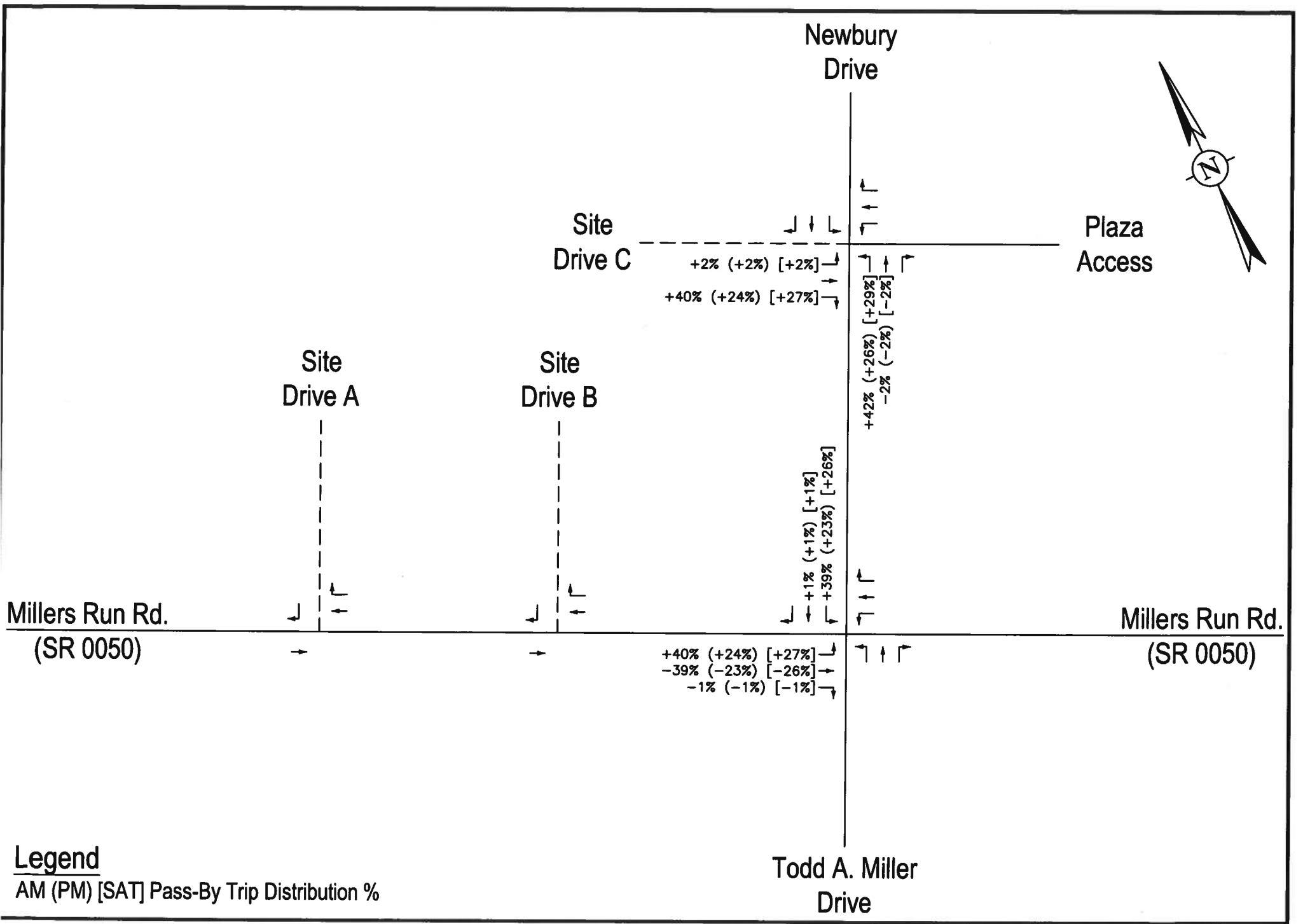
PROPOSED RETAIL DEVELOPMENT - South Fayette Township
 Pass-By Trip Distribution Percentages (From North)



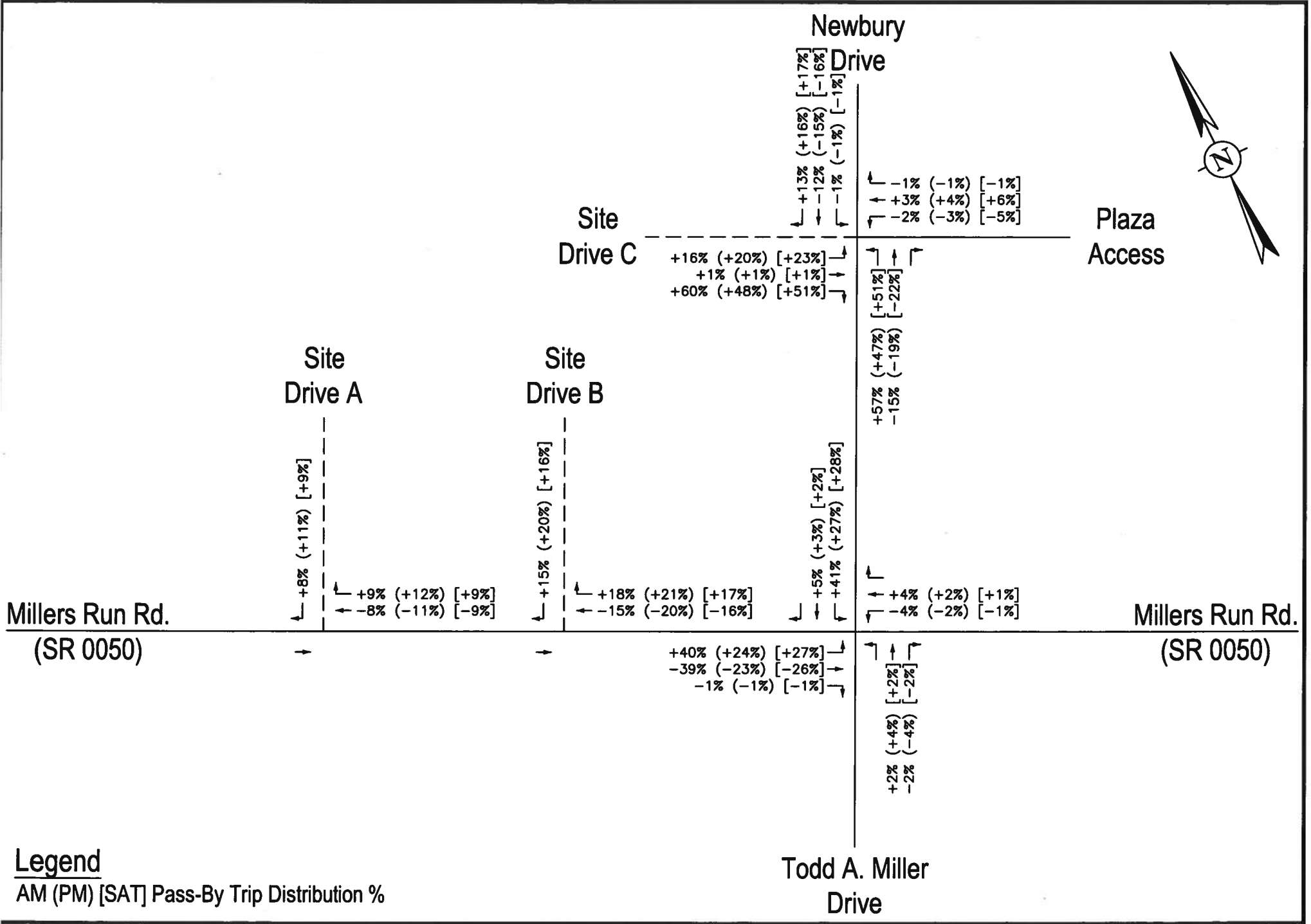
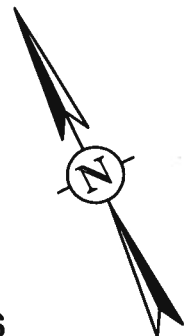
PROPOSED RETAIL DEVELOPMENT - South Fayette Township
Pass-By Trip Distribution Percentages (From South)



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trip Distribution Percentages (From East)



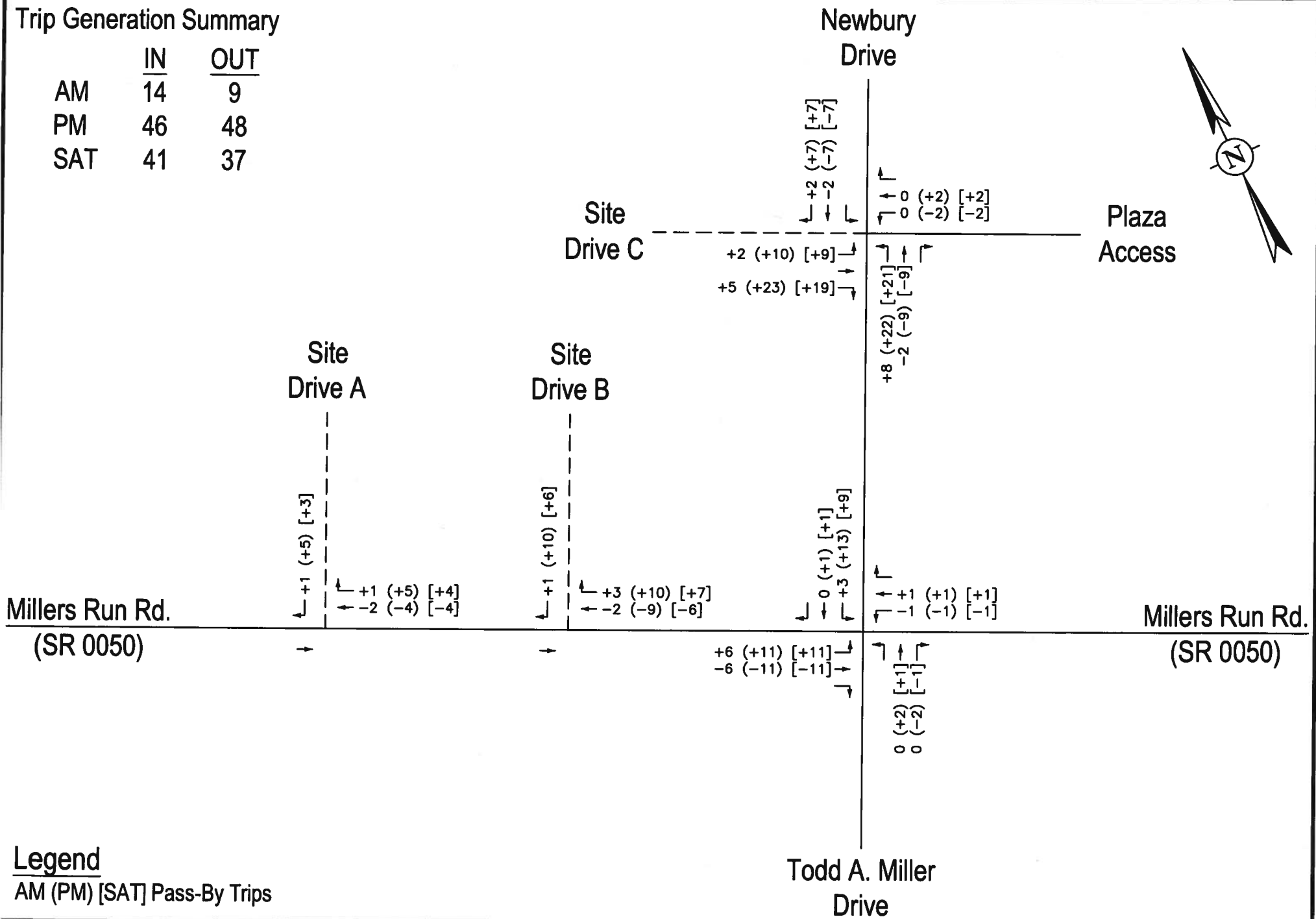
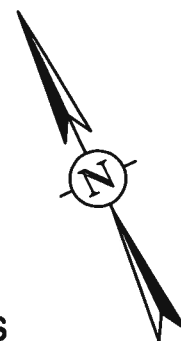
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Pass-By Trip Distribution Percentages (From West)



PROPOSED RETAIL DEVELOPMENT - South Fayette Township
Pass-By Trip Distribution Percentages (Total)

Trip Generation Summary

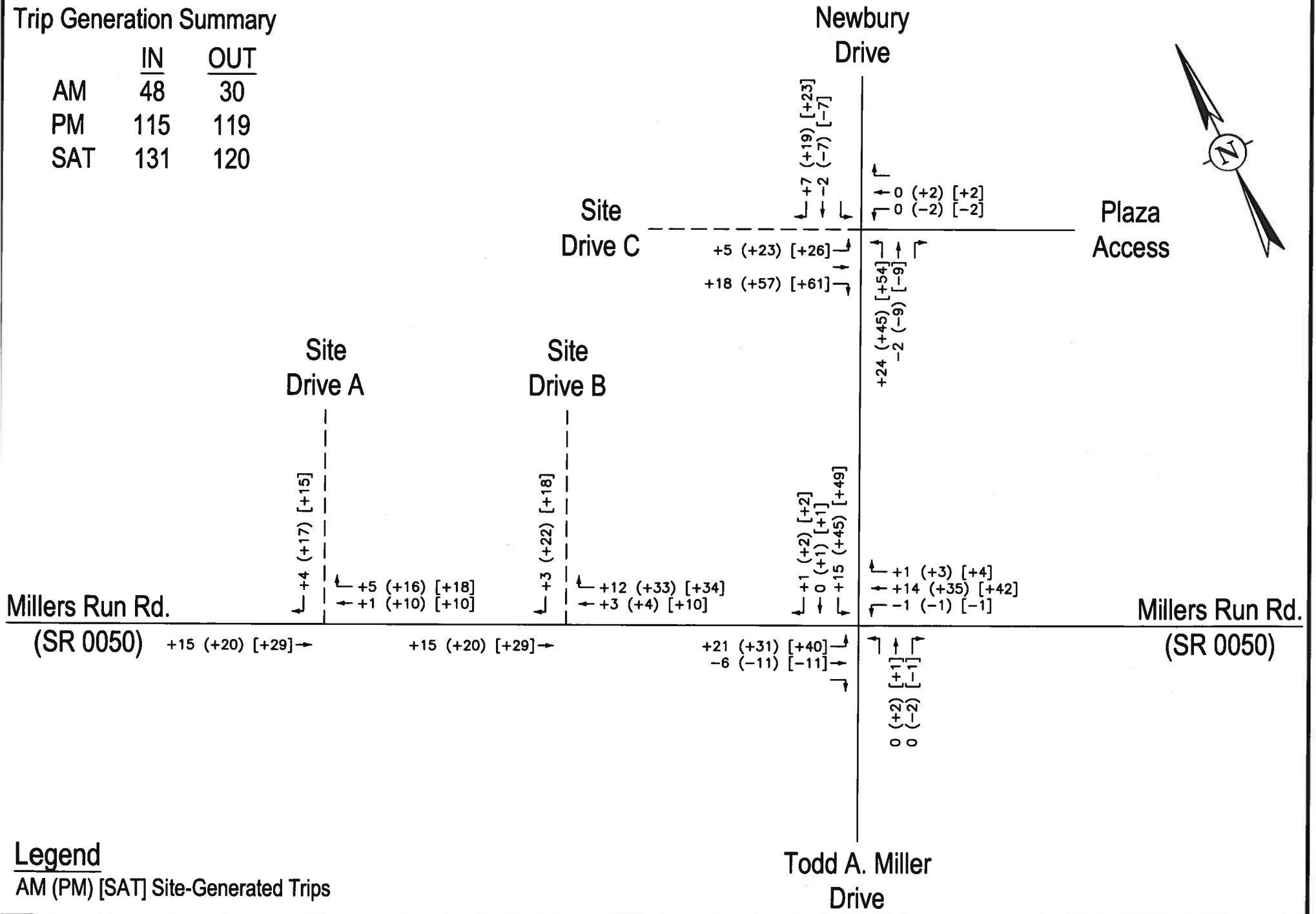
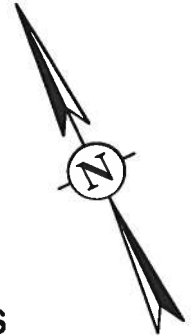
	IN	OUT
AM	14	9
PM	46	48
SAT	41	37



PROPOSED RETAIL DEVELOPMENT - South Fayette Township
 Site-Generated Pass-By Trips

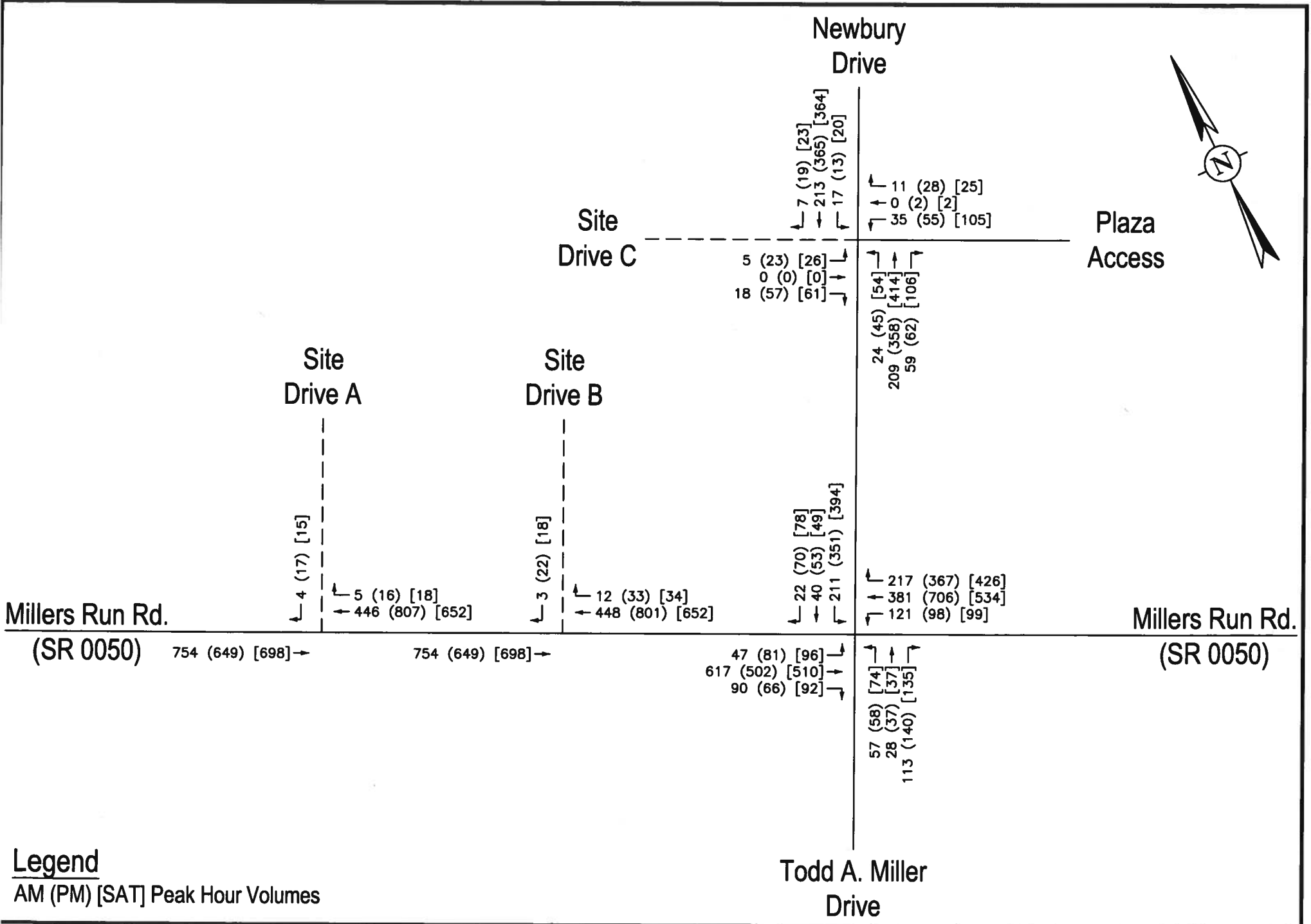
Trip Generation Summary

	<u>IN</u>	<u>OUT</u>
AM	48	30
PM	115	119
SAT	131	120

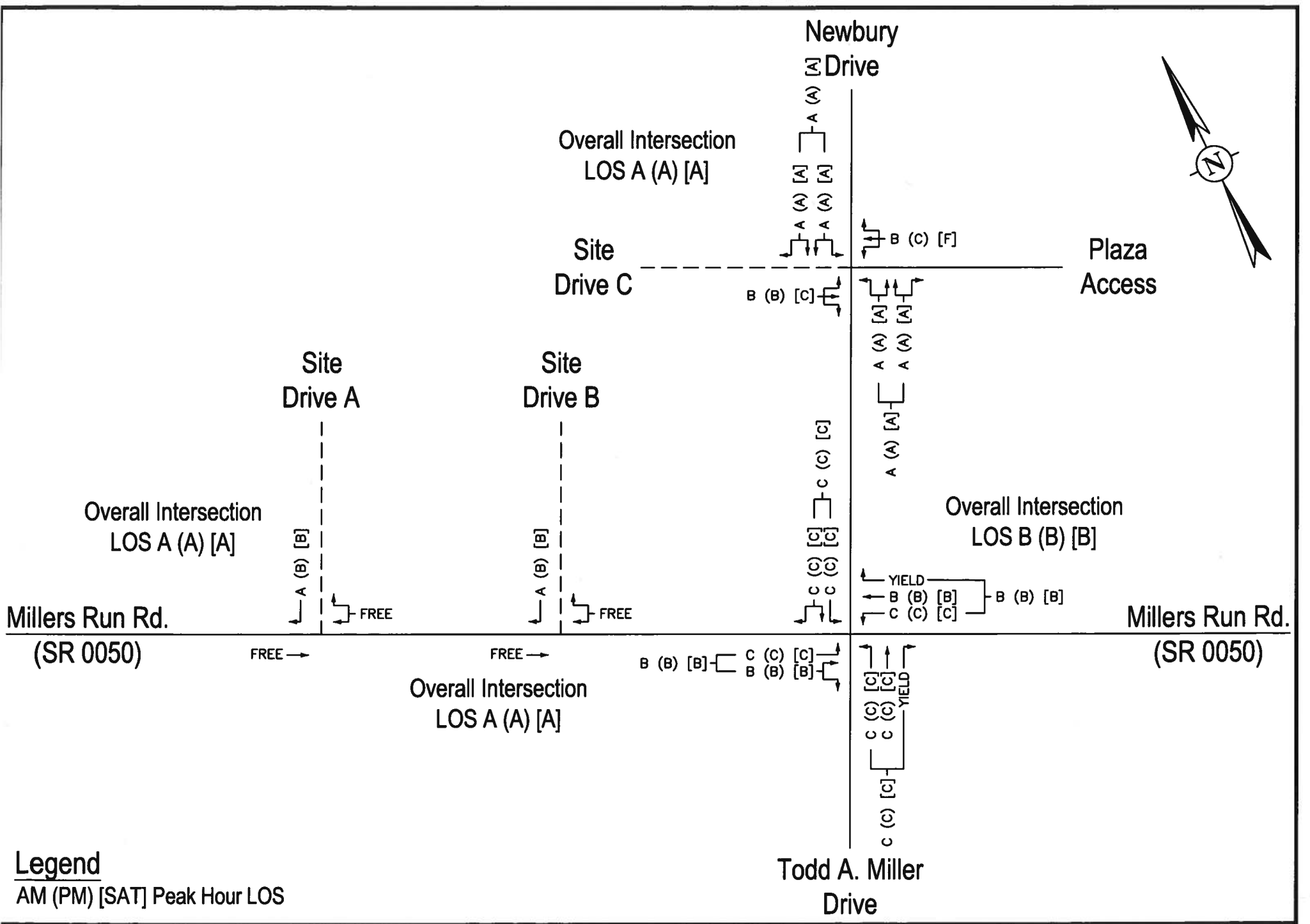


Legend
 AM (PM) [SAT] Site-Generated Trips

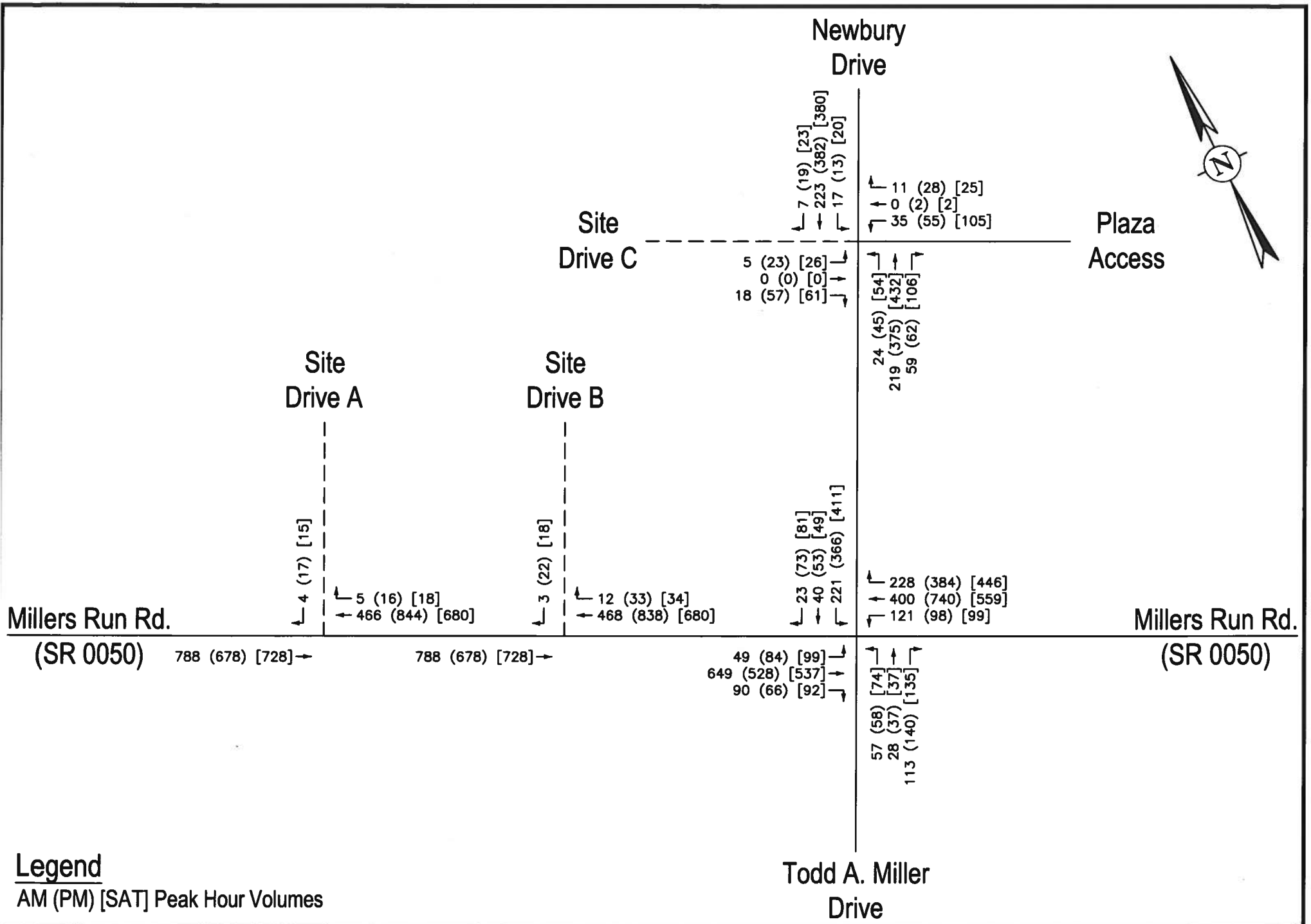
PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Total Site-Generated Trips



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Opening Year 2024 With Development Condition Peak Hour Traffic Volumes

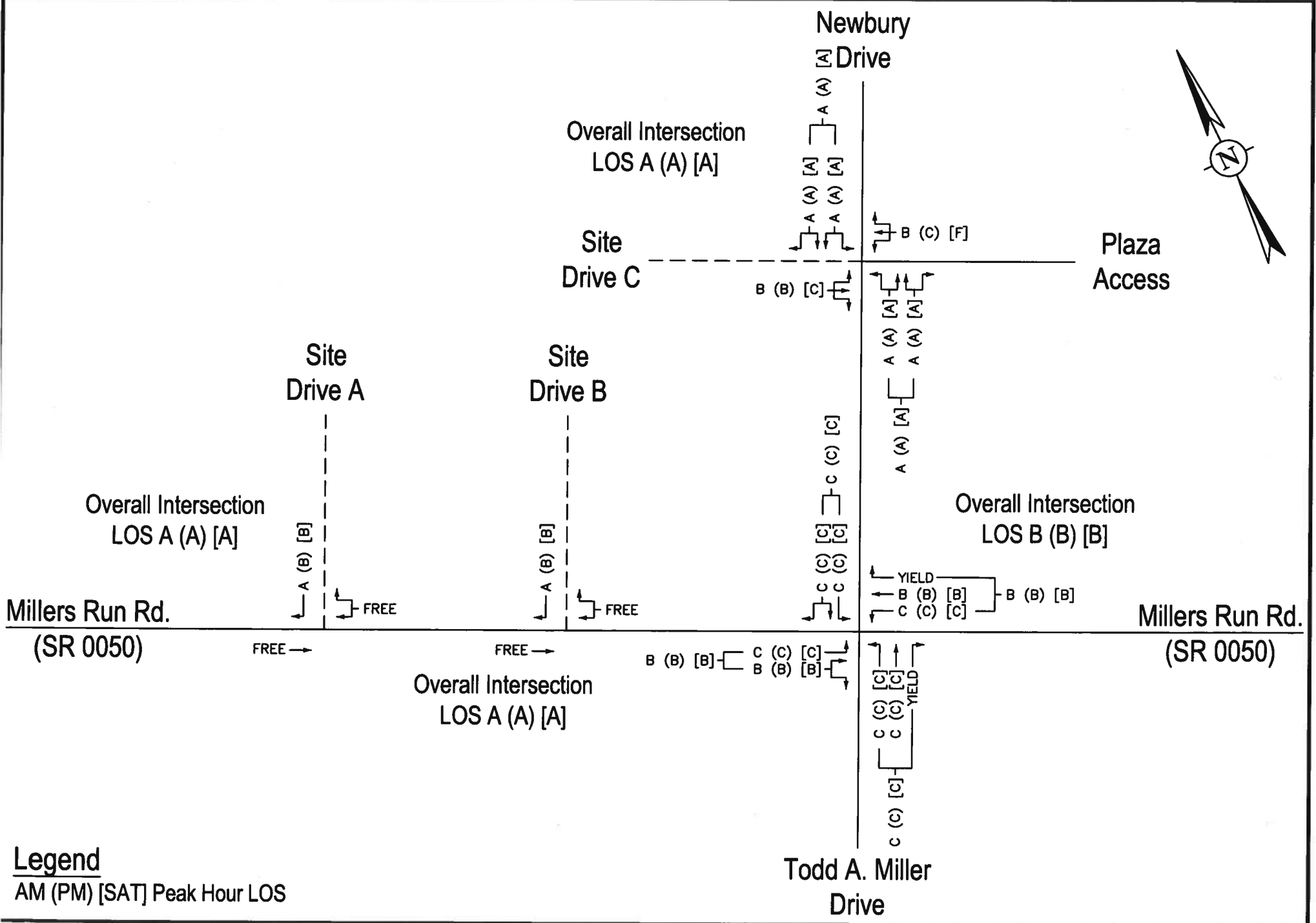
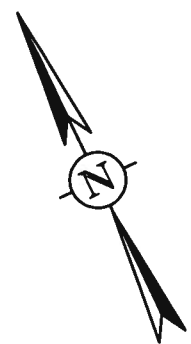


PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Opening Year 2024 With Development Condition Peak Hour LOS



PROPOSED RETAIL DEVELOPMENT – South Fayette Township
Design Year 2029 With Development Condition Peak Hour Traffic Volumes

FIGURE 19a
4392-092723



Legend
 AM (PM) [SAT] Peak Hour LOS

PROPOSED RETAIL DEVELOPMENT – South Fayette Township
 Design Year 2029 With Development Condition Peak Hour LOS

APPENDICES

APPENDIX A

TIS Scoping Checklist

**TRANSPORTATION IMPACT STUDY (TIS)
SCOPING MEETING CHECKLIST**

Scoping Meeting Date: Thursday, September 14, 2023 – 10:00 AM – via Microsoft Teams

Applicant: Cozza Enterprises, LLC

Applicants Consultant: David E. Wooster and Associates, LLC Phone: 412-921-3303

Applicant's Primary Contact: Craig Cozza Phone: 412-417-9700

(1) LOCATION OF PROPOSED DEVELOPMENT:

*Site location map is attached to this checklist.

PennDOT Engineering Dist.: 11-0 County: Allegheny

Municipality: South Fayette Township

State Route(s): SR 0050

From Segment / Offset: 0090 / 1000

To Segment / Offset: 0090 / 1359

(2) DESCRIPTION OF PROPOSED DEVELOPMENT:

*Preliminary site plan attached to this checklist.

Proposed site access: Two (2) right-in right-out only (RIRO) site access proposed along the northern side of Millers Run Road (SR 0050); full access site drive along the western side of Newbury Drive.

Proposed land uses: Retail

Community linkages: Sidewalk requirements per South Fayette Township ordinances (if any); no additional bus stops proposed; no new cross easements proposed; pedestrian accommodations (if any) in the study area to be maintained.

(3) DEVELOPMENT SCHEDULE AND STAGING:

Anticipated Opening Date: 2024

Full Buildout Date: -

Describe Proposed Development Schedule/Staging:

None.

(4) TRIP GENERATION:

Trip generation for the proposed development will be based on:

ITE Trip Generation Manual

- LU Code #821 (Shopping Plaza 40-150k without Supermarket) with gross floor area as the independent variable.

Other independent surveys

List land development and trip generation information, as appropriate. If necessary, attach additional sheets to indicate additional land uses or development phases.

Land Use(s)	Size	ADT (In / Out)	Peak Hour Trips (In / Out)		
			AM Peak	PM Peak	SAT Peak
#821	~ 45,126 SF	3,048 (1,524 / 1,524)	78 (48 / 30)	234 (115 / 119)	251 (131 / 120)
		<i>Primary</i>	55 (34 / 21)	140 (69 / 71)	173 (90 / 83)
		<i>Pass-By</i>	23 (14 / 9)	94 (46 / 48)	78 (41 / 37)
	Totals	3,048 (1,524 / 1,524)	78 (48 / 30)	234 (115 / 119)	251 (131 / 120)

*The 11th Edition of the Trip Generation Manual was used.

(5) ESTIMATED DAILY TRIP GENERATION/DRIVEWAY CLASSIFICATION:

- (a) Estimated Daily Trip Generation of Proposed Development – Assuming One Access Point and Full Buildout/Occupancy of Entire Tract: **3,048** trips/day

(b) Driveway Classification Based on Trip Generation and One Access Point:

Minimum Use: _____ Medium Volume: _____

Low Volume: _____ High Volume: X

(6) TRAFFIC IMPACT STUDY REQUIRED?

 No

 X Yes, based on: X 3,000 or more vehicle trips/day generated
 X During any one-hour time period, 100 or more new (added)
vehicle trips generated entering or 100 or more new (added)
vehicle trips generated exiting development

 Other considerations described below:

(7) TRAFFIC IMPACT ASSESSMENT REQUIRED?

 X No
 Yes

If a TIS or TIA is required, the following sections of this checklist will be discussed at the TIS Scoping Meeting. The applicant may provide preliminary information.

(8) TIS STUDY AREA:

(a) Roadway and Study Intersections

- **Millers Run Road (SR 0050) with Newbury Drive/Todd A. Miller Drive – Existing Signalized**
- **Proposed Site Drives**

(b) Land Use Context

Suburban Corridor

(c) Known Congestion Areas

- **Millers Run Road (SR 0050) with Newbury Drive/Todd A. Miller Drive**
- **Proposed Cane's Restaurant (Concern by Township/PennDOT)**

(d) Known Safety Concerns

None.

(e) Known Environmental Constraints

None.

(f) Pedestrian / Bike Review

Not Applicable.

(g) Transit Review

Not Applicable.

(9) STUDY AREA TYPE Urban X Rural

(10) TIS ANALYSIS PERIODS AND TIMES:

**Existing Year 2023 Condition
Opening Year 2024 Without and With Development
Design Year 2029 Without and With Development**

(11) TRAFFIC ADJUSTMENT FACTORS:

(a) Seasonal Adjustment:

No Seasonal Adjustment proposed. Counts will be performed during a typical weekday.
Source

(b) Annual Base Traffic Growth: 1.00% %/yr (linear) SPC – 4/25/2023

(c) Pass-By Trips:

<u>Land Use</u>	<u>%</u>	<u>Source – ITETripGen Web-based App</u>
#821	30% AM	PM – 10% 2021 Pass-By Rates (PM) 2021 Pass-By Rates (SAT)
	40% PM	
	31% SAT	

(d) Captured Trips for Multi-Use Sites:

Not Applicable.

(e) Modal Split Reductions:

Not Applicable.

(f) Other Reductions:

Not Applicable.

(12) OTHER PROJECTS WITHIN STUDY AREA TO BE ADDED TO BASE TRAFFIC:

- **The Piazza Development**
 - **6,800 SF High-Turnover (Sit-Down) Restaurant**
 - **4,000 SF Fast Food Restaurant with Drive-Through Window**
 - **4,250 SF Fast Food Restaurant with Drive-Through Window (Cane's)**
- **South Fayette Commons Development**
- **Cigar, Bar, & Restaurant (Newbury)**

(13) TRIP DISTRIBUTION AND ASSIGNMENT:

Distribution of the development trips will be based on the turning movement count data at the existing study intersection, as well as engineering judgment relative to the convenience of accessing the site from various directions.

(14) APPROVAL OF DATA COLLECTION ELEMENTS AND METHODOLOGIES:

<u>Location</u>	<u>Period</u>	<u>Type</u>
Existing intersection(s) listed in Section (8a) above	7:00-9:00 am (T-Th) 4:00-6:00 pm (T-Th) 11:00 am-2:00 pm (Sat)	Turning Movement Counts

(15) CAPACITY/LOS ANALYSES:

<u>Location</u>	<u>Period</u>	<u>Type</u>
Intersections listed in Section (8a)	AM, PM, & SAT Peak Hours	HCM 6 th Ed. Synchro 11

(16) ROADWAY IMPROVEMENTS/MODIFICATIONS BY OTHERS TO BE INCLUDED:

None.

(17) OTHER NEEDED ANALYSES:

(a) Sight Distance Analyses:

Yes – at all proposed site accesses.

(b) Signal Warrant Analysis:

If/as necessary.

(c) Required Signal Phasing/Timing Modifications:

If/as necessary.

(d) Traffic Signal Corridor/Network Analyses:

If/as necessary.

(e) Analyses of the Need for Turning Lanes:

Wooster will compare forecasted traffic volumes at the proposed site access with criteria outlined in Publication 46, Chapter 11 Traffic Studies, dated 2012 for the consideration of auxiliary turn lanes under future With Development conditions.

(f) Turning Lane Lengths:

Length of any required turn lanes will be based on criteria outlined in Publication 46, Chapter 11 Traffic Studies, dated 2012, which includes SimTraffic (queuing) analyses. Existing turn lane lengths will be evaluated to ensure that they are equipped to adequately accommodate projected traffic volumes.

(g) Left Turn Signal Phasing Analyses:

If/as necessary.

(h) Queuing Analyses:

Yes – queue analyses will be performed using SimTraffic to determine if existing turn lanes are of sufficient length to accommodate the projected traffic. To perform these analyses, five (5) separate 60-minute simulations with a 10-minute seeding interval will be evaluated for each peak hour and averaged.

(i) Gap Studies:

Not Applicable.

(j) Crash Analyses:

Yes – reportable crash data will be obtained from the Pennsylvania Department of Transportation (PennDOT) Pennsylvania Crash Information Tool (PCIT) for the study area.

(k) Weaving Analyses:

Not Applicable.

(l) Other Required Studies:

None.

(18) ADDITIONAL COMMENTS OR RECOMMENDATIONS RELATIVE TO THE SCOPE OF THE TIS:

None.



Joshua A. Haydo, P.E., PTOE

Date: 10-11-2023

Signature of Applicant's Engineer
Wooster and Associates

Date: _____

Signature of District Traffic PennDOT Representative
PennDOT District 11-0

Date: _____

Signature of District Permit PennDOT Representative
PennDOT District 11-0

Date: _____

Signature of Municipal Representative
South Fayette Township

APPENDIX B

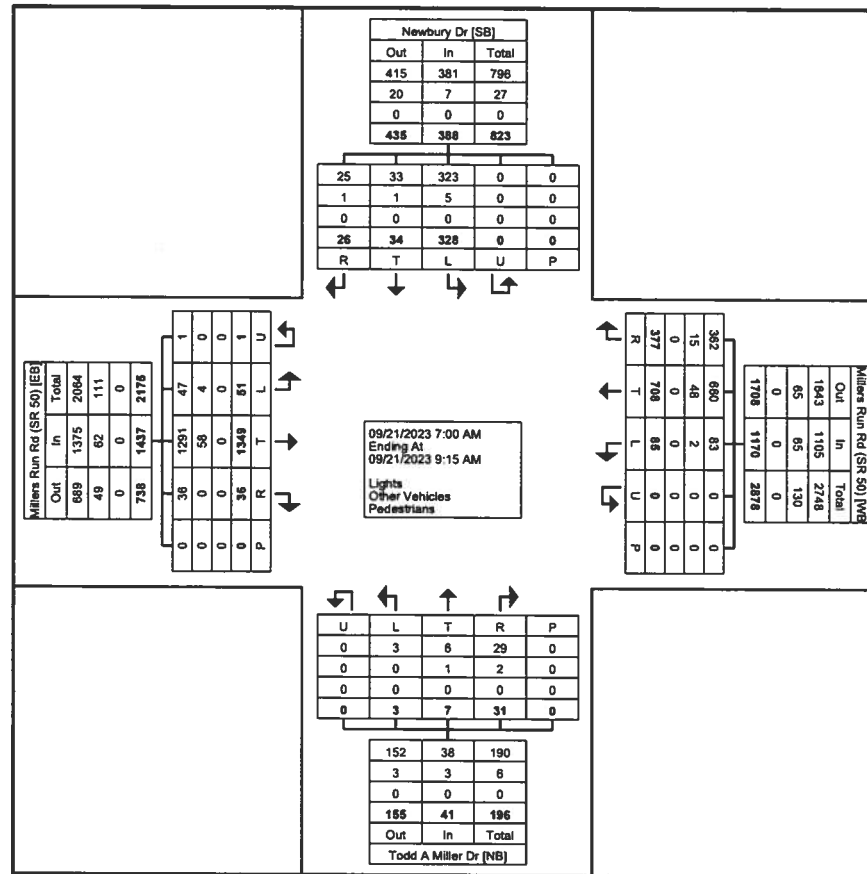
Turning Movement Counts



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (7-9 am)
Site Code: 4392
Start Date: 09/21/2023
Page No: 2



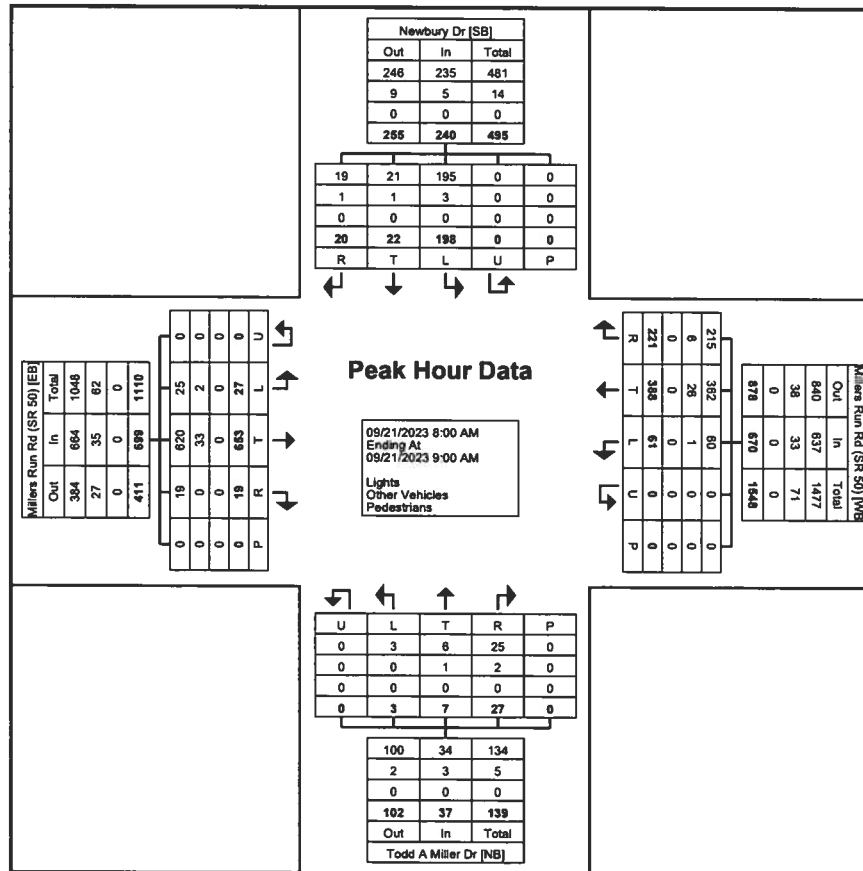
Turning Movement Data Plot



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (7-9 am)
Site Code: 4392
Start Date: 09/21/2023
Page No: 4



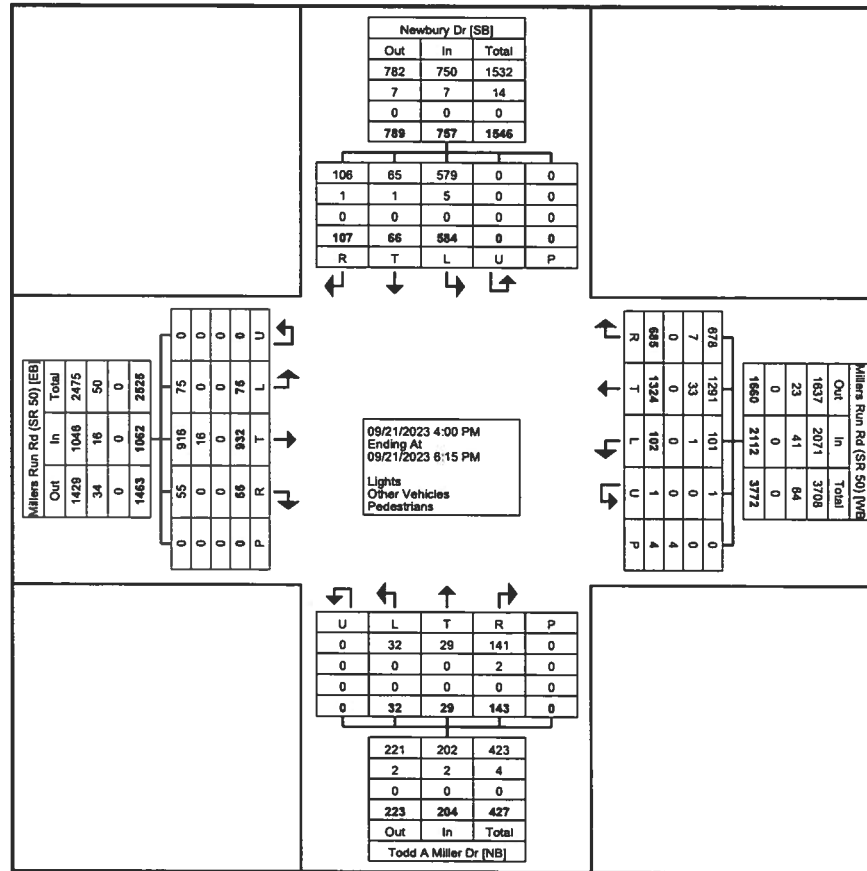
Turning Movement Peak Hour Data Plot (8:00 AM)



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023
Page No: 2



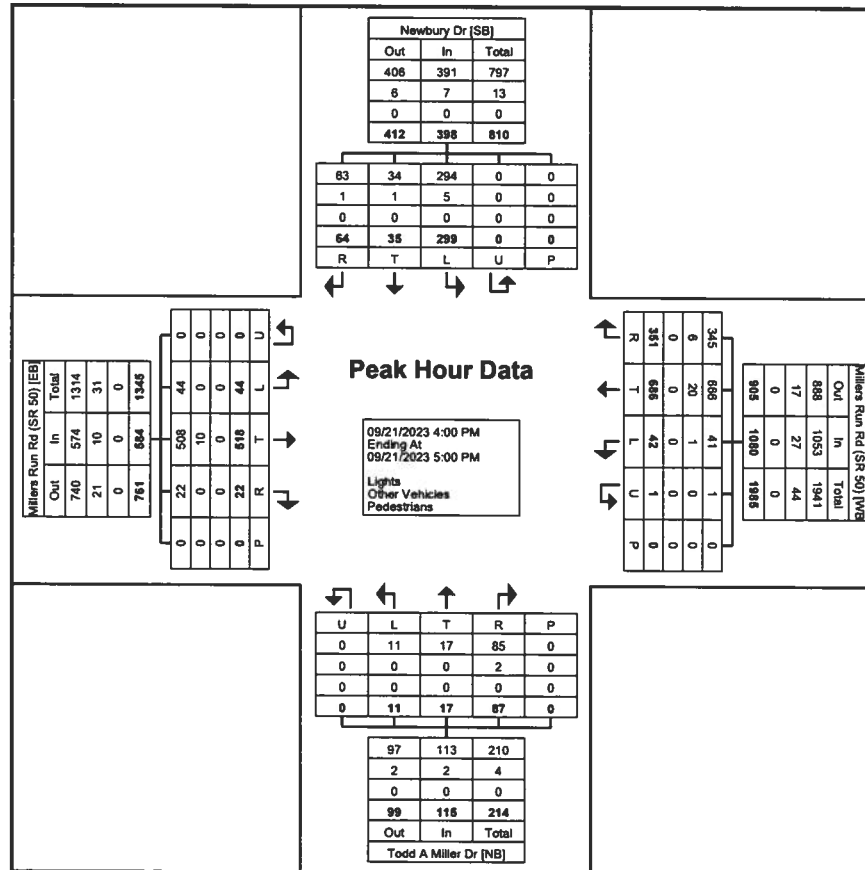
Turning Movement Data Plot



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023
Page No: 4



Turning Movement Peak Hour Data Plot (4:00 PM)



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023
Page No: 1

Turning Movement Data

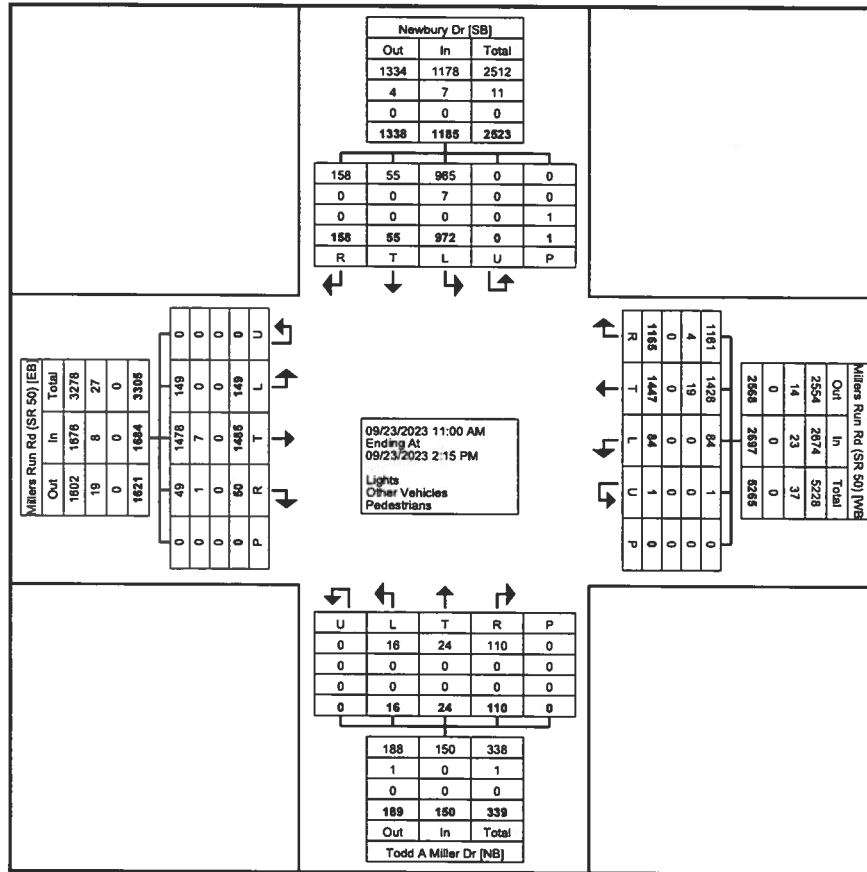
Start Time	Millers Run Rd (SR 50) Eastbound							Millers Run Rd (SR 50) Westbound							Todd A Miller Dr Northbound							Newbury Dr Southbound							Int. Total
	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	U-Turn	Left	Thru	Right	Right on Red	Peds	App. Total	
11:00 AM	0	16	116	2	0	0	134	1	2	121	79	24	0	227	0	0	1	3	5	0	9	0	64	8	2	5	0	79	449
11:15 AM	0	10	130	4	0	0	144	0	7	138	62	17	0	224	0	0	1	5	5	0	11	0	88	8	3	9	0	108	487
11:30 AM	0	13	108	2	0	0	123	0	9	78	96	15	0	198	0	2	0	0	2	0	4	0	73	1	4	10	0	88	413
11:45 AM	0	12	119	6	0	0	137	0	12	111	71	22	0	216	0	2	3	2	6	0	13	0	100	9	4	8	1	121	487
Hourly Total	0	51	473	14	0	0	538	1	30	448	308	78	0	865	0	4	5	10	18	0	37	0	325	26	13	32	1	396	1836
12:00 PM	0	9	132	3	0	0	144	0	7	117	57	15	0	196	0	1	6	5	3	0	15	0	71	3	5	7	0	86	441
12:15 PM	0	10	171	5	2	0	188	0	6	113	82	19	0	220	0	2	1	2	6	0	11	0	81	7	5	3	0	96	515
12:30 PM	0	14	130	9	2	0	155	0	5	115	78	21	0	219	0	2	0	4	2	0	8	0	75	3	5	9	0	92	474
12:45 PM	0	11	122	1	0	0	134	0	6	142	70	38	0	256	0	1	5	3	5	0	14	0	104	4	14	16	0	138	542
Hourly Total	0	44	555	18	4	0	621	0	24	487	287	93	0	891	0	6	12	14	16	0	48	0	331	17	29	35	0	412	1972
1:00 PM	0	9	108	2	0	0	119	0	10	134	74	23	0	241	0	0	2	9	2	0	13	0	78	6	9	7	0	100	473
1:15 PM	0	11	106	3	1	0	121	0	8	124	65	30	0	227	0	1	1	3	5	0	10	0	77	4	4	3	0	88	446
1:30 PM	0	19	129	4	0	0	152	0	6	140	86	32	0	264	0	4	2	8	12	0	26	0	83	2	5	5	0	95	537
1:45 PM	0	15	114	4	0	0	133	0	6	114	64	25	0	209	0	1	2	3	10	0	16	0	78	0	6	10	0	94	452
Hourly Total	0	54	457	13	1	0	525	0	30	512	289	110	0	941	0	6	7	23	29	0	65	0	316	12	24	25	0	377	1908
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	149	1485	45	5	0	1684	1	84	1447	884	281	0	2697	0	16	24	47	63	0	150	0	972	55	66	92	1	1185	5716
Approach %	0.0	8.8	88.2	2.7	0.3	-	-	0.0	3.1	53.7	32.8	10.4	-	-	0.0	10.7	16.0	31.3	42.0	-	-	0.0	82.0	4.6	5.6	7.8	-	-	-
Total %	0.0	2.6	26.0	0.8	0.1	-	29.5	0.0	1.5	25.3	15.5	4.9	-	47.2	0.0	0.3	0.4	0.8	1.1	-	2.6	0.0	17.0	1.0	1.2	1.6	-	20.7	-
Lights	0	149	1478	44	5	-	1676	1	84	1428	880	281	-	2674	0	16	24	47	63	-	150	0	965	55	66	92	-	1178	5678
% Lights	-	100.0	99.5	97.8	100.0	-	99.5	100.0	100.0	98.7	99.5	100.0	-	99.1	-	100.0	100.0	100.0	100.0	-	100.0	-	99.3	100.0	100.0	100.0	-	99.4	99.3
Other Vehicles	0	0	7	1	0	-	8	0	0	19	4	0	-	23	0	0	0	0	0	-	0	0	7	0	0	0	-	7	38
% Other Vehicles	-	0.0	0.5	2.2	0.0	-	0.5	0.0	0.0	1.3	0.5	0.0	-	0.9	-	0.0	0.0	0.0	0.0	-	0.0	-	0.7	0.0	0.0	0.0	-	0.6	0.7
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023
Page No: 2



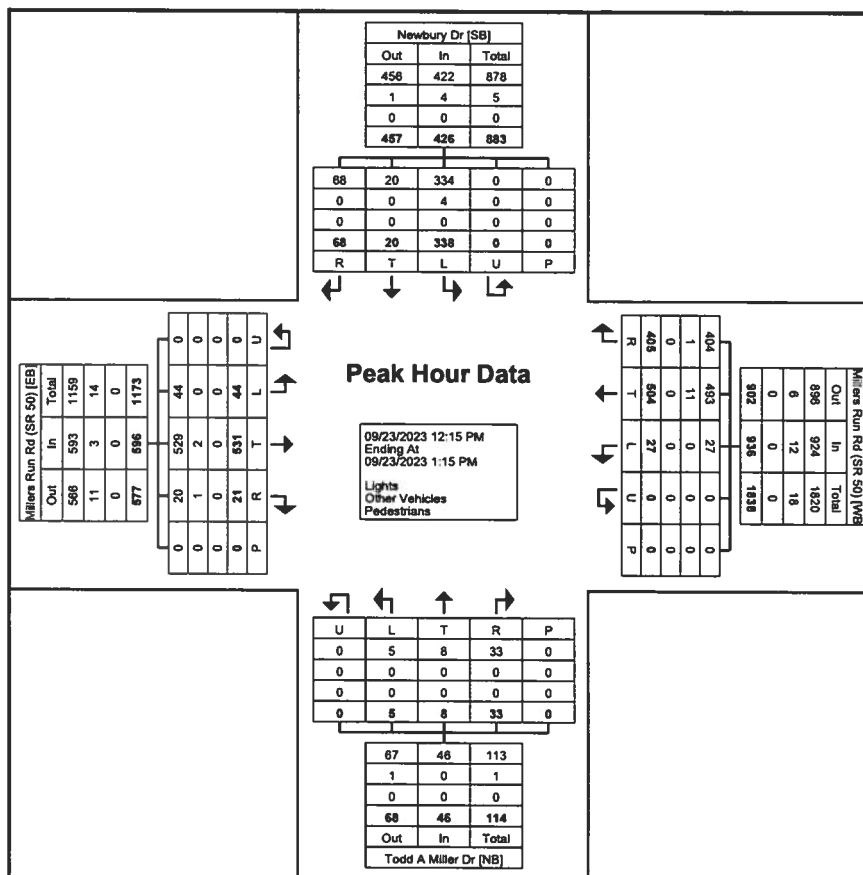
Turning Movement Data Plot



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: SR 50 & Newbury Dr. (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023
Page No: 4



Turning Movement Peak Hour Data Plot (12:15 PM)



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (7-9 am)
Site Code: 4392
Start Date: 09/21/2023
Page No: 1

Turning Movement Data

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	37	7	0	44	0	0	32	0	0	32	77
7:15 AM	0	0	0	0	0	0	0	5	0	0	0	5	0	0	21	15	0	36	0	2	25	0	0	27	68
7:30 AM	0	0	0	0	0	0	0	5	0	0	0	5	0	0	29	6	0	35	0	1	37	0	0	38	78
7:45 AM	0	0	0	0	0	0	0	4	0	2	0	6	0	0	46	20	0	66	0	2	47	0	0	49	121
Hourly Total	0	0	0	0	0	0	0	14	0	3	0	17	0	0	133	48	0	181	0	5	141	0	0	146	344
8:00 AM	0	0	0	0	0	0	0	10	0	3	0	13	0	0	35	15	0	50	0	4	37	0	0	41	104
8:15 AM	0	0	0	0	0	0	0	6	0	4	0	10	0	0	46	13	0	59	0	5	42	0	0	47	116
8:30 AM	0	0	0	0	0	0	0	11	0	1	0	12	1	0	57	16	0	74	0	3	63	0	0	66	152
8:45 AM	0	0	0	0	0	0	0	8	0	3	2	11	0	0	58	15	0	73	0	5	55	0	0	60	144
Hourly Total	0	0	0	0	0	0	0	35	0	11	2	46	1	0	196	59	0	256	0	17	197	0	0	214	516
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	49	0	14	2	63	1	0	329	107	0	437	0	22	338	0	0	360	860
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	77.8	0.0	22.2	-	-	0.2	0.0	75.3	24.5	-	-	0.0	6.1	93.9	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	5.7	0.0	1.6	-	7.3	0.1	0.0	38.3	12.4	-	50.8	0.0	2.6	39.3	0.0	-	41.9	-
Lights	0	0	0	0	-	0	0	48	0	14	-	62	1	0	310	107	-	418	0	22	330	0	-	352	832
% Lights	-	-	-	-	-	-	-	98.0	-	100.0	-	98.4	100.0	-	94.2	100.0	-	95.7	-	100.0	97.6	-	-	97.8	96.7
Other Vehicles	0	0	0	0	-	0	0	1	0	0	-	1	0	0	19	0	-	19	0	0	8	0	-	8	28
% Other Vehicles	-	-	-	-	-	-	-	2.0	-	0.0	-	1.6	0.0	-	5.8	0.0	-	4.3	-	0.0	2.4	-	-	2.2	3.3
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (7-9 am)
Site Code: 4392
Start Date: 09/21/2023
Page No: 3

Turning Movement Peak Hour Data (8:00 AM)

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
8:00 AM	0	0	0	0	0	0	0	10	0	3	0	13	0	0	35	15	0	50	0	4	37	0	0	41	104
8:15 AM	0	0	0	0	0	0	0	6	0	4	0	10	0	0	46	13	0	59	0	5	42	0	0	47	116
8:30 AM	0	0	0	0	0	0	0	11	0	1	0	12	1	0	57	16	0	74	0	3	63	0	0	66	152
8:45 AM	0	0	0	0	0	0	0	8	0	3	2	11	0	0	58	15	0	73	0	5	55	0	0	60	144
Total	0	0	0	0	0	0	0	35	0	11	2	46	1	0	196	59	0	256	0	17	197	0	0	214	516
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	76.1	0.0	23.9	-	-	0.4	0.0	76.6	23.0	-	49.6	0.0	7.9	92.1	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	6.8	0.0	2.1	-	8.9	0.2	0.0	38.0	11.4	-	49.6	0.0	3.3	38.2	0.0	-	41.5	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.795	0.000	0.688	-	0.885	0.250	0.000	0.845	0.922	-	0.865	0.000	0.850	0.782	0.000	-	0.811	0.849
Lights	0	0	0	0	-	0	0	34	0	11	-	45	1	0	187	59	-	247	0	17	193	0	-	210	502
% Lights	-	-	-	-	-	-	-	97.1	-	100.0	-	97.8	100.0	-	95.4	100.0	-	96.5	-	100.0	98.0	-	-	98.1	97.3
Other Vehicles	0	0	0	0	-	0	0	1	0	0	-	1	0	0	9	0	-	9	0	0	4	0	-	4	14
% Other Vehicles	-	-	-	-	-	-	-	2.9	-	0.0	-	2.2	0.0	-	4.6	0.0	-	3.5	-	0.0	2.0	-	-	1.9	2.7
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

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Count Name: Newbury Dr. & Plaza Access (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023
Page No: 1

Turning Movement Data

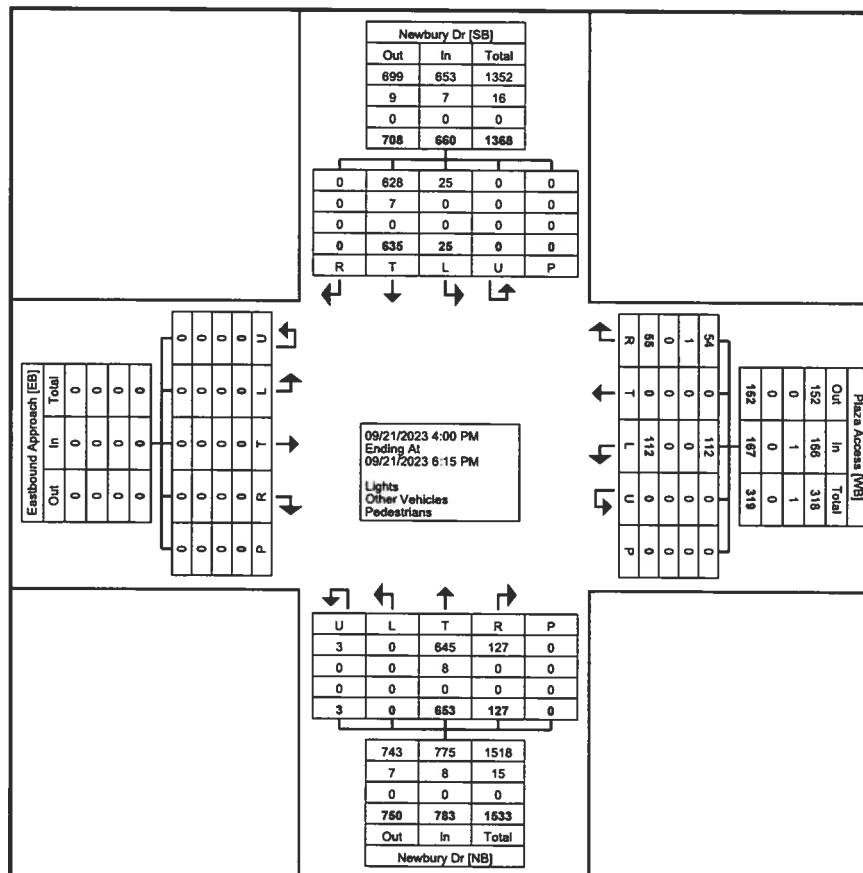
Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:00 PM	0	0	0	0	0	0	0	10	0	9	0	19	0	0	75	11	0	86	0	4	115	0	0	119	224
4:15 PM	0	0	0	0	0	0	0	17	0	7	0	24	0	0	64	12	0	76	0	6	80	0	0	86	186
4:30 PM	0	0	0	0	0	0	0	15	0	5	0	20	0	0	101	17	0	118	0	1	65	0	0	66	204
4:45 PM	0	0	0	0	0	0	0	15	0	7	0	22	0	0	91	22	0	113	0	2	74	0	0	76	211
Hourly Total	0	0	0	0	0	0	0	57	0	28	0	85	0	0	331	62	0	393	0	13	334	0	0	347	825
5:00 PM	0	0	0	0	0	0	0	19	0	9	0	28	0	0	82	15	0	97	0	4	76	0	0	80	205
5:15 PM	0	0	0	0	0	0	0	18	0	8	0	26	0	0	69	15	0	84	0	5	66	0	0	71	181
5:30 PM	0	0	0	0	0	0	0	7	0	8	0	15	0	0	89	18	0	107	0	0	81	0	0	81	203
5:45 PM	0	0	0	0	0	0	0	11	0	2	0	13	3	0	82	17	0	102	0	3	78	0	0	81	196
Hourly Total	0	0	0	0	0	0	0	55	0	27	0	82	3	0	322	65	0	390	0	12	301	0	0	313	785
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	112	0	55	0	167	3	0	653	127	0	783	0	25	635	0	0	660	1610
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	67.1	0.0	32.9	-	-	0.4	0.0	83.4	16.2	-	-	0.0	3.8	96.2	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	7.0	0.0	3.4	-	10.4	0.2	0.0	40.6	7.9	-	48.6	0.0	1.6	39.4	0.0	-	41.0	-
Lights	0	0	0	0	-	0	0	112	0	54	-	166	3	0	645	127	-	775	0	25	628	0	-	653	1594
% Lights	-	-	-	-	-	-	-	100.0	-	98.2	-	99.4	100.0	-	98.8	100.0	-	99.0	-	100.0	98.9	-	-	98.9	99.0
Other Vehicles	0	0	0	0	-	0	0	0	0	1	-	1	0	0	8	0	-	8	0	0	7	0	-	7	16
% Other Vehicles	-	-	-	-	-	-	-	0.0	-	1.8	-	0.6	0.0	-	1.2	0.0	-	1.0	-	0.0	1.1	-	-	1.1	1.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023
Page No: 2



Turning Movement Data Plot



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (4-6 pm)
Site Code: 4392
Start Date: 09/21/2023
Page No: 3

Turning Movement Peak Hour Data (4:00 PM)

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:00 PM	0	0	0	0	0	0	0	10	0	9	0	19	0	0	75	11	0	86	0	4	115	0	0	119	224
4:15 PM	0	0	0	0	0	0	0	17	0	7	0	24	0	0	64	12	0	76	0	6	80	0	0	86	186
4:30 PM	0	0	0	0	0	0	0	15	0	5	0	20	0	0	101	17	0	118	0	1	65	0	0	66	204
4:45 PM	0	0	0	0	0	0	0	15	0	7	0	22	0	0	91	22	0	113	0	2	74	0	0	76	211
Total	0	0	0	0	0	0	0	57	0	28	0	85	0	0	331	62	0	393	0	13	334	0	0	347	825
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	67.1	0.0	32.9	-	-	0.0	0.0	84.2	15.8	-	-	0.0	3.7	96.3	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	6.9	0.0	3.4	-	10.3	0.0	0.0	40.1	7.5	-	47.6	0.0	1.6	40.5	0.0	-	-	42.1
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.838	0.000	0.778	-	0.885	0.000	0.000	0.819	0.705	-	0.833	0.000	0.542	0.726	0.000	-	-	0.729
Lights	0	0	0	0	-	0	0	57	0	28	-	85	0	0	323	62	-	385	0	13	327	0	-	-	340
% Lights	-	-	-	-	-	-	-	100.0	-	100.0	-	100.0	-	-	97.6	100.0	-	98.0	-	100.0	97.9	-	-	-	98.0
Other Vehicles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	8	0	-	8	0	0	7	0	-	-	7
% Other Vehicles	-	-	-	-	-	-	-	0.0	-	0.0	-	0.0	-	-	2.4	0.0	-	2.0	-	0.0	2.1	-	-	-	2.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

Pittsburgh, Pennsylvania, United States 15205
412-921-3303 jnelson@dewooster.com

Count Name: Newbury Dr. & Plaza Access (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023
Page No: 1

Turning Movement Data

Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
11:00 AM	0	0	0	0	0	0	0	14	0	7	0	21	0	1	87	28	0	116	0	4	68	0	0	72	209
11:15 AM	0	0	0	0	0	0	0	29	0	3	0	32	0	0	74	18	0	92	0	3	79	0	0	82	206
11:30 AM	0	0	0	0	0	0	0	24	0	7	0	31	0	0	95	33	0	128	0	5	65	0	0	70	229
11:45 AM	0	0	0	0	0	0	0	22	0	10	0	32	0	0	71	31	0	102	0	4	98	0	0	102	236
Hourly Total	0	0	0	0	0	0	0	89	0	27	0	116	0	1	327	110	0	438	0	16	310	0	0	326	880
12:00 PM	0	0	0	0	0	0	0	24	0	4	0	28	1	0	67	21	0	89	0	4	66	0	0	70	187
12:15 PM	0	0	0	0	0	0	0	22	0	8	2	30	0	0	78	36	4	114	0	3	71	0	0	74	218
12:30 PM	0	0	0	1	0	1	0	26	0	5	0	31	0	0	91	24	0	115	0	3	66	0	0	69	216
12:45 PM	0	0	0	0	0	0	0	36	0	9	0	45	0	0	91	31	0	122	0	6	104	0	0	110	277
Hourly Total	0	0	0	1	0	1	0	108	0	26	2	134	1	0	327	112	4	440	0	16	307	0	0	323	898
1:00 PM	0	0	0	0	0	0	0	19	0	8	0	27	1	0	88	22	0	111	0	3	76	0	0	79	217
1:15 PM	0	0	0	0	0	0	0	27	0	1	0	28	0	0	85	20	0	105	0	4	62	0	0	66	199
1:30 PM	0	0	0	0	0	0	0	25	0	7	0	32	0	0	102	33	0	135	0	7	73	0	0	80	247
1:45 PM	0	0	0	0	0	0	0	26	0	3	0	29	0	0	89	16	0	105	0	6	71	0	0	77	211
Hourly Total	0	0	0	0	0	0	0	97	0	19	0	116	1	0	364	91	0	456	0	20	282	0	0	302	874
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	1	0	1	0	294	0	72	2	366	2	1	1018	313	4	1334	0	52	899	0	0	951	2652
Approach %	0.0	0.0	0.0	100.0	-	-	0.0	80.3	0.0	19.7	-	-	0.1	0.1	76.3	23.5	-	-	0.0	5.5	94.5	0.0	-	35.9	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	11.1	0.0	2.7	-	13.8	0.1	0.0	38.4	11.8	-	50.3	0.0	2.0	33.9	0.0	-	35.9	-
Lights	0	0	0	0	-	0	0	293	0	71	-	364	2	0	1014	310	-	1326	0	52	893	0	-	945	2635
% Lights	-	-	-	0.0	-	0.0	-	99.7	-	98.6	-	99.5	100.0	0.0	99.6	99.0	-	99.4	-	100.0	99.3	-	-	99.4	99.4
Other Vehicles	0	0	0	1	-	1	0	1	0	1	-	2	0	1	4	3	-	8	0	0	6	0	-	6	17
% Other Vehicles	-	-	-	100.0	-	100.0	-	0.3	-	1.4	-	0.5	0.0	100.0	0.4	1.0	-	0.6	-	0.0	0.7	-	-	0.6	0.6
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	4	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-



David E. Wooster and Associates : Main Account
2 East Crafton Ave.

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Count Name: Newbury Dr. & Plaza Access (SAT 11-2)
Site Code: 4392
Start Date: 09/23/2023
Page No: 3

Turning Movement Peak Hour Data (12:45 PM)

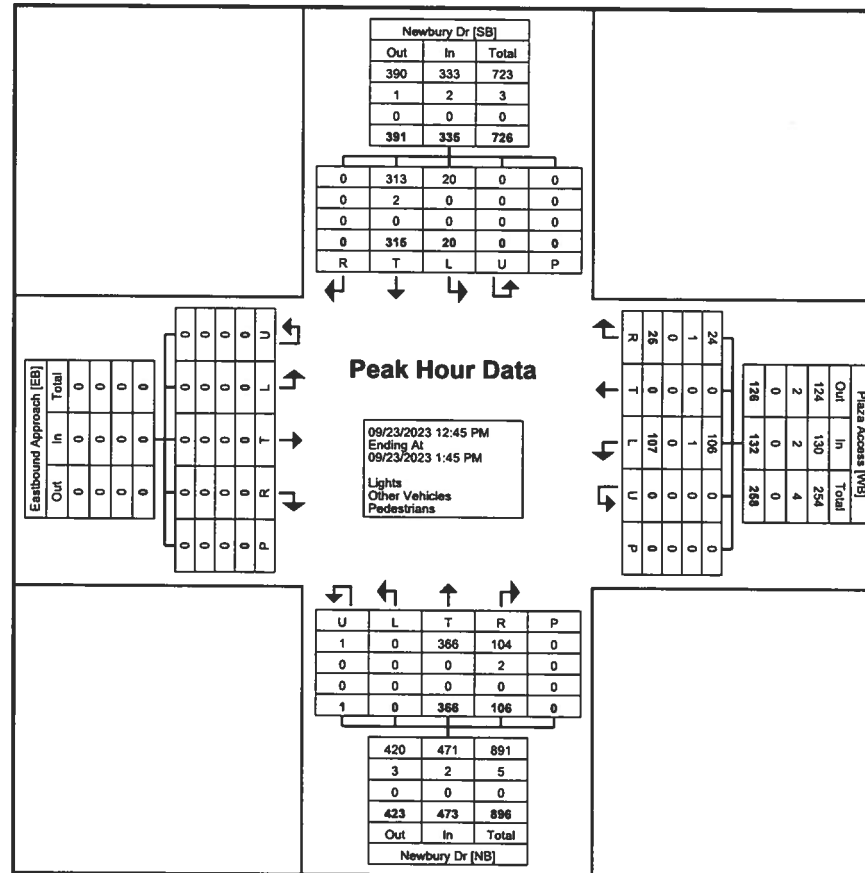
Start Time	Eastbound Approach Eastbound						Plaza Access Westbound						Newbury Dr Northbound						Newbury Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	0	0	0	0	0	0	36	0	9	0	45	0	0	91	31	0	122	0	6	104	0	0	110	277
1:00 PM	0	0	0	0	0	0	0	19	0	8	0	27	1	0	88	22	0	111	0	3	76	0	0	79	217
1:15 PM	0	0	0	0	0	0	0	27	0	1	0	28	0	0	85	20	0	105	0	4	62	0	0	66	199
1:30 PM	0	0	0	0	0	0	0	25	0	7	0	32	0	0	102	33	0	135	0	7	73	0	0	80	247
Total	0	0	0	0	0	0	0	107	0	25	0	132	1	0	366	106	0	473	0	20	315	0	0	335	940
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	81.1	0.0	18.9	-	-	0.2	0.0	77.4	22.4	-	-	0.0	6.0	94.0	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	11.4	0.0	2.7	-	14.0	0.1	0.0	38.9	11.3	-	50.3	0.0	2.1	33.5	0.0	-	35.6	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.743	0.000	0.694	-	0.733	0.250	0.000	0.897	0.803	-	0.876	0.000	0.714	0.757	0.000	-	0.761	0.848
Lights	0	0	0	0	-	0	0	106	0	24	-	130	1	0	366	104	-	471	0	20	313	0	-	333	934
% Lights	-	-	-	-	-	-	-	99.1	-	96.0	-	98.5	100.0	-	100.0	98.1	-	99.6	-	100.0	99.4	-	-	99.4	99.4
Other Vehicles	0	0	0	0	-	0	0	1	0	1	-	2	0	0	0	2	-	2	0	0	2	0	-	2	6
% Other Vehicles	-	-	-	-	-	-	-	0.9	-	4.0	-	1.5	0.0	-	0.0	1.9	-	0.4	-	0.0	0.6	-	-	0.6	0.6
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



David E. Wooster and Associates : Main Account
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Count Name: Newbury Dr. & Plaza Access (SAT
11-2)
Site Code: 4392
Start Date: 09/23/2023
Page No: 4



Turning Movement Peak Hour Data Plot (12:45 PM)

APPENDIX C

Photo Log of Existing Study Intersections



On Todd A .Miller Drive, looking north toward the intersection with Millers Run Road (SR 0050) / Newbury Drive



On Newbury Drive, looking south toward the intersection with Millers Run Road (SR 0050) / Todd A .Miller Drive





On Newbury Drive, looking north toward the intersection with the Plaza Access / Proposed Site Drive C



On Newbury Drive, looking south toward the intersection with the Plaza Access / Proposed Site Drive C



APPENDIX D

Traffic Signal Permit Plans

SIGNS

PLAN SYMBOL	SERIES DESIGNATION	SIZE W x H	DESCRIPTION	QTY.
A	R10-3EL	9"x15"	EDUC. PUSH BUTTON FOR WALK SIGNAL WITH COUNTDOWN TIMER SIGN	4
B	R10-3ER	9"x15"	EDUC. PUSH BUTTON FOR WALK SIGNAL WITH COUNTDOWN TIMER SIGN	4
C	R3-5L	30"x36"	LEFT TURN	5
D	R3-5S	30"x36"	STRAIGHT THRU	4
E	R3-5R	30"x36"	RIGHT TURN	2
F	R3-6SR	30"x36"	OPTIONAL RIGHT TURN	2
G	D3-4*	**	Millers Run Rd	2
H	D3-5*	**	Newbury Dr Municipal Dr	1
I	D3-5*	**	Municipal Dr Newbury Dr	1
J	R1-2	36"x36"	YIELD	2
K	R1-5L	18"x18"	YIELD HERE TO PEDESTRIANS	2
L	R10-10L	30"x36"	LEFT TURN SIGNAL	5
M	OM1-3	18"x18"	OBJECT MARKER	1
N	R4-7	24"x30"	KEEP RIGHT	1
O	R4-102	30"x36"	LEFT LANE NO TRUCKS	1

SIGNAL ASSEMBLY NOTES:

EQUIP VEHICLE SIGNALS WITH SCOOP TUNNEL VISORS.
 EQUIP ALL VEHICLE SIGNALS WITH METAL LOUVERED REFLECTIVE BACK PLATES.
 ALL SIGNALS L.E.D. MODULES.
 MIN/MAX HEIGHT FOR VEHICULAR SIGNALS OVER ROADWAY SHALL BE 17' / 18'.
 MIN/MAX HEIGHT FOR PEDESTRIAN SIGNALS SHALL BE 10' / 15'.
 ALL PEDESTRIAN SIGNALS SINGLE UNIT, HAND/MAN OVERLAY, EQUIPPED WITH L.E.D. LENSES.
 FINAL PLACEMENT OF SIGNALS DETERMINED BY REPRESENTATIVE OF TRAFFIC ENGINEERING UNIT.
 LASH SIGNAL CABLE TO SPAN. NO CABLE TIES PERMITTED.

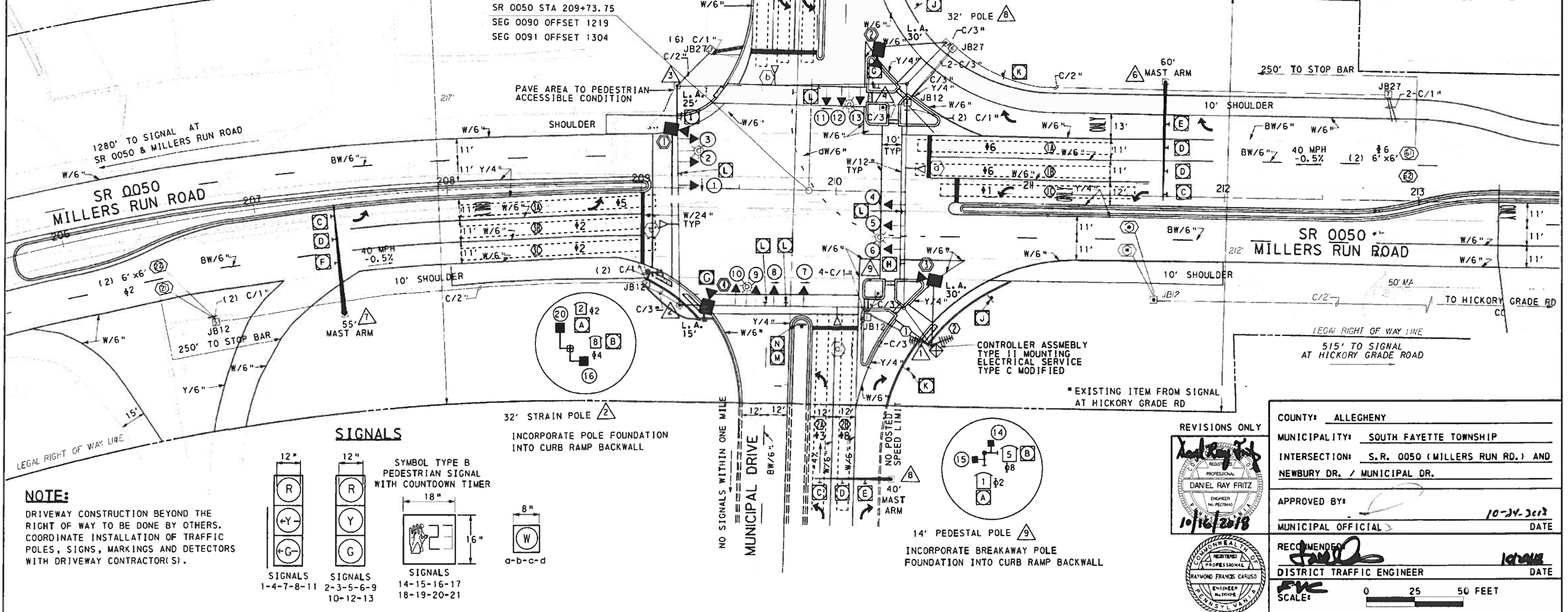
NOTES:

- SEE SHEET 2 OF 20 FOR:
 - DETECTOR LAYOUT DETAIL
 - ELECTRICAL SERVICE TYPE B MODIFIED DETAIL
 - ELECTRICAL SERVICE TYPE B MODIFIED DETAIL
 - TYPICAL SIGNAL SUPPORT GROUNDING DETAIL
 - GENERAL NOTES
- SEE SIGNING, PAVEMENT MARKING & DELINEATION PLANS FOR ADDITIONAL INFORMATION

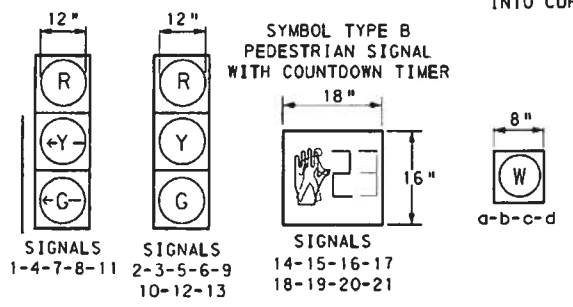
LEGEND

- DY/4" - DOUBLE YELLOW/WIDTH
- BW/6" - BROKEN WHITE LINE/WIDTH
- W/6" - SOLID WHITE LINE/WIDTH
- Y/4" - SOLID YELLOW LINE/WIDTH
- dw/6" - DASHED WHITE LINE/WIDTH
- 6" - MAST ARM
- 2" - STRAIN POLE
- 2" - PEDESTAL POLE
- C/2" - CONDUIT/SIZE
- ☒ - CONTROLLER ASSEMBLY
- ☐ - DETECTABLE WARNING SURFACE
- - WORK DONE BY OTHERS
- - RADIO COMMUNICATION ANTENNA
- ☐ - VEHICLE VIDEO DETECTOR
- ☐ - VIDEO DETECTION ZONE
- ⑭ - PEDESTRIAN SIGNAL HEAD
- ④ - VEHICULAR SIGNAL HEAD
- ⓐ - SIGN
- ④ - VEHICLE DETECTOR
- ④ - PEDESTRIAN PUSH BUTTON
- ④ - JUNCTION BOX
- ④ - PREEMPTION DETECTOR
- ☉ - CONFIRMATION LIGHT
- - EXISTING FENCE
- - EXISTING VEHICLE DETECTOR
- - EXISTING SIGN
- - EXISTING JUNCTION BOX
- - EXISTING SIGN ON JUNCTION BOX
- - LUMINAIRE ARM

*WHITE LEGEND ON GREEN BACKGROUND
 **THE CONTRACTOR IS RESPONSIBLE TO PROVIDE SHOP DRAWINGS, DETAILING THE REQUIRED SIGN SIZE, IN ACCORDANCE WITH PENNDOT PUBLICATION 111M, TC-8700 SERIES MOST RECENT VERSION, AND THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).



SIGNALS



NOTE:
 DRIVEWAY CONSTRUCTION BEYOND THE RIGHT OF WAY TO BE DONE BY OTHERS. COORDINATE INSTALLATION OF TRAFFIC POLES, SIGNS, MARKINGS AND DETECTORS WITH DRIVEWAY CONTRACTOR(S).

COUNTY: ALLEGHENY
 MUNICIPALITY: SOUTH FAYETTE TOWNSHIP
 INTERSECTION: S.R. 0050 (MILLERS RUN RD.) AND NEWBURY DR. / MUNICIPAL DR.
 APPROVED BY: *[Signature]* 10-24-2018 DATE
 MUNICIPAL OFFICIAL: _____
 RECOMMENDED BY: *[Signature]* 10/16/2018 DATE
 DISTRICT TRAFFIC ENGINEER: _____ DATE
 SCALE: 0 25 50 FEET

OPERATOR: cshaker
 FILE NAME: N:\35013-000\CAD\Traffic\2015 GLG 127 - South Fayette\Initial\Permit Plans\26-003-35013-TSP (SR 50 AT MUNICIPAL DR AND NEWBURY DR).dgn
 PLOTTED: 9/17/2018 2:53:11 PM
 REVISION: 10-04

OPERATOR: cbocker
 FILE NAME: N:\35013-000\CADD\Traffic\2015 GLG 127 - South Fayette\Inol Permitt Plans\26-004-35013-TSP (SR 50 AT MUNICIPAL DR AND NEWBURY DR).cadd.rpt
 REVISED (10-04)
 PLOTTED: 9/17/2018 2:53:30 PM

PHASING DIAGRAM

SIGNALS	PHASE 1+5				PHASE 1+6				PHASE 2+5				PHASE 2+6				PHASE 3+7				PHASE 3+8				PHASE 4+7				PHASE 4+8				PREEMPT 1+6				PREEMPT 2+5				PREEMPT 3+8				PREEMPT 4+7				EMERGENCY FLASHING
	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	
1	-G	-Y	R		-G	-Y	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		-G	-Y	R		-G	-Y	R		R	R	R		R	R	R		OFF				
2-3	R	R	R		G	G	Y	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		G	G	Y	R		R	R	R		R	R	R		Y						
4	-G	-Y	R		R	R	R		-G	-Y	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		OFF				
5-6	R	R	R		R	R	R		G	G	Y	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		Y							
7-8	R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		-G	-Y	R		R	R	R		R	R	R		R	R	R		R	R	R		OFF								
9-10	R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		Y								
11	R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		-G	-Y	R		R	R	R		R	R	R		R	R	R		R	R	R		OFF								
12-13	R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R	R	R		R								
14-19	DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		W	FD	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		OFF								
15-16	DW	DW	DW		DW	DW	DW		W	FD	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		OFF								
17-18	DW	DW	DW		W	FD	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		OFF								
20-21	DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		DW	DW	DW		OFF								
a	OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF								
b	OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF								
c	OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF								
d	OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF	OFF	OFF		OFF								
FIXED	X				X				X				X				X				X				X				X				X				X				X								
MIN. INITIAL	5				5				5				15				5				5				5																								
SEC. ACT.									2.1																																								
MAX. INITIAL									25																																								
PASSAGE	2				2				10				2				2				2				2																								
BEFORE RED.									15																																								
TO REDUCE.									10																																								
MIN. GAP									6																																								
MAX. I	25				25				10				60				20				20				20																								
MAX. II	25				25				10				60				20				20				20																								
PEDESTRIAN					P				P				7	23			P2				P2				7	23																							
MEMORY	L				L				L				MN				L				L				NL																								

MEMORY INCLUDES

- MAX I - ALL OTHER TIMES
- MAX II - 0600 TO 0900, MON THRU FRI
- MAX III - 1500 TO 1800, MON THRU FRI
- P UPON PEDESTRIAN ACTIVATION, OTHERWISE "DON'T WALK" AT ALL TIMES.
- P TIMING WILL BE AS SHOWN IN PHASE 2+6. IT MAY TIME OUT IN THIS PHASE OR BE COMPLETED IN PHASE 2+6
- P2 TIMING WILL BE AS SHOWN IN PHASE 4+8 IT MAY TIME OUT IN THIS PHASE OR BE COMPLETED IN PHASE 4+8
- X DURATION OF EMERGENCY VEHICLE ACTUATION OR MAXIMUM OF 60 SECONDS.

COORDINATION NOTES

CONTROLLERS INTERCONNECTED USING WIRELESS RADIO COMMUNICATION SYSTEM AND ARE PART OF AN ADAPTIVE SIGNAL SYSTEM AT THE FOLLOWING INTERSECTIONS:
 S.R. 0050 AT S.R. 3026 (MILLERS RUN RD)
 S.R. 0050 AT MUNICIPAL DR & NEWBURY DR
 S.R. 0050 AT HICKORY GRADE RD
 S.R. 0050 AT I-79 SB RAMP
 S.R. 0050 AT I-79 NB RAMP
 S.R. 0050 AT S.R. 3003 (WASHINGTON PIKE)
 S.R. 0050 AT S.R. 3034 (CHARTIERS ST) & CHURCH ST
 S.R. 3003 (WASHINGTON PIKE) AT DANIELL DR

EMERGENCY VEHICLE PREEMPTION NOTES

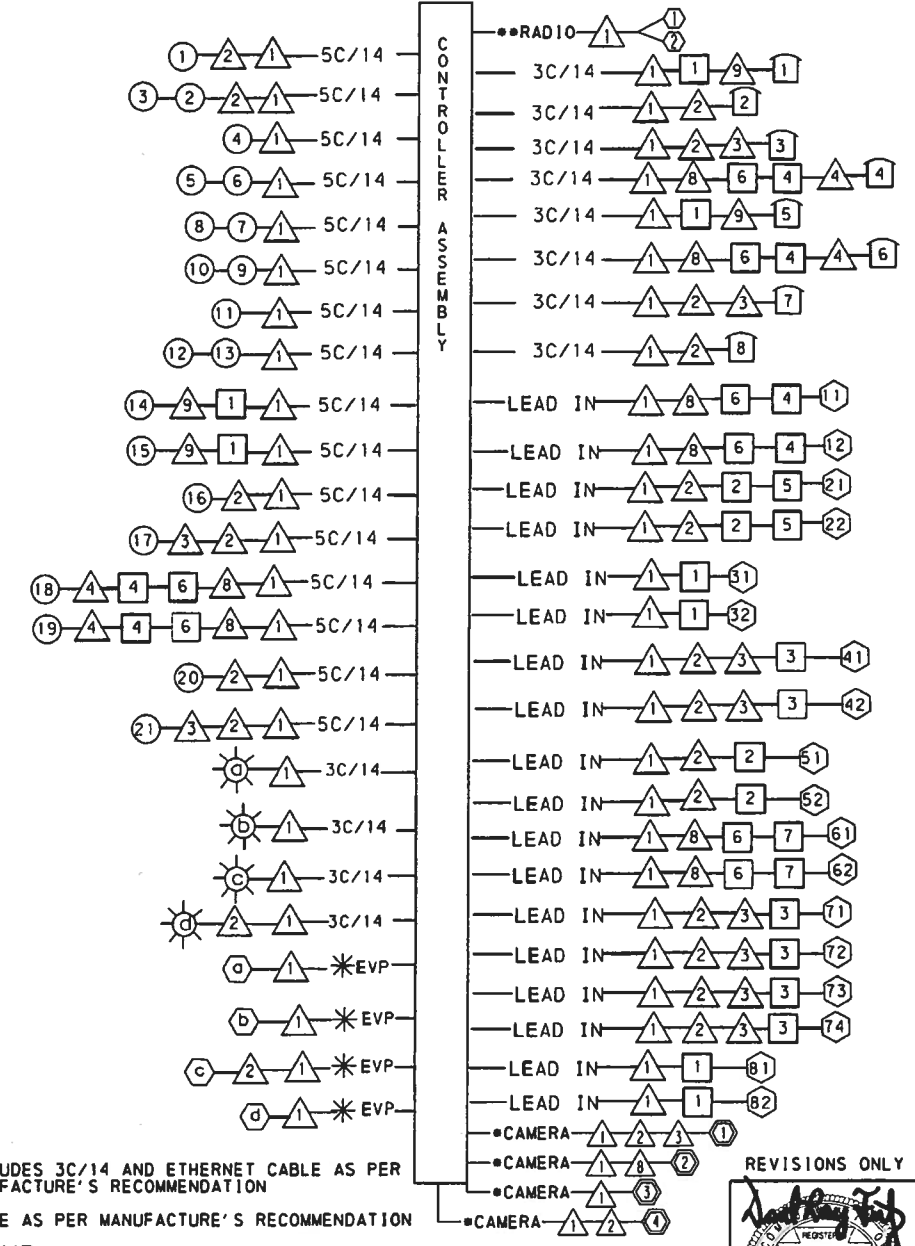
- EMERGENCY PREEMPTION MAY OCCUR DURING ANY INTERVAL OF THE NORMAL CONTROLLER OPERATION. DEPENDING ON THE DIRECTION OF TRAVEL OF THE EMERGENCY VEHICLE, ONE OF THE FOLLOWING SHALL BE DISPLAYED: EMERGENCY PREEMPT PHASE 1+6, 2+5, 3+8, OR 4+7. THE SYSTEM SHALL PROVIDE SERVICE ON A FIRST-COME-FIRST-SERVED BASIS. ONCE THE FIRST PRIORITY VEHICLE CALLS THE SYSTEM, OTHER PREEMPTIVE VEHICLES SHALL BE PREVENTED FROM ENTERING CALLS UNTIL THE FIRST EMERGENCY VEHICLE RELEASES CONTROL AND CLEARS THE INTERSECTION.
- UPON ACTIVATION OF AN EMERGENCY VEHICLE:
- IF THE CONTROLLER OPERATION IS IN INTERVAL 1 OF A NON-PREEMPTION PHASE, THE CONTROLLER SHALL TERMINATE THE INTERVAL IMMEDIATELY AND PROCEED NORMALLY THROUGH THE YELLOW AND ALL RED INTERVALS PROCEEDING TO THE PREEMPTION PHASE.
 - IF THE CONTROLLER OPERATION IS IN INTERVAL 1 OF A PREEMPTION PHASE, THE CONTROLLER SHALL REMAIN IN THAT INTERVAL.
 - IF THE CONTROLLER OPERATION IS IN THE YELLOW OR ALL RED INTERVAL OF ANY PHASE, THE CONTROLLER SHALL TIME OUT THOSE INTERVALS NORMALLY BEFORE PROCEEDING TO THE PREEMPTION PHASE.
 - PROVIDE A FAIL SAFE INDICATION CONSISTING OF A FLASHING WHITE LIGHT FOR THE DIRECTION ON WHICH THE EMERGENCY VEHICLE IS APPROACHING. WHEN A CALL IS RECEIVING, THE FAIL SAFE INDICATION SHALL BE ACTIVATED. FLASH AT A RATE NOT LESS THAN 50 NOT MORE THAN 60 TIMES PER MINUTE.
 - UPON TERMINATION OF THE PREEMPTION PHASES, THE CONTROLLER SHALL PROCEED NORMALLY THROUGH THE YELLOW AND ALL RED INTERVALS TO NORMAL "PHASE NEXT" OPERATION.
 - ANY WALK INDICATION SHALL TERMINATE IMMEDIATELY FOLLOWED BY A FLASHING DON'T WALK INDICATION FOR THE NORMAL PEDESTRIAN CLEARANCE INTERVAL BEFORE PROCEEDING TO THE EMERGENCY PREEMPTION ROUTINE IN NOTES 1 & 2.
 - DISPLAY ANY FLASHING DON'T WALK INDICATION FOR NORMAL PEDESTRIAN CLEARANCE INTERVAL BEFORE PROCEEDING TO THE EMERGENCY PREEMPTION ROUTINE IN NOTES 1 & 2.
 - THE PREEMPTION PHASE GREEN INTERVAL SHALL BE 10 SECONDS AND THEN EXTEND FOR THE LENGTH OF THE PREEMPTION ACTUATION OR A MAXIMUM OF 60 SECONDS.
 - IF THE PREEMPTION OCCURS DURING CONFLICT/TIME CLOCK FLASH THE TRAFFIC SIGNAL SHALL CONTINUE FLASHING.
 - PREEMPT TO COORDINATION: USED WHEN EMERGENCY PREEMPTION IS ACTIVATED DURING COORDINATION OPERATION TO ALLOW THE NEXT PERMISSIVE PHASE IN THE COORDINATION CYCLE TO BE SERVICED FOLLOWING PREEMPTION.

PHASING NOTES

- 1 -G- IF PHASE 1+6 FOLLOWS
- 2 -G- IF PHASE 1+5 FOLLOWS
- 3 -G- IF PHASE 2+5 FOLLOWS
- 4 G IF PHASE 2+6 FOLLOWS
- 5 G IF PHASE 1+6 FOLLOWS
- 6 G IF PHASE 2+5 FOLLOWS
- 7 -G- IF PHASE 4+7 FOLLOWS
- 8 -G- IF PHASE 3+7 FOLLOWS
- 9 -G- IF PHASE 3+8 FOLLOWS
- 10 G IF PHASE 4+8 FOLLOWS
- 11 PASSAGE TIME EQUALS THE TIME THE EMERGENCY VEHICLE ACTUATION IS IN CONTROL OF THE INTERSECTION.
- 12 DURATION OF EMERGENCY VEHICLE ACTUATION OR MAXIMUM OF 60 SECONDS.

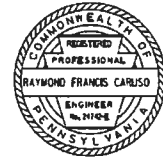
WIRING DIAGRAM

DISPLAYS DETECTORS



LEGEND

- 5C/14 - CABLE. (NO. OF CONDUCTORS/SIZE AWG.)
- △ SIGNAL SUPPORT
- SIGNAL HEAD
- ⬡ DETECTOR
- ☀ CONFIRMATION LIGHT
- ⊙ PREEMPTION DETECTOR
- JUNCTION BOX
- ▣ PEDESTRIAN PUSH BUTTON
- ⬠ VIDEO DETECTOR
- ⊙ RADIO COMMUNICATION ANTENNA



COUNTY: ALLEGHENY
 MUNICIPALITY: SOUTH FAYETTE TOWNSHIP
 INTERSECTION: S.R. 0050 (MILLERS RUN RD.) AND NEWBURY DR. / MUNICIPAL DR.

APPROVED BY: *[Signature]* 10-24-2018
 MUNICIPAL OFFICIAL: _____ DATE: _____

RECOMMENDED BY: *[Signature]*
 DISTRICT TRAFFIC ENGINEER: _____ DATE: _____

APPENDIX E

Level of Service (LOS) Criteria Summary

LEVEL-OF-SERVICE CRITERIA SIGNALIZED INTERSECTIONS

Level-of-Service for signalized intersections is defined in terms of delay. Delay is a measure of driver discomfort, frustration, fuel consumption, and lost travel time. Specifically, Level-of-Service criteria are stated in terms of the average stopped delay per vehicle for a 15-minute analysis period.

Level-of-Service A describes operations with very low delay, i.e., less than 10.0 seconds per vehicle.

Level-of-Service B describes operations with delay in the range of 10.1 to 20.0 seconds per vehicle.

Level-of-Service C describes operations with delay in the range of 20.1 to 35.0 seconds per vehicle.

Level-of-Service D describes operations with delay in the range of 35.1 to 55.0 seconds per vehicle.

Level-of-Service E describes operations with delay in the range of 55.1 to 80.0 seconds per vehicle. This is considered to be the limit of acceptable delay.

Level-of-Service F describes operations with delay in excess of 80.0 seconds per vehicle. This is considered to be unacceptable to most drivers.

UNSIGNALIZED INTERSECTIONS

AVERAGE TOTAL DELAY (sec/veh)	LEVEL OF SERVICE	EXPECTED DELAY TO MINOR STREET TRAFFIC
≤ 10	A	Little or no delay
>10 and ≤15	B	Short traffic delays
>15 and ≤25	C	Average traffic delays
>25 and ≤35	D	Long traffic delays
>35 and ≤50	E	Very long delays
>50	F	*

* -- When demand volume exceeds the capacity of the lane, extreme delays will be encountered with queuing which may cause severe congestion affecting other traffic movements in the intersection. This condition usually warrants improvements to the intersection.

APPENDIX F

Synchro Printouts – *Existing Year 2023 Condition*

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	27	653	19	61	388	221	3	7	27	198	22	20
Future Volume (vph)	27	653	19	61	388	221	3	7	27	198	22	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frnt		0.996				0.850			0.850		0.928	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3331	0	1778	3278	1613	1841	1700	1540	3416	1656	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3331	0	1778	3278	1613	1841	1700	1540	3416	1656	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2				243			117		22	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	30	718	21	67	426	243	3	8	30	218	24	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	30	739	0	67	426	243	3	8	30	218	46	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	6.8	24.8		8.0	31.2	31.2	5.8	5.9	5.9	9.9	12.8	
Actuated g/C Ratio	0.11	0.38		0.12	0.48	0.48	0.09	0.09	0.09	0.15	0.20	
v/c Ratio	0.18	0.58		0.30	0.27	0.27	0.02	0.05	0.12	0.42	0.13	
Control Delay	37.7	19.8		36.6	12.9	3.3	38.3	38.1	1.0	32.2	18.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	37.7	19.8		36.6	12.9	3.3	38.3	38.1	1.0	32.2	18.6	
LOS	D	B		D	B	A	D	D	A	C	B	
Approach Delay		20.5			11.9			11.0			29.8	
Approach LOS		C			B			B			C	
Queue Length 50th (ft)	13	139		28	47	0	1	3	0	46	8	
Queue Length 95th (ft)	44	226		76	118	42	11	19	0	94	42	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	554	2678		601	2635	1344	458	877	851	851	865	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.05	0.28		0.11	0.16	0.18	0.01	0.01	0.04	0.26	0.05	

Intersection Summary























Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 64.6
 Natural Cycle: 55
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.58
 Intersection Signal Delay: 18.1
 Intersection Capacity Utilization 50.1%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s












1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 PM Peak Hour Condition

10/11/2023

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	44	518	22	42	686	351	11	17	87	299	35	64
Future Volume (vph)	44	518	22	42	686	351	11	17	87	299	35	64
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850			0.850		0.903	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3420	0	1778	3405	1644	1841	1938	1599	3416	1657	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3420	0	1778	3405	1644	1841	1938	1599	3416	1657	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4				362			117		59	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	45	534	23	43	707	362	11	18	90	308	36	66
Shared Lane Traffic (%)												
Lane Group Flow (vph)	45	557	0	43	707	362	11	18	90	308	102	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 PM Peak Hour Condition

10/11/2023









												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.5	31.1		7.4	31.0	31.0	6.1	6.3	6.3	13.7	19.2	
Actuated g/C Ratio	0.10	0.42		0.10	0.42	0.42	0.08	0.08	0.08	0.18	0.26	
v/c Ratio	0.25	0.39		0.24	0.50	0.40	0.07	0.11	0.37	0.49	0.22	
Control Delay	44.2	16.5		44.2	18.0	3.2	45.3	45.1	9.6	36.6	17.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	44.2	16.5		44.2	18.0	3.2	45.3	45.1	9.6	36.6	17.8	
LOS	D	B		D	B	A	D	D	A	D	B	
Approach Delay		18.6			14.2			18.3			31.9	
Approach LOS		B			B			B			C	
Queue Length 50th (ft)	22	105		21	143	0	6	9	0	76	16	
Queue Length 95th (ft)	66	156		64	206	47	26	35	31	154	78	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	524	2559		531	2547	1321	405	885	793	753	788	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.09	0.22		0.08	0.28	0.27	0.03	0.02	0.11	0.41	0.13	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 74.4
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.50
 Intersection Signal Delay: 18.8
 Intersection Capacity Utilization 53.3%
 Analysis Period (min) 15

Intersection LOS: B
ICU Level of Service A

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

 O1	 O2	 O3	 O4
25 s	60 s	20 s	35 s
 O5	 O6	 O7	 O8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	44	531	21	27	504	405	5	8	33	338	20	68
Future Volume (vph)	44	531	21	27	504	405	5	8	33	338	20	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850			0.850		0.884	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3478	0	1814	3438	1677	1841	1938	1647	3450	1671	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3478	0	1814	3438	1677	1841	1938	1647	3450	1671	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3				440			117		74	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	48	577	23	29	548	440	5	9	36	367	22	74
Shared Lane Traffic (%)												
Lane Group Flow (vph)	48	600	0	29	548	440	5	9	36	367	96	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.2	24.4		6.4	21.5	21.5	5.7	5.7	5.7	15.6	17.0	
Actuated g/C Ratio	0.11	0.39		0.10	0.34	0.34	0.09	0.09	0.09	0.25	0.27	
v/c Ratio	0.24	0.45		0.16	0.47	0.51	0.03	0.05	0.14	0.43	0.19	
Control Delay	36.1	16.5		36.8	19.3	4.3	37.8	37.5	1.2	27.8	11.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	36.1	16.5		36.8	19.3	4.3	37.8	37.5	1.2	27.8	11.6	
LOS	D	B		D	B	A	D	D	A	C	B	
Approach Delay		17.9			13.3			11.4			24.4	
Approach LOS		B			B			B			C	
Queue Length 50th (ft)	18	81		11	104	0	2	3	0	68	5	
Queue Length 95th (ft)	63	163		45	155	53	14	21	0	#162	57	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	588	2878		608	2844	1463	455	992	901	853	892	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.21		0.05	0.19	0.30	0.01	0.01	0.04	0.43	0.11	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 63.1
 Natural Cycle: 55
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.51
 Intersection Signal Delay: 17.0
 Intersection Capacity Utilization 50.8%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
























25 s	60 s	20 s	35 s
25 s	60 s	20 s	35 s

APPENDIX G

HCM Printouts – *Existing Year 2023 Condition*

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 AM Peak Hour Condition

10/05/2023

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	27	653	19	61	388	221	3	7	27	198	22	20
Future Volume (veh/h)	27	653	19	61	388	221	3	7	27	198	22	20
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	30	718	21	67	426	0	3	8	0	218	24	7
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	60	1115	33	113	1202		8	19		353	148	43
Arrive On Green	0.03	0.32	0.32	0.06	0.34	0.00	0.00	0.01	0.00	0.10	0.11	0.11
Sat Flow, veh/h	1747	3514	103	1818	3485	1656	1959	1847	1654	3445	1354	395
Grp Volume(v), veh/h	30	362	377	67	426	0	3	8	0	218	0	31
Grp Sat Flow(s),veh/h/ln	1747	1771	1846	1818	1743	1656	1959	1847	1654	1722	0	1749
Q Serve(g_s), s	0.8	8.3	8.3	1.7	4.3	0.0	0.1	0.2	0.0	2.9	0.0	0.8
Cycle Q Clear(g_c), s	0.8	8.3	8.3	1.7	4.3	0.0	0.1	0.2	0.0	2.9	0.0	0.8
Prop In Lane	1.00		0.06	1.00		1.00	1.00		1.00	1.00		0.23
Lane Grp Cap(c), veh/h	60	562	586	113	1202		8	19		353	0	191
V/C Ratio(X)	0.50	0.64	0.64	0.60	0.35		0.37	0.41		0.62	0.00	0.16
Avail Cap(c_a), veh/h	702	2023	2108	731	3981		580	1133		1020	0	1073
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.4	13.8	13.8	21.6	11.6	0.0	23.5	23.2	0.0	20.3	0.0	19.1
Incr Delay (d2), s/veh	2.4	0.6	0.5	1.9	0.1	0.0	10.4	5.1	0.0	0.7	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.6	2.8	0.7	1.3	0.0	0.1	0.1	0.0	1.1	0.0	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.8	14.4	14.4	23.5	11.6	0.0	33.9	28.3	0.0	21.0	0.0	19.2
LnGrp LOS	C	B	B	C	B		C	C		C	A	B
Approach Vol, veh/h		769			493			11			249	
Approach Delay, s/veh		14.8			13.2			29.8			20.8	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.9	21.0	6.2	11.2	7.6	22.3	10.8	6.5				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+I1), s	3.7	10.3	2.1	2.8	2.8	6.3	4.9	2.2				
Green Ext Time (p_c), s	0.1	3.0	0.0	0.1	0.0	1.9	0.3	0.0				

Intersection Summary

HCM 6th Ctrl Delay	15.4
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 PM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	44	518	22	42	686	351	11	17	87	299	35	64
Future Volume (veh/h)	44	518	22	42	686	351	11	17	87	299	35	64
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	45	534	20	43	707	0	11	18	0	308	36	33
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	86	1101	41	82	1106		28	46		456	125	114
Arrive On Green	0.05	0.31	0.31	0.05	0.31	0.00	0.01	0.02	0.00	0.13	0.14	0.14
Sat Flow, veh/h	1847	3566	133	1818	3599	1683	1959	2057	1705	3445	889	814
Grp Volume(v), veh/h	45	271	283	43	707	0	11	18	0	308	0	69
Grp Sat Flow(s),veh/h/ln	1847	1814	1885	1818	1800	1683	1959	2057	1705	1722	0	1703
Q Serve(g_s), s	1.2	5.9	6.0	1.1	8.3	0.0	0.3	0.4	0.0	4.2	0.0	1.8
Cycle Q Clear(g_c), s	1.2	5.9	6.0	1.1	8.3	0.0	0.3	0.4	0.0	4.2	0.0	1.8
Prop In Lane	1.00		0.07	1.00		1.00	1.00		1.00	1.00		0.48
Lane Grp Cap(c), veh/h	86	560	582	82	1106		28	46		456	0	239
V/C Ratio(X)	0.52	0.48	0.49	0.52	0.64		0.40	0.39		0.68	0.00	0.29
Avail Cap(c_a), veh/h	719	2006	2085	708	3981		562	1222		988	0	1012
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.7	13.7	13.7	22.8	14.6	0.0	23.9	23.5	0.0	20.2	0.0	18.8
Incr Delay (d2), s/veh	1.8	0.3	0.3	1.9	0.3	0.0	3.4	2.0	0.0	0.7	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	2.0	2.0	0.5	2.7	0.0	0.1	0.2	0.0	1.6	0.0	0.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.5	14.0	14.0	24.7	14.9	0.0	27.2	25.6	0.0	20.8	0.0	19.1
LnGrp LOS	C	B	B	C	B		C	C		C	A	B
Approach Vol, veh/h		599			750			29			377	
Approach Delay, s/veh		14.8			15.4			26.2			20.5	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.2	21.1	6.7	12.8	8.3	21.0	12.5	7.1				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+1), s	3.1	8.0	2.3	3.8	3.2	10.3	6.2	2.4				
Green Ext Time (p_c), s	0.0	2.1	0.0	0.2	0.0	3.4	0.4	0.0				

Intersection Summary























HCM 6th Ctrl Delay	16.5
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
Existing Year 2023 SAT Peak Hour Condition

10/05/2023

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	44	531	21	27	504	405	5	8	33	338	20	68
Future Volume (veh/h)	44	531	21	27	504	405	5	8	33	338	20	68
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	48	577	19	29	548	0	5	9	0	367	22	36
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	90	1163	38	61	1102		13	24		519	100	163
Arrive On Green	0.05	0.32	0.32	0.03	0.30	0.00	0.01	0.01	0.00	0.15	0.15	0.15
Sat Flow, veh/h	1847	3640	120	1847	3628	1709	1959	2057	1743	3472	646	1058
Grp Volume(v), veh/h	48	292	304	29	548	0	5	9	0	367	0	58
Grp Sat Flow(s),veh/h/ln	1847	1842	1918	1847	1814	1709	1959	2057	1743	1736	0	1704
Q Serve(g_s), s	1.3	6.3	6.3	0.8	6.1	0.0	0.1	0.2	0.0	5.0	0.0	1.5
Cycle Q Clear(g_c), s	1.3	6.3	6.3	0.8	6.1	0.0	0.1	0.2	0.0	5.0	0.0	1.5
Prop In Lane	1.00		0.06	1.00		1.00	1.00		1.00	1.00		0.62
Lane Grp Cap(c), veh/h	90	588	613	61	1102		13	24		519	0	263
V/C Ratio(X)	0.53	0.50	0.50	0.47	0.50		0.38	0.37		0.71	0.00	0.22
Avail Cap(c_a), veh/h	711	2015	2097	711	3968		556	1208		985	0	1001
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.9	13.6	13.6	23.4	14.1	0.0	24.4	24.2	0.0	20.0	0.0	18.3
Incr Delay (d2), s/veh	1.8	0.3	0.3	2.1	0.2	0.0	6.6	3.5	0.0	0.7	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	2.1	2.2	0.3	2.0	0.0	0.1	0.1	0.0	1.9	0.0	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.7	13.9	13.9	25.5	14.3	0.0	31.0	27.7	0.0	20.6	0.0	18.4
LnGrp LOS	C	B	B	C	B		C	C		C	A	B
Approach Vol, veh/h		644			577			14			425	
Approach Delay, s/veh		14.7			14.8			28.9			20.3	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.6	21.8	6.3	13.6	8.4	21.0	13.4	6.6				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	2.8	8.3	2.1	3.5	3.3	8.1	7.0	2.2				
Green Ext Time (p_c), s	0.0	2.3	0.0	0.2	0.0	2.5	0.5	0.0				

Intersection Summary

HCM 6th Ctrl Delay	16.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

5: Newbury Drive & Plaza Access
Existing Year 2023 AM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↓			↑↑
Traffic Vol, veh/h	35	11	196	59	17	197
Future Vol, veh/h	35	11	196	59	17	197
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	0	5	0	0	2
Mvmt Flow	41	13	231	69	20	232

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	422	150	0	0	300
Stage 1	266	-	-	-	-
Stage 2	156	-	-	-	-
Critical Hdwy	6.86	6.9	-	-	4.1
Critical Hdwy Stg 1	5.86	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-
Follow-up Hdwy	3.53	3.3	-	-	2.2
Pot Cap-1 Maneuver	557	876	-	-	1273
Stage 1	751	-	-	-	-
Stage 2	853	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	547	876	-	-	1273
Mov Cap-2 Maneuver	547	-	-	-	-
Stage 1	751	-	-	-	-
Stage 2	838	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.6	0	0.7
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	601	1273
HCM Lane V/C Ratio	-	-	0.09	0.016
HCM Control Delay (s)	-	-	11.6	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.3	0

5: Newbury Drive & Plaza Access
Existing Year 2023 PM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	1.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↓			↑↑
Traffic Vol, veh/h	57	28	331	62	13	334
Future Vol, veh/h	57	28	331	62	13	334
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	62	30	360	67	14	363

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	604	214	0	0	427
Stage 1	394	-	-	-	-
Stage 2	210	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1
Critical Hdwy Stg 1	5.8	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	435	797	-	-	1143
Stage 1	656	-	-	-	-
Stage 2	811	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	428	797	-	-	1143
Mov Cap-2 Maneuver	428	-	-	-	-
Stage 1	656	-	-	-	-
Stage 2	799	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.7	0	0.4
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	505	1143
HCM Lane V/C Ratio	-	-	0.183	0.012
HCM Control Delay (s)	-	-	13.7	8.2
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.7	0

5: Newbury Drive & Plaza Access
Existing Year 2023 SAT Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	3.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↓			↔↑
Traffic Vol, veh/h	107	25	366	106	20	315
Future Vol, veh/h	107	25	366	106	20	315
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	1	4	0	2	0	1
Mvmt Flow	126	29	431	125	24	371

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	728	278	0	0	556
Stage 1	494	-	-	-	-
Stage 2	234	-	-	-	-
Critical Hdwy	6.82	6.98	-	-	4.1
Critical Hdwy Stg 1	5.82	-	-	-	-
Critical Hdwy Stg 2	5.82	-	-	-	-
Follow-up Hdwy	3.51	3.34	-	-	2.2
Pot Cap-1 Maneuver	361	713	-	-	1025
Stage 1	582	-	-	-	-
Stage 2	786	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	351	713	-	-	1025
Mov Cap-2 Maneuver	351	-	-	-	-
Stage 1	582	-	-	-	-
Stage 2	763	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	20.3	0	0.6
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	388	1025
HCM Lane V/C Ratio	-	-	0.4	0.023
HCM Control Delay (s)	-	-	20.3	8.6
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	1.9	0.1

APPENDIX H

Source Data for Background Developments

From: Michael J. Haberman, P.E. <mhaberman@gatewayengineers.com>

Sent: Tuesday, October 3, 2023 2:54 PM

To: Josh Haydo <jhaydo@dewooster.com>

Cc: Mark Szewcow <mark.szewcow@gibson-thomas.com>; Paone, Talia <tpaone@pa.gov>; John M. Barrett <JBarrett@sftwp.com>; Eileen Botti <eileen.botti@gibson-thomas.com>; Simmons, Michael (PENNDOT) <MICSIMMONS@pa.gov>; Miller, Ed M. <edmille@pa.gov>; Siewe, Emmanuel <esiewe@pa.gov>; Fedio, Daniel <dafedio@pa.gov>; Jesse Nelson <nelson@dewooster.com>; Suleiman Swai <sswai@dewooster.com>; Joseph M. Galbraith, P.E. <jgalbraith@gatewayengineers.com>

Subject: RE: [External] #4392 - Retail Development - Former Pro Bike + Run - TIS Scoping

Josh,

A formal study was not completed for the Raising Cane's. The developer's consultant submitted a trip generation addendum for the purposes of developing an updated amount for traffic impact fees. I've attached the original TIS for The Piazza development, the trip generation addendum for the Raising Cane's, and the amended site plan for The Piazza with the removal of the drive-in bank and inclusion of the Raising Cane's (note that the final size of the Raising Cane's is 4,250 s.f., not what is show on the plan). Note the following as it relates to The Piazza development:

- To date, the following uses within The Piazza are built/occupied:
 - 11,700 s.f. of high-turnover sit-down restaurants (building #1 and Building #4 on site plan)
 - 21,200 s.f. of shopping center (building #3 and part of building #5 on site plan)
 - 3,000 s.f. fast-food restaurant without drive-through (within building #5 on site plan)
- The following uses within The Piazza are not yet constructed/occupied:
 - 6,800 s.f. of high-turnover sit-down restaurant (building #2 on site plan)
 - 4,000 s.f. fast-food restaurant with drive-through (building #6 on site plan)
 - 4,250 s.f. fast-food restaurant with drive-through (Raising Cane's)

You'll need to use the above summary and attached reports to establish the additional approved background trips for your study associated with The Piazza.

As for the South Fayette Commons development (Dunkin Donuts and Washington Federal), I've attached the original approved TIS for that development (prepared by Wooster) and the trip generation for the Taco Bell (prepared by Wooster). To date, the only uses that have been constructed are the Dunkin Donuts and the Washington Federal Bank. The Taco Bell is approved and under construction. To simplify the trip generation and background trips associated with South Fayette Commons, I would recommend taking the difference between the total trips in the approved TIS and the estimated trips associated with the Dunkin Donuts and Washington Federal Bank and adding those to the study area. That way the trips associated with the Taco Bell and the remainder of the development will be included in the background for this study. If PennDOT and/or Gibson Thomas feel differently about that approach, I will defer to them.

There is no study for the Cigar Bar that I am aware of, so you'll have to estimate the trips associated with that land use as well.

Let me know if you want to discuss any of this in more detail or if you have any questions.

Thanks,
Mike

From: Mark Szewcow

Sent: Friday, September 15, 2023 11:31 AM

To: Paone, Talia <tpaone@pa.gov>; John M. Barrett <JBarrett@sftwp.com>; Eileen Botti <eileen.botti@gibson-thomas.com>; Josh Haydo <haydo@dewooster.com>; Simmons, Michael (PENNDOT) <MICSIMMONS@pa.gov>; Miller, Ed M. <edmille@pa.gov>; Siewe, Emmanuel <esiewe@pa.gov>; Fedio, Daniel <dafedio@pa.gov>

Subject: RE: [External] #4392 - Retail Development - Former Pro Bike + Run - TIS Scoping

Hello all,

I wanted to follow up on some issues that were discussed at the meeting yesterday. After talking with John Barrett, here is a summary of the responses from the Township.

- 1.) The Township agrees to only add traffic from developments that are currently approved but not yet constructed in the Newberry Site. There is currently a 9,377 SF Cigar Bar and Restaurant approved to be constructed near Top Golf.
- 2.) The Township is agreeable to the intersections and time periods to be studied.

If anybody has any questions concerning this matter, please do not hesitate to call or email me.

Thanks

Mark Szewcow, PE

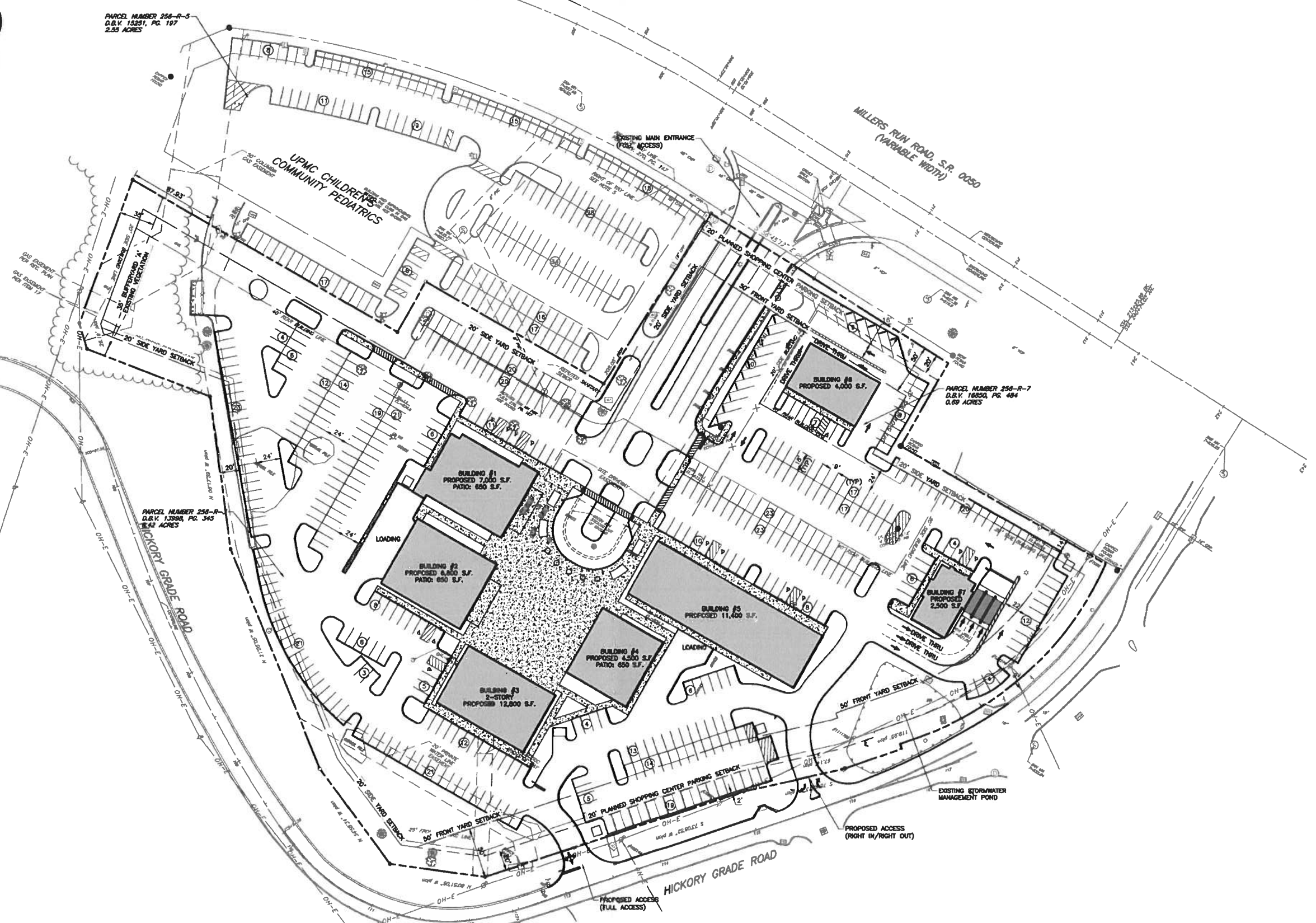
Phone: 724-539-8562

Cell: 412-417-7712

mark.szewcow@gibson-thomas.com



PARCEL NUMBER 256-R-5
O.B.V. 13281, PG. 197
2.35 ACRES



PARCEL NUMBER 256-R-6
O.B.V. 13988, PG. 345
3.42 ACRES

PARCEL NUMBER 256-R-7
O.B.V. 16250, PG. 484
0.69 ACRES

P:\2019\191-730\191-730\191-730-TR02-Figures-11x17.dwg[SITE PLAN] LS:(5/5/2020 - qberkey) - LP: 5/5/2020 12:27 PM

CEC
Civil & Environmental Consultants, Inc.
 333 Baldwin Road · Pittsburgh, PA 15205
 412-429-2324 · 800-365-2324
 www.cecinc.com

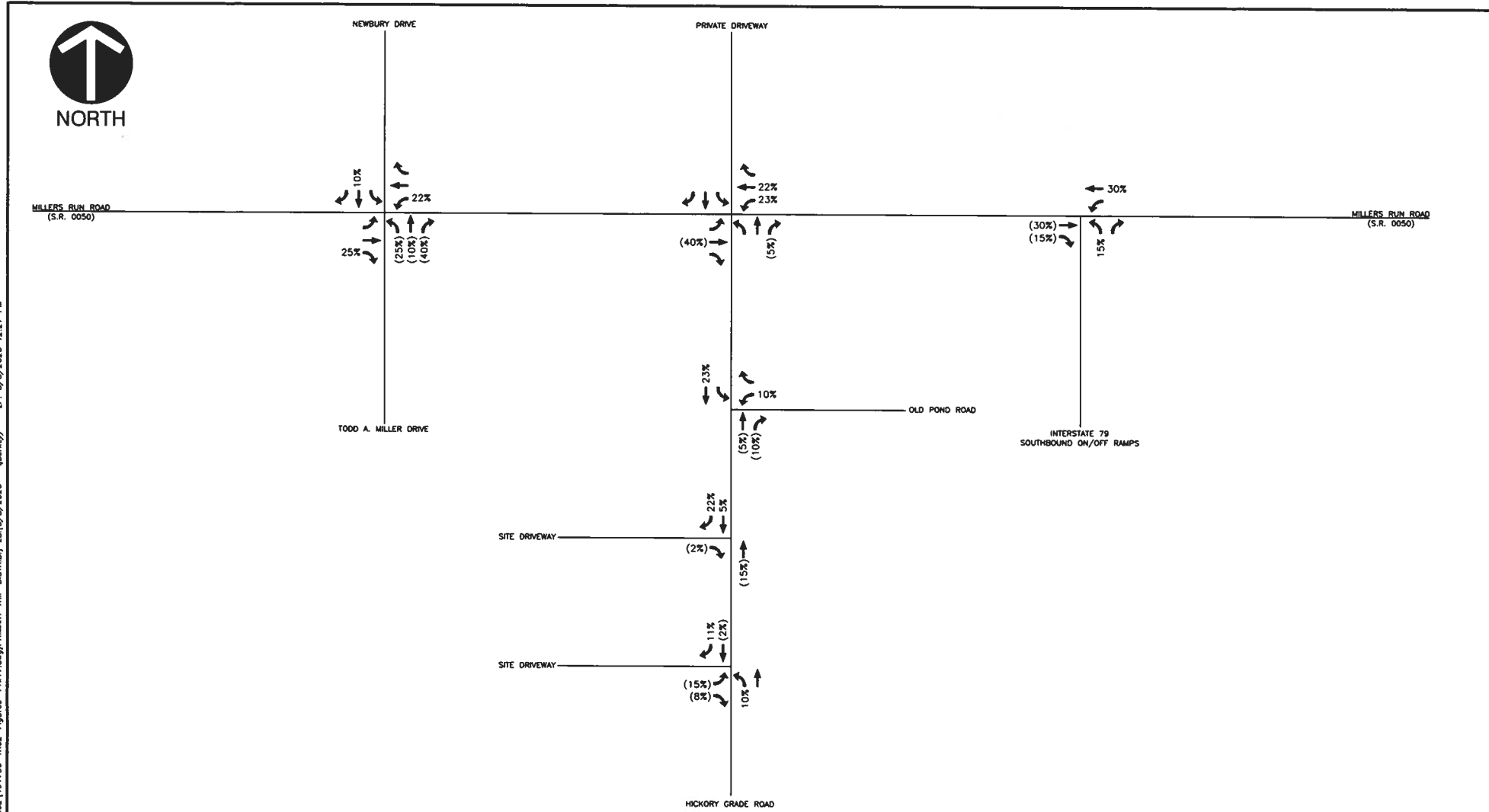
THE PIAZZA RETAIL DEVELOPMENT
TRAFFIC IMPACT STUDY
SOUTH FAYETTE TOWNSHIP,
ALLEGHENY COUNTY, PENNSYLVANIA

SITE PLAN

DRAWN BY:	QAB	CHECKED BY:	JRT	APPROVED BY:	JMD	FIGURE NO.:	2
DATE:	APRIL 2020	DWG SCALE:	NOT TO SCALE	PROJECT NO.:	191-730		



P:\2019\191-7301-0000\0mg\1702\191730-PROJ-FIGURES-11x17.dwg\PRIMARY TRIP DISTRIB.] LS:(5/5/2020 - ebrtesy) - LP: 3/3/2020 12:27 PM



LEGEND

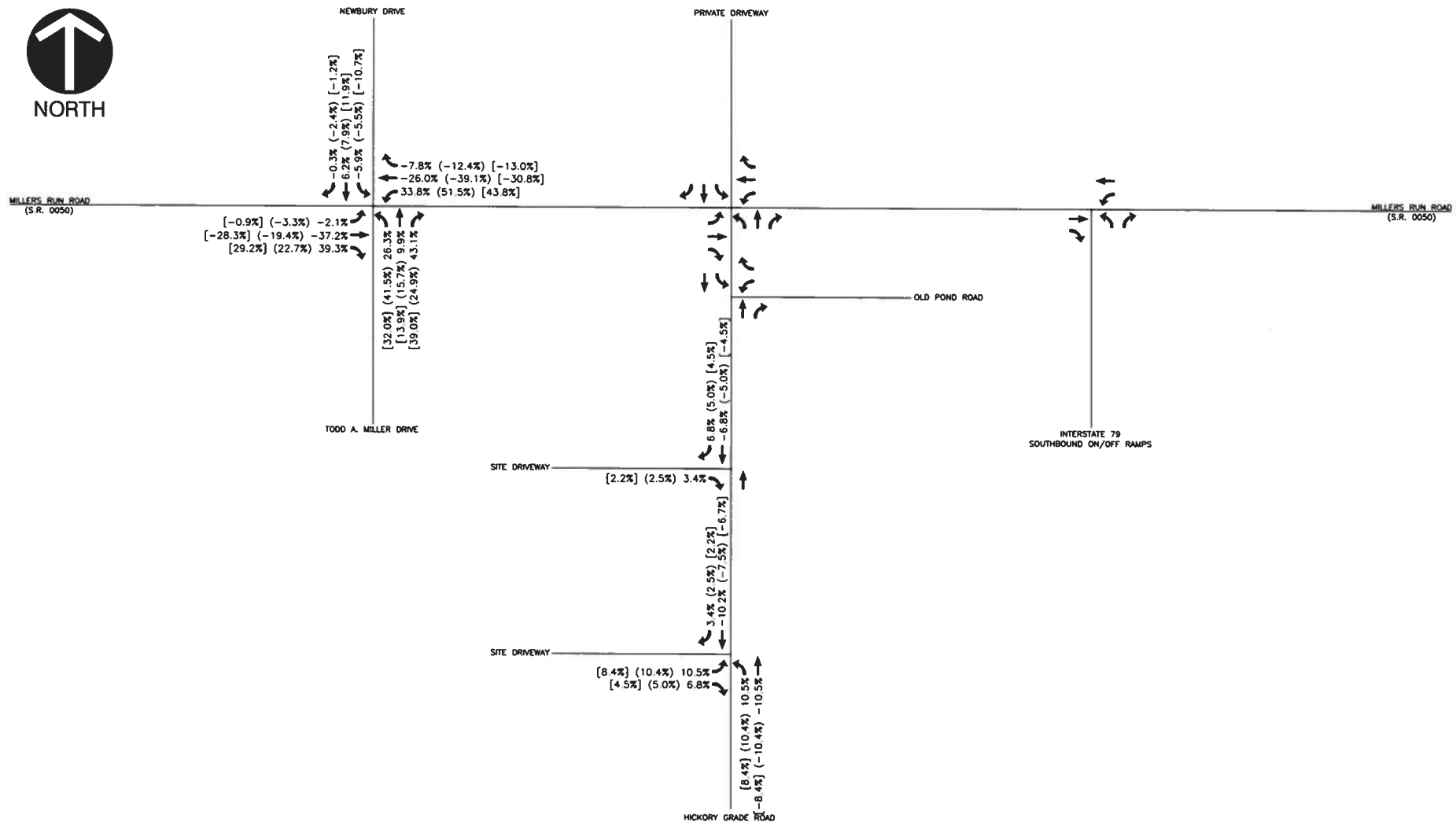
12% Arrival Trip Distribution

(12%) Departure Trip Distribution

<p>Civil & Environmental Consultants, Inc. 333 Baldwin Road · Pittsburgh, PA 15205 412-429-2324 · 800-365-2324 www.cecinc.com</p>		<p>THE PIAZZA RETAIL DEVELOPMENT TRAFFIC IMPACT STUDY SOUTH FAYETTE TOWNSHIP, ALLEGHENY COUNTY, PENNSYLVANIA</p>	
		<p>SITE GENERATED PRIMARY TRIP DISTRIBUTION</p>	
DRAWN BY: QAB DATE: APRIL 2020	CHECKED BY: JRT DWG SCALE: NOT TO SCALE	APPROVED BY: JMD PROJECT NO: 191-730	FIGURE NO.: 15



P:\2019\191-7301-CADD\DWG\191720-TR02-FIGURES-11x17.dwg\PASS BY TRIP DISTRIBUTION - 11x17.dwg\PASS BY TRIP DISTRIBUTION - 11x17.dwg - (P: 5/3/2020 12:27 PM)



LEGEND

12%	A.M. Trip Distribution Percentage
(12%)	P.M. Trip Distribution Percentage
[12%]	Saturday Trip Distribution Percentage


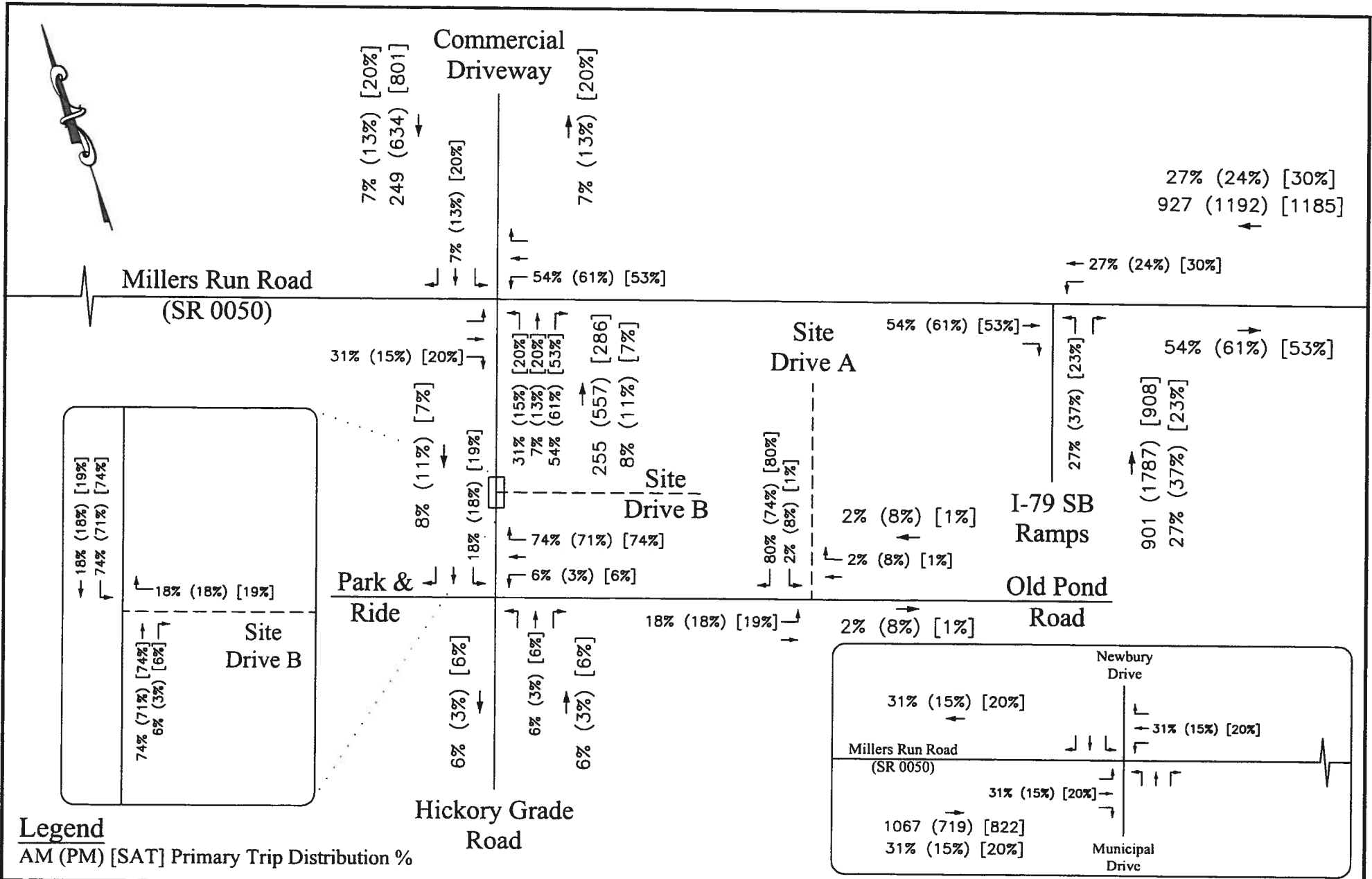
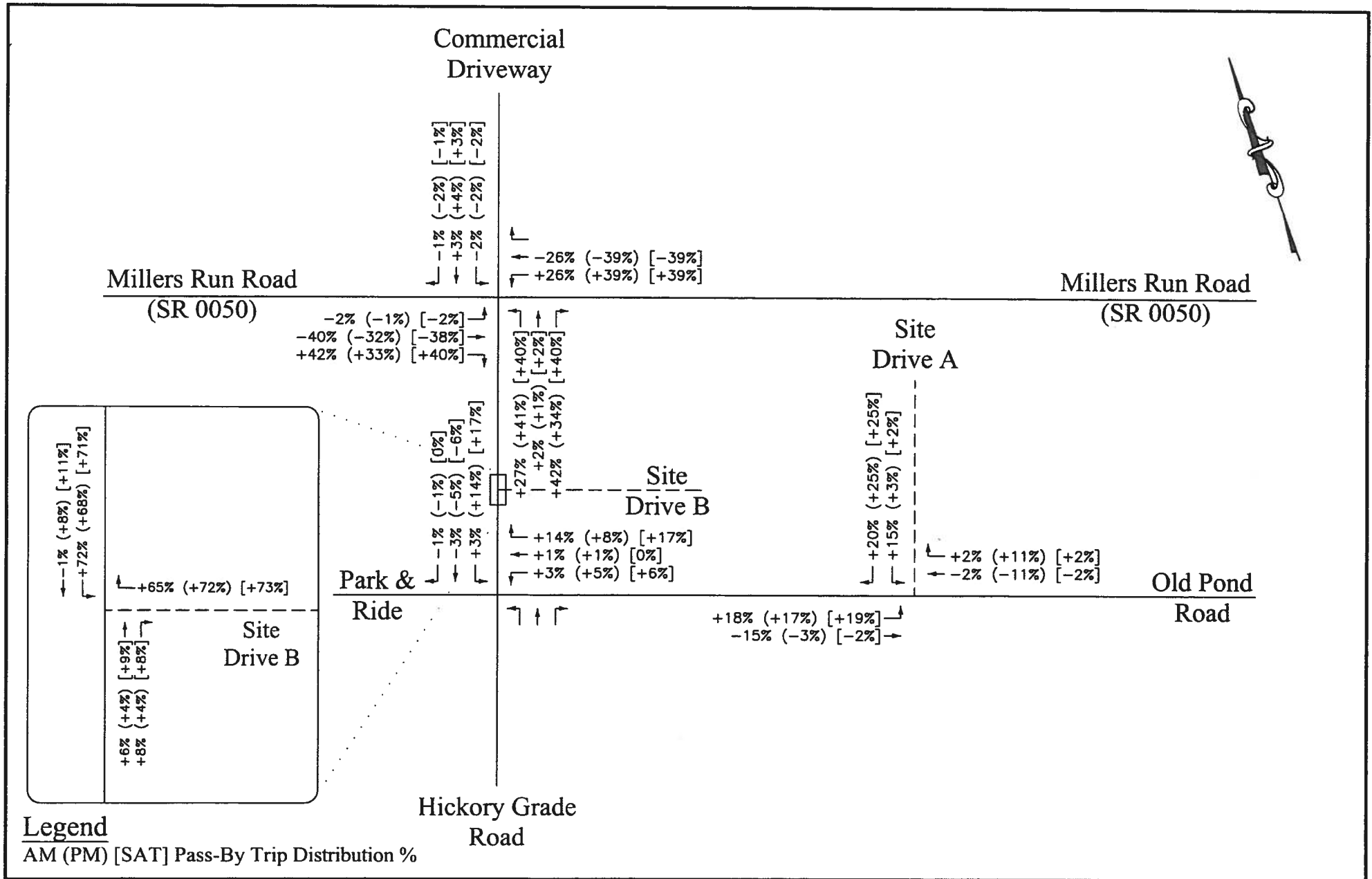
 Civil & Environmental Consultants, Inc. 333 Baldwin Road · Pittsburgh, PA 15205 412-429-2324 · 800-365-2324 www.cecinc.com		THE PIAZZA RETAIL DEVELOPMENT TRAFFIC IMPACT STUDY SOUTH FAYETTE TOWNSHIP, ALLEGHENY COUNTY, PENNSYLVANIA SITE GENERATED PASS-BY TRIP DISTRIBUTION	
DRAWN BY: QAB	CHECKED BY: JRT	APPROVED BY: JMD	FIGURE NO.: 16
DATE: APRIL 2020		DWG SCALE: NOT TO SCALE	
PROJECT NO: 191-7301			

Table 2
Trip Generation Summary

Time Period	Anticipated Trip Generation		
	In	Out	Total
LU Code #826 - Specialty Retail Center - 10,500 SF			
ADT	244	244	488
AM Peak Hour	0	0	0
Internal Trips (18%)	0	0	0
Primary Trips	0	0	0
Pass-By Trips (0%)	0	0	0
PM Peak Hour	21	26	47
Internal Trips (40%)	8	10	18
Primary Trips	13	16	29
Pass-By Trips (0%)	0	0	0
SAT Peak Hour	23	21	44
Internal Trips (53%)	12	11	23
Primary Trips	11	10	21
Pass-By Trips (0%)	0	0	0
LU Code #912 - Drive-in Bank - 3 drive-in lanes			
ADT	209	209	418
AM Peak Hour	17	11	28
Internal Trips (18%)	3	2	5
Primary Trips	9	6	15
Pass-By Trips (37%)	5	3	8
PM Peak Hour	49	51	100
Internal Trips (40%)	20	20	40
Primary Trips	15	16	31
Pass-By Trips (47%)	14	15	29
SAT Peak Hour	42	44	86
Internal Trips (53%)	22	23	45
Primary Trips	13	13	26
Pass-By Trips (37%)	7	8	15
LU Code #932 - High-Turnover (Sit-Down) Restaurant - 3,000 SF			
ADT	191	191	382
AM Peak Hour	18	14	32
Internal Trips (18%)	3	3	6
Primary Trips	10	7	17
Pass-By Trips (33%)	5	4	9
PM Peak Hour	18	12	30
Internal Trips (40%)	7	5	12
Primary Trips	6	4	10
Pass-By Trips (43%)	5	3	8
SAT Peak Hour	22	20	42
Internal Trips (53%)	12	11	23
Primary Trips	7	6	13
Pass-By Trips (33%)	3	3	6
LU Code #934 - Fast-Food Restaurant with Drive-Through Window - 2,800 SF			
ADT	695	695	1,390
AM Peak Hour	65	62	127
Internal Trips (18%)	12	11	23
Primary Trips	27	26	53
Pass-By Trips (49%)	26	25	51
PM Peak Hour	47	44	91
Internal Trips (40%)	19	18	37
Primary Trips	14	13	27
Pass-By Trips (50%)	14	13	27
SAT Peak Hour	84	81	165
Internal Trips (53%)	45	43	88
Primary Trips	23	23	46
Pass-By Trips (40%)	16	15	31
Total Trip Generation			
ADT	1,339	1,339	2,678
AM Peak Hour	100	87	187
Internal Trips	18	16	34
Primary Trips	46	39	85
Pass-By Trips	36	32	68
PM Peak Hour	135	133	268
Internal Trips	54	53	107
Primary Trips	48	49	97
Pass-By Trips	33	31	64
SAT Peak Hour	171	166	337
Internal Trips	91	88	179
Primary Trips	54	52	106
Pass-By Trips	26	26	52



PROPOSED COMMERCIAL DEVELOPMENT – South Fayette Township, PA
Primary Trip Distribution Percentages



PROPOSED COMMERCIAL DEVELOPMENT – South Fayette Township, PA
Total Pass-By Trip Distribution Percentages

APPENDIX I

Synchro Printouts – Opening Year 2024 Without Development Condition

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	26	623	90	122	367	216	57	28	113	196	40	21
Future Volume (vph)	26	623	90	122	367	216	57	28	113	196	40	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frnt		0.981				0.850			0.850		0.949	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3296	0	1778	3278	1613	1841	1700	1540	3416	1698	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3296	0	1778	3278	1613	1841	1700	1540	3416	1698	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13				237			124		17	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	29	685	99	134	403	237	63	31	124	215	44	23
Shared Lane Traffic (%)												
Lane Group Flow (vph)	29	784	0	134	403	237	63	31	124	215	67	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	6.5	28.9		11.5	39.7	39.7	7.8	6.8	6.8	10.3	8.7	
Actuated g/C Ratio	0.08	0.35		0.14	0.48	0.48	0.09	0.08	0.08	0.12	0.11	
v/c Ratio	0.22	0.68		0.54	0.26	0.26	0.36	0.22	0.52	0.51	0.35	
Control Delay	46.6	25.9		45.1	14.6	2.9	46.4	45.6	17.1	41.5	36.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	46.6	25.9		45.1	14.6	2.9	46.4	45.6	17.1	41.5	36.5	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		26.6			16.3			29.6			40.3	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	14	168		64	67	0	31	15	0	53	24	
Queue Length 95th (ft)	49	287		150	119	41	85	52	56	113	78	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	395	2263		428	2247	1180	327	625	645	607	635	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.07	0.35		0.31	0.18	0.20	0.19	0.05	0.19	0.35	0.11	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 82.7
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 25.0
 Intersection Capacity Utilization 54.1%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service A

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	50	513	66	99	671	364	58	35	142	306	52	68
Future Volume (vph)	50	513	66	99	671	364	58	35	142	306	52	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.983				0.850			0.850		0.915	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12				375			146		42	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	52	529	68	102	692	375	60	36	146	315	54	70
Shared Lane Traffic (%)												
Lane Group Flow (vph)	52	597	0	102	692	375	60	36	146	315	124	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.6	28.6		10.0	30.7	30.7	7.8	7.2	7.2	13.5	11.3	
Actuated g/C Ratio	0.09	0.35		0.12	0.38	0.38	0.10	0.09	0.09	0.17	0.14	
v/c Ratio	0.32	0.50		0.47	0.54	0.44	0.34	0.21	0.53	0.56	0.46	
Control Delay	47.6	22.2		47.0	21.4	3.7	47.3	45.1	15.9	40.6	33.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.6	22.2		47.0	21.4	3.7	47.3	45.1	15.9	40.6	33.5	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		24.2			17.9			28.0			38.6	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	26	123		51	143	0	30	18	0	79	39	
Queue Length 95th (ft)	77	203		128	226	51	86	58	60	#178	118	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	448	2373		454	2382	1262	346	755	712	643	680	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.12	0.25		0.22	0.29	0.30	0.17	0.05	0.21	0.49	0.18	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 81.6
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.56
 Intersection Signal Delay: 24.2
 Intersection Capacity Utilization 53.1%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service A
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O3
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	56	521	92	100	492	422	74	36	136	345	48	76
Future Volume (vph)	56	521	92	100	492	422	74	36	136	345	48	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.977				0.850			0.850		0.908	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3396	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3396	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				459			148		52	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	61	566	100	109	535	459	80	39	148	375	52	83
Shared Lane Traffic (%)												
Lane Group Flow (vph)	61	666	0	109	535	459	80	39	148	375	135	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.7	24.4		9.7	29.2	29.2	8.4	7.3	7.3	14.5	16.3	
Actuated g/C Ratio	0.10	0.30		0.12	0.36	0.36	0.10	0.09	0.09	0.18	0.20	
v/c Ratio	0.37	0.64		0.50	0.43	0.51	0.42	0.22	0.52	0.60	0.35	
Control Delay	45.1	26.7		44.7	21.4	4.2	44.9	41.4	14.6	38.7	26.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	45.1	26.7		44.7	21.4	4.2	44.9	41.4	14.6	38.7	26.0	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.2			16.5			27.6			35.3	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	28	140		49	105	0	36	18	0	84	35	
Queue Length 95th (ft)	83	237		127	179	60	101	57	58	#210	115	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	428	2360		442	2384	1303	331	722	706	620	672	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.14	0.28		0.25	0.22	0.35	0.24	0.05	0.21	0.60	0.20	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 80.8
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.64
 Intersection Signal Delay: 24.6
 Intersection Capacity Utilization 56.9%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

01	02	03	04
25 s	60 s	20 s	35 s
05	06	07	08
25 s	60 s	20 s	35 s

APPENDIX J

HCM Printouts – *Opening Year 2024 Without Development Condition*

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development AM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	26	623	90	122	367	216	57	28	113	196	40	21
Future Volume (veh/h)	26	623	90	122	367	216	57	28	113	196	40	21
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	29	685	99	134	403	0	63	31	0	215	44	8
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	58	935	135	176	1270		113	64		342	114	21
Arrive On Green	0.03	0.30	0.30	0.10	0.36	0.00	0.06	0.03	0.00	0.10	0.08	0.08
Sat Flow, veh/h	1747	3106	448	1818	3485	1656	1959	1847	1654	3445	1499	272
Grp Volume(v), veh/h	29	390	394	134	403	0	63	31	0	215	0	52
Grp Sat Flow(s),veh/h/ln	1747	1771	1784	1818	1743	1656	1959	1847	1654	1722	0	1771
Q Serve(g_s), s	0.8	10.1	10.1	3.7	4.3	0.0	1.6	0.8	0.0	3.1	0.0	1.4
Cycle Q Clear(g_c), s	0.8	10.1	10.1	3.7	4.3	0.0	1.6	0.8	0.0	3.1	0.0	1.4
Prop In Lane	1.00		0.25	1.00		1.00	1.00		1.00	1.00		0.15
Lane Grp Cap(c), veh/h	58	533	537	176	1270		113	64		342	0	135
V/C Ratio(X)	0.50	0.73	0.73	0.76	0.32		0.56	0.48		0.63	0.00	0.38
Avail Cap(c_a), veh/h	648	1867	1880	674	3673		535	1046		941	0	1002
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.4	16.1	16.1	22.6	11.7	0.0	23.5	24.3	0.0	22.2	0.0	22.5
Incr Delay (d2), s/veh	2.5	0.9	0.9	2.6	0.1	0.0	1.6	2.1	0.0	0.7	0.0	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	3.4	3.5	1.5	1.3	0.0	0.7	0.4	0.0	1.2	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.9	17.0	17.0	25.2	11.8	0.0	25.1	26.3	0.0	22.9	0.0	23.2
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		813			537			94			267	
Approach Delay, s/veh		17.3			15.1			25.5			22.9	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.9	21.4	9.0	9.9	7.7	24.7	11.1	7.8				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+I1), s	5.7	12.1	3.6	3.4	2.8	6.3	5.1	2.8				
Green Ext Time (p_c), s	0.1	3.3	0.0	0.1	0.0	1.8	0.3	0.1				

Intersection Summary

HCM 6th Ctrl Delay	18.0
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development PM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	50	513	66	99	671	364	58	35	142	306	52	68
Future Volume (veh/h)	50	513	66	99	671	364	58	35	142	306	52	68
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	52	529	65	102	692	0	60	36	0	315	54	37
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	94	944	116	135	1129		109	80		456	118	81
Arrive On Green	0.05	0.29	0.29	0.07	0.31	0.00	0.06	0.04	0.00	0.13	0.12	0.12
Sat Flow, veh/h	1847	3253	398	1818	3599	1683	1959	2057	1705	3445	1023	701
Grp Volume(v), veh/h	52	294	300	102	692	0	60	36	0	315	0	91
Grp Sat Flow(s),veh/h/ln	1847	1814	1838	1818	1800	1683	1959	2057	1705	1722	0	1724
Q Serve(g_s), s	1.4	7.1	7.2	2.8	8.4	0.0	1.5	0.9	0.0	4.5	0.0	2.5
Cycle Q Clear(g_c), s	1.4	7.1	7.2	2.8	8.4	0.0	1.5	0.9	0.0	4.5	0.0	2.5
Prop In Lane	1.00		0.22	1.00		1.00	1.00		1.00	1.00		0.41
Lane Grp Cap(c), veh/h	94	526	533	135	1129		109	80		456	0	199
V/C Ratio(X)	0.55	0.56	0.56	0.75	0.61		0.55	0.45		0.69	0.00	0.46
Avail Cap(c_a), veh/h	679	1894	1919	668	3759		531	1154		933	0	967
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.0	15.5	15.6	23.5	15.1	0.0	23.8	24.3	0.0	21.4	0.0	21.4
Incr Delay (d2), s/veh	1.9	0.4	0.4	3.2	0.2	0.0	1.6	1.5	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.5	2.5	1.2	2.8	0.0	0.7	0.4	0.0	1.7	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	25.8	16.0	16.0	26.7	15.3	0.0	25.4	25.7	0.0	22.1	0.0	22.0
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		646			794			96			406	
Approach Delay, s/veh		16.8			16.8			25.5			22.1	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.8	21.0	8.9	12.0	8.6	22.2	12.8	8.0				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	4.8	9.2	3.5	4.5	3.4	10.4	6.5	2.9				
Green Ext Time (p_c), s	0.1	2.3	0.0	0.3	0.0	3.3	0.4	0.1				

Intersection Summary























HCM 6th Ctrl Delay	18.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 Without Development SAT Peak Hour Condition

10/05/2023

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	56	521	92	100	492	422	74	36	136	345	48	76
Future Volume (veh/h)	56	521	92	100	492	422	74	36	136	345	48	76
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	61	566	96	109	535	0	80	39	0	375	52	45
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	103	888	150	144	1102		128	85		516	117	101
Arrive On Green	0.06	0.28	0.28	0.08	0.30	0.00	0.07	0.04	0.00	0.15	0.12	0.12
Sat Flow, veh/h	1847	3153	533	1847	3628	1709	1959	2057	1743	3472	937	811
Grp Volume(v), veh/h	61	330	332	109	535	0	80	39	0	375	0	97
Grp Sat Flow(s),veh/h/ln	1847	1842	1843	1847	1814	1709	1959	2057	1743	1736	0	1748
Q Serve(g_s), s	1.7	8.3	8.4	3.1	6.4	0.0	2.1	1.0	0.0	5.5	0.0	2.7
Cycle Q Clear(g_c), s	1.7	8.3	8.4	3.1	6.4	0.0	2.1	1.0	0.0	5.5	0.0	2.7
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.46
Lane Grp Cap(c), veh/h	103	519	519	144	1102		128	85		516	0	218
V/C Ratio(X)	0.59	0.64	0.64	0.76	0.49		0.63	0.46		0.73	0.00	0.44
Avail Cap(c_a), veh/h	659	1868	1869	659	3679		515	1120		913	0	952
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	16.7	16.8	24.1	15.1	0.0	24.3	25.0	0.0	21.6	0.0	21.6
Incr Delay (d2), s/veh	2.0	0.6	0.6	3.1	0.2	0.0	1.9	1.4	0.0	0.7	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	3.0	3.0	1.3	2.2	0.0	1.0	0.5	0.0	2.1	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.6	17.3	17.4	27.1	15.3	0.0	26.1	26.4	0.0	22.4	0.0	22.1
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		723			644			119			472	
Approach Delay, s/veh		18.1			17.3			26.2			22.3	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.1	21.0	9.5	12.6	9.0	22.2	13.9	8.2				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+1), s	5.1	10.4	4.1	4.7	3.7	8.4	7.5	3.0				
Green Ext Time (p_c), s	0.1	2.6	0.1	0.3	0.0	2.4	0.5	0.1				

Intersection Summary

HCM 6th Ctrl Delay	19.4
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

5: Newbury Drive & Plaza Access
 Opening Year 2024 Without Development AM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↓			↑↑
Traffic Vol, veh/h	35	11	211	59	17	215
Future Vol, veh/h	35	11	211	59	17	215
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	0	5	0	0	2
Mvmt Flow	41	13	248	69	20	253

Major/Minor	Minor1	Major1	Major2	Major3	Major4	Major5
Conflicting Flow All	450	159	0	0	317	0
Stage 1	283	-	-	-	-	-
Stage 2	167	-	-	-	-	-
Critical Hdwy	6.86	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-
Follow-up Hdwy	3.53	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	535	864	-	-	1255	-
Stage 1	737	-	-	-	-	-
Stage 2	842	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	525	864	-	-	1255	-
Mov Cap-2 Maneuver	525	-	-	-	-	-
Stage 1	737	-	-	-	-	-
Stage 2	826	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.9	0	0.7
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	579	1255
HCM Lane V/C Ratio	-	-	0.093	0.016
HCM Control Delay (s)	-	-	11.9	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.3	0

5: Newbury Drive & Plaza Access

Opening Year 2024 Without Development PM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	1.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↑			↑↑
Traffic Vol, veh/h	57	28	367	62	13	372
Future Vol, veh/h	57	28	367	62	13	372
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	62	30	399	67	14	404

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	663	233	0	0	466
Stage 1	433	-	-	-	-
Stage 2	230	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1
Critical Hdwy Stg 1	5.8	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	399	775	-	-	1106
Stage 1	627	-	-	-	-
Stage 2	792	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	393	775	-	-	1106
Mov Cap-2 Maneuver	393	-	-	-	-
Stage 1	627	-	-	-	-
Stage 2	779	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	14.5	0	0.4
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	469	1106
HCM Lane V/C Ratio	-	-	0.197	0.013
HCM Control Delay (s)	-	-	14.5	8.3
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.7	0

5: Newbury Drive & Plaza Access
 Opening Year 2024 Without Development SAT Peak Hour Condition

10/05/2023

Intersection						
Int Delay, s/veh	3.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↓			↑↑
Traffic Vol, veh/h	107	25	423	106	20	371
Future Vol, veh/h	107	25	423	106	20	371
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	1	4	0	2	0	1
Mvmt Flow	126	29	498	125	24	436

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	827	312	0	0	623
Stage 1	561	-	-	-	-
Stage 2	266	-	-	-	-
Critical Hdwy	6.82	6.98	-	-	4.1
Critical Hdwy Stg 1	5.82	-	-	-	-
Critical Hdwy Stg 2	5.82	-	-	-	-
Follow-up Hdwy	3.51	3.34	-	-	2.2
Pot Cap-1 Maneuver	312	678	-	-	968
Stage 1	538	-	-	-	-
Stage 2	757	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	302	678	-	-	968
Mov Cap-2 Maneuver	302	-	-	-	-
Stage 1	538	-	-	-	-
Stage 2	732	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	24.5	0	0.5
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	337	968
HCM Lane V/C Ratio	-	-	0.461	0.024
HCM Control Delay (s)	-	-	24.5	8.8
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	2.3	0.1

APPENDIX K

Synchro Printouts – Design Year 2029 Without Development Condition

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	655	90	122	386	227	57	28	113	206	40	22
Future Volume (vph)	28	655	90	122	386	227	57	28	113	206	40	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frnt		0.982				0.850			0.850		0.947	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12				249			124		18	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	31	720	99	134	424	249	63	31	124	226	44	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	31	819	0	134	424	249	63	31	124	226	68	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	6.7	30.9		11.7	41.7	41.7	7.9	6.8	6.8	10.7	9.0	
Actuated g/C Ratio	0.08	0.36		0.14	0.49	0.49	0.09	0.08	0.08	0.13	0.11	
v/c Ratio	0.24	0.68		0.55	0.26	0.27	0.37	0.23	0.52	0.53	0.35	
Control Delay	48.4	26.0		46.9	14.6	2.9	48.0	47.3	17.5	43.2	37.3	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	48.4	26.0		46.9	14.6	2.9	48.0	47.3	17.5	43.2	37.3	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		26.8			16.3			30.6			41.8	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	16	181		67	73	0	32	16	0	58	25	
Queue Length 95th (ft)	53	304		154	125	41	87	52	57	122	80	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	383	2198		416	2194	1162	317	607	629	589	616	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.37		0.32	0.19	0.21	0.20	0.05	0.20	0.38	0.11	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 85.3
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 25.3
 Intersection Capacity Utilization 55.3%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service B

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	53	539	66	99	705	381	58	35	142	321	52	71
Future Volume (vph)	53	539	66	99	705	381	58	35	142	321	52	71
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frnt		0.984				0.850			0.850		0.914	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3391	0	1778	3405	1644	1841	1938	1599	3416	1678	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3391	0	1778	3405	1644	1841	1938	1599	3416	1678	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11				393			146		44	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	55	556	68	102	727	393	60	36	146	331	54	73
Shared Lane Traffic (%)												
Lane Group Flow (vph)	55	624	0	102	727	393	60	36	146	331	127	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.8	32.3		10.1	34.3	34.3	7.9	7.3	7.3	14.4	11.5	
Actuated g/C Ratio	0.09	0.38		0.12	0.40	0.40	0.09	0.08	0.08	0.17	0.13	
v/c Ratio	0.35	0.49		0.49	0.54	0.44	0.36	0.22	0.54	0.58	0.48	
Control Delay	49.7	21.7		49.7	21.2	3.5	49.5	46.6	16.2	42.8	34.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	49.7	21.7		49.7	21.2	3.5	49.5	46.6	16.2	42.8	34.7	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		24.0			17.9			29.0			40.5	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	29	131		54	154	0	32	19	0	88	42	
Queue Length 95th (ft)	81	215		128	243	52	87	58	61	#194	120	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	420	2255		426	2260	1223	325	709	678	603	642	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.13	0.28		0.24	0.32	0.32	0.18	0.05	0.22	0.55	0.20	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 86
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.58
 Intersection Signal Delay: 24.5
 Intersection Capacity Utilization 57.0%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	59	548	92	100	517	442	74	36	136	362	48	79
Future Volume (vph)	59	548	92	100	517	442	74	36	136	362	48	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.978				0.850			0.850		0.907	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3401	0	1814	3438	1677	1841	1938	1647	3450	1715	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3401	0	1814	3438	1677	1841	1938	1647	3450	1715	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		16				480			148		54	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		1114			644			295			551	
Travel Time (s)		19.0			11.0			8.0			15.0	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	64	596	100	109	562	480	80	39	148	393	52	86
Shared Lane Traffic (%)												
Lane Group Flow (vph)	64	696	0	109	562	480	80	39	148	393	138	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	3.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	None
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	8.0	26.2		10.0	31.1	31.1	8.5	7.5	7.5	14.6	16.5	
Actuated g/C Ratio	0.10	0.31		0.12	0.37	0.37	0.10	0.09	0.09	0.18	0.20	
v/c Ratio	0.38	0.65		0.50	0.44	0.52	0.43	0.23	0.52	0.65	0.36	
Control Delay	47.3	26.6		46.5	21.4	4.1	46.9	43.2	14.8	41.4	27.1	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.3	26.6		46.5	21.4	4.1	46.9	43.2	14.8	41.4	27.1	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.3			16.5			28.6			37.7	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	30	148		50	112	0	37	18	0	92	37	
Queue Length 95th (ft)	89	254		134	192	60	106	59	59	#246	122	
Internal Link Dist (ft)		1034			564			215			471	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	418	2309		432	2329	1290	323	705	693	605	658	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.15	0.30		0.25	0.24	0.37	0.25	0.06	0.21	0.65	0.21	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 83.4
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.65
 Intersection Signal Delay: 25.2
 Intersection Capacity Utilization 58.1%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road























Q1	Q2	Q3	Q4
25 s	60 s	20 s	35 s
Q5	Q6	Q7	Q8
25 s	60 s	20 s	35 s

APPENDIX L

HCM Printouts – *Design Year 2029 Without Development Condition*

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development AM Peak Hour Condition

10/05/2023

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	655	90	122	386	227	57	28	113	206	40	22
Future Volume (veh/h)	28	655	90	122	386	227	57	28	113	206	40	22
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	31	720	99	134	424	0	63	31	0	226	44	9
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	60	970	133	175	1296		112	64		353	117	24
Arrive On Green	0.03	0.31	0.31	0.10	0.37	0.00	0.06	0.03	0.00	0.10	0.08	0.08
Sat Flow, veh/h	1747	3128	430	1818	3485	1656	1959	1847	1654	3445	1466	300
Grp Volume(v), veh/h	31	407	412	134	424	0	63	31	0	226	0	53
Grp Sat Flow(s),veh/h/ln	1747	1771	1787	1818	1743	1656	1959	1847	1654	1722	0	1766
Q Serve(g_s), s	0.9	10.8	10.9	3.8	4.6	0.0	1.6	0.9	0.0	3.3	0.0	1.5
Cycle Q Clear(g_c), s	0.9	10.8	10.9	3.8	4.6	0.0	1.6	0.9	0.0	3.3	0.0	1.5
Prop In Lane	1.00		0.24	1.00		1.00	1.00		1.00	1.00		0.17
Lane Grp Cap(c), veh/h	60	549	554	175	1296		112	64		353	0	141
V/C Ratio(X)	0.51	0.74	0.74	0.76	0.33		0.56	0.48		0.64	0.00	0.38
Avail Cap(c_a), veh/h	631	1818	1835	657	3578		522	1019		917	0	974
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.9	16.3	16.3	23.2	11.8	0.0	24.2	24.9	0.0	22.7	0.0	23.0
Incr Delay (d2), s/veh	2.5	0.9	0.9	2.6	0.1	0.0	1.6	2.1	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	3.7	3.7	1.6	1.4	0.0	0.8	0.4	0.0	1.3	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.4	17.2	17.2	25.8	11.9	0.0	25.8	27.0	0.0	23.4	0.0	23.6
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		850			558			94			279	
Approach Delay, s/veh		17.6			15.2			26.2			23.4	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.1	22.3	9.0	10.2	7.8	25.6	11.4	7.8				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	5.8	12.9	3.6	3.5	2.9	6.6	5.3	2.9				
Green Ext Time (p_c), s	0.1	3.5	0.0	0.1	0.0	1.9	0.3	0.1				

Intersection Summary

HCM 6th Ctrl Delay	18.2
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development PM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	53	539	66	99	705	381	58	35	142	321	52	71
Future Volume (veh/h)	53	539	66	99	705	381	58	35	142	321	52	71
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	55	556	65	102	727	0	60	36	0	331	54	40
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	97	944	110	135	1115		109	80		472	119	88
Arrive On Green	0.05	0.29	0.29	0.07	0.31	0.00	0.06	0.04	0.00	0.14	0.12	0.12
Sat Flow, veh/h	1847	3273	382	1818	3599	1683	1959	2057	1705	3445	987	731
Grp Volume(v), veh/h	55	307	314	102	727	0	60	36	0	331	0	94
Grp Sat Flow(s),veh/h/ln	1847	1814	1841	1818	1800	1683	1959	2057	1705	1722	0	1718
Q Serve(g_s), s	1.5	7.6	7.6	2.9	9.1	0.0	1.6	0.9	0.0	4.8	0.0	2.6
Cycle Q Clear(g_c), s	1.5	7.6	7.6	2.9	9.1	0.0	1.6	0.9	0.0	4.8	0.0	2.6
Prop In Lane	1.00		0.21	1.00		1.00	1.00		1.00	1.00		0.43
Lane Grp Cap(c), veh/h	97	523	531	135	1115		109	80		472	0	207
V/C Ratio(X)	0.57	0.59	0.59	0.76	0.65		0.55	0.45		0.70	0.00	0.45
Avail Cap(c_a), veh/h	675	1883	1911	664	3737		527	1147		927	0	958
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.1	15.9	15.9	23.6	15.5	0.0	23.9	24.4	0.0	21.4	0.0	21.3
Incr Delay (d2), s/veh	1.9	0.5	0.5	3.2	0.3	0.0	1.6	1.5	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.6	2.7	1.2	3.1	0.0	0.7	0.4	0.0	1.8	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.0	16.3	16.4	26.9	15.8	0.0	25.5	25.9	0.0	22.1	0.0	21.9
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		676			829			96			425	
Approach Delay, s/veh		17.1			17.2			25.7			22.1	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.9	21.0	8.9	12.3	8.7	22.1	13.1	8.0				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+I1), s	4.9	9.6	3.6	4.6	3.5	11.1	6.8	2.9				
Green Ext Time (p_c), s	0.1	2.4	0.0	0.3	0.0	3.5	0.4	0.1				

Intersection Summary

HCM 6th Ctrl Delay	18.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 Without Development SAT Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	59	548	92	100	517	442	74	36	136	362	48	79
Future Volume (veh/h)	59	548	92	100	517	442	74	36	136	362	48	79
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	64	596	96	109	562	0	80	39	0	393	52	48
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	106	889	143	144	1089		127	85		534	118	109
Arrive On Green	0.06	0.28	0.28	0.08	0.30	0.00	0.06	0.04	0.00	0.15	0.13	0.13
Sat Flow, veh/h	1847	3179	511	1847	3628	1709	1959	2057	1743	3472	907	837
Grp Volume(v), veh/h	64	345	347	109	562	0	80	39	0	393	0	100
Grp Sat Flow(s),veh/h/ln	1847	1842	1847	1847	1814	1709	1959	2057	1743	1736	0	1743
Q Serve(g_s), s	1.8	8.9	8.9	3.1	6.9	0.0	2.1	1.0	0.0	5.8	0.0	2.8
Cycle Q Clear(g_c), s	1.8	8.9	8.9	3.1	6.9	0.0	2.1	1.0	0.0	5.8	0.0	2.8
Prop In Lane	1.00		0.28	1.00		1.00	1.00		1.00	1.00		0.48
Lane Grp Cap(c), veh/h	106	515	517	144	1089		127	85		534	0	226
V/C Ratio(X)	0.60	0.67	0.67	0.76	0.52		0.63	0.46		0.74	0.00	0.44
Avail Cap(c_a), veh/h	655	1856	1861	655	3654		512	1113		907	0	943
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.7	17.1	17.1	24.2	15.5	0.0	24.4	25.1	0.0	21.7	0.0	21.5
Incr Delay (d2), s/veh	2.1	0.7	0.7	3.1	0.2	0.0	1.9	1.5	0.0	0.8	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	3.2	3.3	1.3	2.4	0.0	1.0	0.5	0.0	2.3	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.7	17.8	17.8	27.3	15.7	0.0	26.3	26.6	0.0	22.4	0.0	22.0
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		756			671			119			493	
Approach Delay, s/veh		18.6			17.6			26.4			22.3	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.2	21.0	9.5	13.0	9.1	22.1	14.2	8.2				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+I1), s	5.1	10.9	4.1	4.8	3.8	8.9	7.8	3.0				
Green Ext Time (p_c), s	0.1	2.8	0.1	0.3	0.0	2.6	0.5	0.1				

Intersection Summary

HCM 6th Ctrl Delay	19.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

5: Newbury Drive & Plaza Access
 Design Year 2029 Without Development AM Peak Hour Condition

10/05/2023

Intersection						
Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↑			↑↑
Traffic Vol, veh/h	35	11	221	59	17	225
Future Vol, veh/h	35	11	221	59	17	225
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	0	5	0	0	2
Mvmt Flow	41	13	260	69	20	265

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	468	165	0	0	329
Stage 1	295	-	-	-	-
Stage 2	173	-	-	-	-
Critical Hdwy	6.86	6.9	-	-	4.1
Critical Hdwy Stg 1	5.86	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-
Follow-up Hdwy	3.53	3.3	-	-	2.2
Pot Cap-1 Maneuver	521	857	-	-	1242
Stage 1	727	-	-	-	-
Stage 2	837	-	-	-	-
Platoon blocked, %					
Mov Cap-1 Maneuver	511	857	-	-	1242
Mov Cap-2 Maneuver	511	-	-	-	-
Stage 1	727	-	-	-	-
Stage 2	821	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12	0	0.7
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	566	1242
HCM Lane V/C Ratio	-	-	0.096	0.016
HCM Control Delay (s)	-	-	12	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.3	0

5: Newbury Drive & Plaza Access
 Design Year 2029 Without Development PM Peak Hour Condition

10/05/2023

Intersection						
Int Delay, s/veh	1.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↑			↑↑
Traffic Vol, veh/h	57	28	384	62	13	389
Future Vol, veh/h	57	28	384	62	13	389
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	62	30	417	67	14	423

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	691	242	0	0	484
Stage 1	451	-	-	-	-
Stage 2	240	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1
Critical Hdwy Stg 1	5.8	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	383	765	-	-	1089
Stage 1	614	-	-	-	-
Stage 2	783	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	376	765	-	-	1089
Mov Cap-2 Maneuver	376	-	-	-	-
Stage 1	614	-	-	-	-
Stage 2	770	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	15	0	0.4
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	452	1089
HCM Lane V/C Ratio	-	-	0.204	0.013
HCM Control Delay (s)	-	-	15	8.3
HCM Lane LOS	-	-	C	A
HCM 95th %tile Q(veh)	-	-	0.8	0

5: Newbury Drive & Plaza Access
 Design Year 2029 Without Development SAT Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	3.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		↑↑			↑↑
Traffic Vol, veh/h	107	25	441	106	20	387
Future Vol, veh/h	107	25	441	106	20	387
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	-1	-	-	1
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	1	4	0	2	0	1
Mvmt Flow	126	29	519	125	24	455

Major/Minor	Minor1	Major1	Major2	Major2	Major2
Conflicting Flow All	858	322	0	0	644
Stage 1	582	-	-	-	-
Stage 2	276	-	-	-	-
Critical Hdwy	6.82	6.98	-	-	4.1
Critical Hdwy Stg 1	5.82	-	-	-	-
Critical Hdwy Stg 2	5.82	-	-	-	-
Follow-up Hdwy	3.51	3.34	-	-	2.2
Pot Cap-1 Maneuver	298	668	-	-	951
Stage 1	525	-	-	-	-
Stage 2	749	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	288	668	-	-	951
Mov Cap-2 Maneuver	288	-	-	-	-
Stage 1	525	-	-	-	-
Stage 2	724	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	26	0	0.5
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	323	951
HCM Lane V/C Ratio	-	-	0.481	0.025
HCM Control Delay (s)	-	-	26	8.9
HCM Lane LOS	-	-	D	A
HCM 95th %tile Q(veh)	-	-	2.5	0.1

APPENDIX M
Trip Generation Calculations

South Fayette Commons

Trip Generation Summary

Time Period	Anticipated Trip Generation		
	In	Out	Total
LU Code #826 - Specialty Retail Center - 10,500 SF			
ADT	244	244	488
AM Peak Hour	0	0	0
<i>Internal Trips (18%)</i>	<i>0</i>	<i>0</i>	<i>0</i>
<i>Primary Trips</i>	<i>0</i>	<i>0</i>	<i>0</i>
<i>Pass-By Trips (0%)</i>	<i>0</i>	<i>0</i>	<i>0</i>
PM Peak Hour	21	26	47
<i>Internal Trips (40%)</i>	<i>8</i>	<i>10</i>	<i>18</i>
<i>Primary Trips</i>	<i>13</i>	<i>16</i>	<i>29</i>
<i>Pass-By Trips (0%)</i>	<i>0</i>	<i>0</i>	<i>0</i>
SAT Peak Hour	23	21	44
<i>Internal Trips (53%)</i>	<i>12</i>	<i>11</i>	<i>23</i>
<i>Primary Trips</i>	<i>11</i>	<i>10</i>	<i>21</i>
<i>Pass-By Trips (0%)</i>	<i>0</i>	<i>0</i>	<i>0</i>
LU Code #932 - High-Turnover (Sit-Down) Restaurant - 3,000 SF			
ADT	191	191	382
AM Peak Hour	18	14	32
<i>Internal Trips (18%)</i>	<i>3</i>	<i>3</i>	<i>6</i>
<i>Primary Trips</i>	<i>10</i>	<i>7</i>	<i>17</i>
<i>Pass-By Trips (33%)</i>	<i>5</i>	<i>4</i>	<i>9</i>
PM Peak Hour	18	12	30
<i>Internal Trips (40%)</i>	<i>7</i>	<i>5</i>	<i>12</i>
<i>Primary Trips</i>	<i>6</i>	<i>4</i>	<i>10</i>
<i>Pass-By Trips (43%)</i>	<i>5</i>	<i>3</i>	<i>8</i>
SAT Peak Hour	22	20	42
<i>Internal Trips (53%)</i>	<i>12</i>	<i>11</i>	<i>23</i>
<i>Primary Trips</i>	<i>7</i>	<i>6</i>	<i>13</i>
<i>Pass-By Trips (33%)</i>	<i>3</i>	<i>3</i>	<i>6</i>
Remaining Development Trips			
ADT	435	435	870
AM Peak Hour	18	14	32
<i>Internal Trips</i>	<i>3</i>	<i>3</i>	<i>6</i>
<i>Primary Trips</i>	<i>10</i>	<i>7</i>	<i>17</i>
<i>Pass-By Trips</i>	<i>5</i>	<i>4</i>	<i>9</i>
PM Peak Hour	39	38	77
<i>Internal Trips</i>	<i>15</i>	<i>15</i>	<i>30</i>
<i>Primary Trips</i>	<i>19</i>	<i>20</i>	<i>39</i>
<i>Pass-By Trips</i>	<i>5</i>	<i>3</i>	<i>8</i>
SAT Peak Hour	45	41	86
<i>Internal Trips</i>	<i>24</i>	<i>22</i>	<i>46</i>
<i>Primary Trips</i>	<i>18</i>	<i>16</i>	<i>34</i>
<i>Pass-By Trips</i>	<i>3</i>	<i>3</i>	<i>6</i>

The Piazza Development Trip Generation Summary

Time Period	Anticipated Trip Generation		
	In	Out	Total
LU Code #932 - High-Turnover (Sit-Down) Restaurant - 6,800 SF			
ADT	365	365	730
AM Peak Hour	36	29	65
<i>Primary Trips</i>	24	19	43
<i>Pass-By Trips (33%)</i>	12	10	22
PM Peak Hour	38	24	62
<i>Primary Trips</i>	22	14	36
<i>Pass-By Trips (43%)</i>	16	10	26
SAT Peak Hour	39	37	76
<i>Primary Trips</i>	26	25	51
<i>Pass-By Trips (33%)</i>	13	12	25
LU Code #934 - Fast-Food Restaurant with Drive-Through Window - 8,250 SF			
ADT	1,929	1,929	250
AM Peak Hour	188	180	368
<i>Primary Trips</i>	96	92	188
<i>Pass-By Trips (49%)</i>	92	88	180
PM Peak Hour	141	131	272
<i>Primary Trips</i>	97	90	187
<i>Pass-By Trips (31%)</i>	44	41	85
SAT Peak Hour	233	223	456
<i>Primary Trips</i>	184	176	360
<i>Pass-By Trips (21%)</i>	49	47	96
Total Trip Generation			
ADT	2,294	2,294	980
AM Peak Hour	224	209	433
<i>Primary Trips</i>	120	111	231
<i>Pass-By Trips</i>	104	98	202
PM Peak Hour	179	155	334
<i>Primary Trips</i>	119	104	223
<i>Pass-By Trips</i>	60	51	111
SAT Peak Hour	272	260	532
<i>Primary Trips</i>	210	201	411
<i>Pass-By Trips</i>	62	59	121

TRIP GENERATION

David E. Wooster and Associates, LLC
 2 East Crafton Avenue
 Pittsburgh, PA 15205

County : Allegheny County
 Municipality : South Fayette Township
 Client Code: #4392

Land Use Code: 821
 Description: Shopping Plaza (40-150k) without Supermarket

SF Gross Leasable Area: 45,126
 X = 1,000 Square Feet Gross Leasable Area

Page: 212 ADT

Equation: Not Given *Average Rate = 67.52*
 T = 3,048 ADT
 50% entering = 1,524
 50% exiting = 1,524

Page: 213 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given *Average Rate = 1.73*
 T = 78 AM Peak
 62% entering = 48
 38% exiting = 30

Table E.9	
Primary	Pass-by (30%)*
34	14
21	9

Page: 214 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given *Average Rate = 5.19*
 T = 234 PM Peak
 49% entering = 115
 51% exiting = 119

Table E.9	
Primary	Pass-by (40%)
69	46
71	48

Page: 218 Saturday (Peak hour of generator)

Equation: T = 7.75 (X) - 98.93 *R² = 0.58*
 T = 251 SAT Peak
 52% entering = 131
 48% exiting = 120

Table E.10	
Primary	Pass-by (31%)
90	41
83	37

* PM peak hour minus 10%

TRIP GENERATION

David E. Wooster and Associates, LLC
 2 East Crafton Avenue
 Pittsburgh, PA 15205

County : Allegheny County
 Municipality : South Fayette Township
 Client Code: #4392

Land Use Code: 932
 Description: High-Turnover (Sit-Down) Restaurant

SF Gross Floor Area: 6,800
 X = 1,000 Square Feet Gross Floor Area

PIAZZA BACKGROUND TRIPS

Page: 673 ADT

Equation: Not Given Average Rate = 107.20
 T = 730 ADT
 50% entering = 365
 50% exiting = 365

Page: 674 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given Average Rate = 9.57
 T = 65 AM Peak
 55% entering = 36
 45% exiting = 29

Table E.30	
Primary	Pass-by (33%)*
24	12
19	10

Page: 675 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given Average Rate = 9.05
 T = 62 PM Peak
 61% entering = 38
 39% exiting = 24

Table E.30	
Primary	Pass-by (43%)
22	16
14	10

Page: 682 Saturday (peak hour of generator)

Equation: Not Given Average Rate = 11.19
 T = 76 SAT Peak
 51% entering = 39
 49% exiting = 37

Table E.30	
Primary	Pass-by (33%)*
26	13
25	12

* PM peak hour minus 10%

TRIP GENERATION

David E. Wooster and Associates, LLC
 2 East Crafton Avenue
 Pittsburgh, PA 15205

County : Allegheny County
 Municipality : South Fayette Township
 Client Code: #4392

Land Use Code: 934
 Description: Fast-Food Restaurant with Drive-Through Window

SF Gross Floor Area: 8,250
 X = 1,000 Square Feet Gross Floor Area

PIAZZA BACKGROUND TRIPS

Page: 725 ADT

Equation: Not Given *Average Rate = 467.48*
 T = 3,858 ADT
 50% entering = 1,929
 50% exiting = 1,929

Page: 726 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given *Average Rate = 44.61*
 T = 368 AM Peak
 51% entering = 188
 49% exiting = 180

Table E.31	
Primary	Pass-by (49%)
96	92
92	88

Page: 727 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given *Average Rate = 33.03*
 T = 272 PM Peak
 52% entering = 141
 48% exiting = 131

Table E.32	
Primary	Pass-by (31%)
97	44
90	41

Page: 731 Saturday (peak hour of generator)

Equation: Not Given *Average Rate = 55.25*
 T = 456 SAT Peak
 51% entering = 233
 49% exiting = 223

Table E.32	
Primary	Pass-by (21%)*
184	49
176	47

* PM peak hour minus 10%

TRIP GENERATION

David E. Wooster and Associates, LLC
 2 East Crafton Avenue
 Pittsburgh, PA 15205

County : Allegheny County
 Municipality : South Fayette Township
 Client Code: #4392

Land Use Code: 931
 Description: Fine Dining Restaurant

SF Gross Floor Area: 9,377
 X = 1,000 Square Feet Gross Floor Area

NEWBURY CIGAR LOUNGE, BAR, AND RESTAURANT

Page: 646 ADT

Equation: Not Given Average Rate = 83.84
 T = 788 ADT
 50% entering = 394
 50% exiting = 394

Page: 647 AM Peak Hour (adjacent street between 7 and 9 a.m.)

Equation: Not Given Average Rate = 0.73
 T = 7 AM Peak
 **50% entering = 4
 **50% exiting = 3

Table E.29	
Primary	Pass-by (34%)*
3	1
2	1

Page: 648 PM Peak Hour (adjacent street between 4 and 6 p.m.)

Equation: Not Given Average Rate = 7.80
 T = 73 PM Peak
 67% entering = 49
 33% exiting = 24

Table E.29	
Primary	Pass-by (44%)
27	22
13	11

Page: 652 Saturday (peak hour of generator)

Equation: Not Given Average Rate = 10.68
 T = 100 SAT Peak
 59% entering = 59
 41% exiting = 41

Table E.29	
Primary	Pass-by (34%)*
39	20
21	20

**-Directional Distribution Not Available. Distribution Assumed.

* PM peak hour minus 10%

APPENDIX N

Synchro Printouts – Opening Year 2024 With Development Condition

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	617	90	121	381	217	57	28	113	211	40	22
Future Volume (vph)	47	617	90	121	381	217	57	28	113	211	40	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.981				0.850			0.850		0.947	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3297	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3297	0	1778	3278	1613	1841	1700	1540	3416	1694	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13				238			124		18	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	52	678	99	133	419	238	63	31	124	232	44	24
Shared Lane Traffic (%)												
Lane Group Flow (vph)	52	777	0	133	419	238	63	31	124	232	68	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.7	28.8		11.5	35.8	35.8	7.8	6.7	6.7	10.6	8.9	
Actuated g/C Ratio	0.09	0.35		0.14	0.43	0.43	0.09	0.08	0.08	0.13	0.11	
v/c Ratio	0.34	0.67		0.54	0.30	0.29	0.36	0.22	0.52	0.53	0.34	
Control Delay	47.0	25.9		45.3	17.1	3.3	46.3	45.6	17.0	41.6	35.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.0	25.9		45.3	17.1	3.3	46.3	45.6	17.0	41.6	35.8	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		27.2			17.7			29.5			40.3	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	25	167		64	74	0	31	15	0	57	24	
Queue Length 95th (ft)	75	284		148	130	43	86	51	57	121	77	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	394	2260		428	2243	1179	326	625	644	606	634	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.13	0.34		0.31	0.19	0.20	0.19	0.05	0.19	0.38	0.11	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 82.8
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 25.8
 Intersection Capacity Utilization 54.3%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service A

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	81	502	66	98	706	367	58	37	140	351	53	70
Future Volume (vph)	81	502	66	98	706	367	58	37	140	351	53	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frnt		0.983				0.850			0.850		0.915	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3388	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3388	0	1778	3405	1644	1841	1938	1599	3416	1679	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12				378			144		42	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	84	518	68	101	728	378	60	38	144	362	55	72
Shared Lane Traffic (%)												
Lane Group Flow (vph)	84	586	0	101	728	378	60	38	144	362	127	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	9.3	33.5		10.1	37.6	37.6	7.8	7.4	7.4	14.6	17.3	
Actuated g/C Ratio	0.10	0.37		0.11	0.41	0.41	0.09	0.08	0.08	0.16	0.19	
v/c Ratio	0.47	0.47		0.51	0.52	0.42	0.38	0.24	0.55	0.66	0.36	
Control Delay	52.1	22.0		51.9	21.8	3.5	51.7	48.3	16.7	46.0	31.9	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	52.1	22.0		51.9	21.8	3.5	51.7	48.3	16.7	46.0	31.9	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		25.8			18.6			30.3			42.4	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	45	121		54	160	0	32	21	0	99	43	
Queue Length 95th (ft)	114	202		131	257	54	89	62	61	#233	125	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	384	2115		389	2121	1166	297	648	630	551	589	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.28		0.26	0.34	0.32	0.20	0.06	0.23	0.66	0.22	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 90.7
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.66
 Intersection Signal Delay: 26.0
 Intersection Capacity Utilization 58.2%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	96	510	92	99	534	426	74	37	135	394	49	78
Future Volume (vph)	96	510	92	99	534	426	74	37	135	394	49	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.977				0.850			0.850		0.908	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3395	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3395	0	1814	3438	1677	1841	1938	1647	3450	1717	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				463			147		52	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	104	554	100	108	580	463	80	40	147	428	53	85
Shared Lane Traffic (%)												
Lane Group Flow (vph)	104	654	0	108	580	463	80	40	147	428	138	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	10.2	27.1		10.2	30.3	30.3	8.7	7.8	7.8	14.8	16.9	
Actuated g/C Ratio	0.12	0.32		0.12	0.36	0.36	0.10	0.09	0.09	0.17	0.20	
v/c Ratio	0.50	0.60		0.50	0.48	0.52	0.43	0.23	0.52	0.71	0.36	
Control Delay	48.4	25.5		48.0	23.4	4.3	48.5	44.6	14.9	44.6	28.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	48.4	25.5		48.0	23.4	4.3	48.5	44.6	14.9	44.6	28.6	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.7			18.0			29.4			40.7	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	47	137		48	122	0	36	18	0	98	37	
Queue Length 95th (ft)	136	239		140	213	62	112	63	61	#301	131	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	413	2280		427	2304	1276	320	697	686	599	651	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.25	0.29		0.25	0.25	0.36	0.25	0.06	0.21	0.71	0.21	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 85.3
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.71
 Intersection Signal Delay: 26.8
 Intersection Capacity Utilization 57.9%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

O1	O2	O3	O4
25 s	60 s	20 s	35 s
O5	O6	O7	O8
25 s	60 s	20 s	35 s

APPENDIX O

HCM Printouts – *Opening Year 2024 With Development Condition*

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development AM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	47	617	90	121	381	217	57	28	113	211	40	22
Future Volume (veh/h)	47	617	90	121	381	217	57	28	113	211	40	22
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	52	678	99	133	419	0	63	31	0	232	44	9
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	89	925	135	174	1196		113	64		362	121	25
Arrive On Green	0.05	0.30	0.30	0.10	0.34	0.00	0.06	0.03	0.00	0.11	0.08	0.08
Sat Flow, veh/h	1747	3101	452	1818	3485	1656	1959	1847	1654	3445	1466	300
Grp Volume(v), veh/h	52	387	390	133	419	0	63	31	0	232	0	53
Grp Sat Flow(s),veh/h/ln	1747	1771	1783	1818	1743	1656	1959	1847	1654	1722	0	1766
Q Serve(g_s), s	1.5	10.1	10.1	3.7	4.6	0.0	1.6	0.8	0.0	3.3	0.0	1.5
Cycle Q Clear(g_c), s	1.5	10.1	10.1	3.7	4.6	0.0	1.6	0.8	0.0	3.3	0.0	1.5
Prop In Lane	1.00		0.25	1.00		1.00	1.00		1.00	1.00		0.17
Lane Grp Cap(c), veh/h	89	528	532	174	1196		113	64		362	0	145
V/C Ratio(X)	0.58	0.73	0.73	0.76	0.35		0.56	0.48		0.64	0.00	0.36
Avail Cap(c_a), veh/h	644	1857	1869	671	3654		533	1040		936	0	994
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	23.9	16.2	16.2	22.7	12.6	0.0	23.6	24.4	0.0	22.1	0.0	22.4
Incr Delay (d2), s/veh	2.3	0.9	0.9	2.6	0.1	0.0	1.6	2.1	0.0	0.7	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	3.4	3.5	1.5	1.5	0.0	0.8	0.4	0.0	1.3	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.2	17.1	17.1	25.3	12.7	0.0	25.2	26.5	0.0	22.8	0.0	22.9
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		829			552			94			285	
Approach Delay, s/veh		17.7			15.7			25.6			22.8	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.9	21.4	9.0	10.2	8.6	23.7	11.4	7.8				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+1), s	5.7	12.1	3.6	3.5	3.5	6.6	5.3	2.8				
Green Ext Time (p_c), s	0.1	3.2	0.0	0.1	0.0	1.9	0.3	0.1				

Intersection Summary

HCM 6th Ctrl Delay	18.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development PM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	81	502	66	98	706	367	58	37	140	351	53	70
Future Volume (veh/h)	81	502	66	98	706	367	58	37	140	351	53	70
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	84	518	65	101	728	0	60	38	0	362	55	39
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	124	924	116	133	1047		109	83		503	132	94
Arrive On Green	0.07	0.28	0.28	0.07	0.29	0.00	0.06	0.04	0.00	0.15	0.13	0.13
Sat Flow, veh/h	1847	3244	406	1818	3599	1683	1959	2057	1705	3445	1007	714
Grp Volume(v), veh/h	84	289	294	101	728	0	60	38	0	362	0	94
Grp Sat Flow(s),veh/h/ln	1847	1814	1836	1818	1800	1683	1959	2057	1705	1722	0	1721
Q Serve(g_s), s	2.3	7.1	7.2	2.9	9.5	0.0	1.6	1.0	0.0	5.3	0.0	2.6
Cycle Q Clear(g_c), s	2.3	7.1	7.2	2.9	9.5	0.0	1.6	1.0	0.0	5.3	0.0	2.6
Prop In Lane	1.00		0.22	1.00		1.00	1.00		1.00	1.00		0.41
Lane Grp Cap(c), veh/h	124	516	523	133	1047		109	83		503	0	226
V/C Ratio(X)	0.68	0.56	0.56	0.76	0.70		0.55	0.46		0.72	0.00	0.42
Avail Cap(c_a), veh/h	666	1859	1882	656	3689		521	1132		915	0	947
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.0	16.0	16.0	24.0	16.6	0.0	24.2	24.7	0.0	21.5	0.0	21.0
Incr Delay (d2), s/veh	2.4	0.4	0.4	3.3	0.4	0.0	1.6	1.4	0.0	0.7	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	2.5	2.6	1.2	3.3	0.0	0.7	0.5	0.0	2.0	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.4	16.5	16.5	27.3	17.0	0.0	25.9	26.2	0.0	22.2	0.0	21.5
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		667			829			98			456	
Approach Delay, s/veh		17.7			18.2			26.0			22.1	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.9	21.0	8.9	12.9	9.5	21.3	13.7	8.1				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	4.9	9.2	3.6	4.6	4.3	11.5	7.3	3.0				
Green Ext Time (p_c), s	0.1	2.3	0.0	0.3	0.1	3.5	0.4	0.1				

Intersection Summary

HCM 6th Ctrl Delay	19.3
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Opening Year 2024 With Development SAT Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	96	510	92	99	534	426	74	37	135	394	49	78
Future Volume (veh/h)	96	510	92	99	534	426	74	37	135	394	49	78
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	104	554	96	108	580	0	80	40	0	428	53	47
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	137	867	150	142	1012		126	86		567	130	115
Arrive On Green	0.07	0.28	0.28	0.08	0.28	0.00	0.06	0.04	0.00	0.16	0.14	0.14
Sat Flow, veh/h	1847	3141	543	1847	3628	1709	1959	2057	1743	3472	926	821
Grp Volume(v), veh/h	104	324	326	108	580	0	80	40	0	428	0	100
Grp Sat Flow(s),veh/h/ln	1847	1842	1842	1847	1814	1709	1959	2057	1743	1736	0	1746
Q Serve(g_s), s	3.0	8.4	8.5	3.1	7.5	0.0	2.2	1.0	0.0	6.4	0.0	2.8
Cycle Q Clear(g_c), s	3.0	8.4	8.5	3.1	7.5	0.0	2.2	1.0	0.0	6.4	0.0	2.8
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.47
Lane Grp Cap(c), veh/h	137	509	508	142	1012		126	86		567	0	245
V/C Ratio(X)	0.76	0.64	0.64	0.76	0.57		0.63	0.47		0.75	0.00	0.41
Avail Cap(c_a), veh/h	646	1831	1831	646	3606		505	1098		895	0	932
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.7	17.3	17.3	24.6	16.8	0.0	24.8	25.4	0.0	21.7	0.0	21.3
Incr Delay (d2), s/veh	3.2	0.6	0.6	3.1	0.2	0.0	1.9	1.5	0.0	0.8	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	3.1	3.1	1.3	2.6	0.0	1.0	0.5	0.0	2.5	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.9	17.9	17.9	27.7	17.0	0.0	26.7	26.9	0.0	22.5	0.0	21.7
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		754			688			120			528	
Approach Delay, s/veh		19.3			18.7			26.8			22.3	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.2	21.0	9.5	13.6	10.0	21.2	14.9	8.3				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	5.1	10.5	4.2	4.8	5.0	9.5	8.4	3.0				
Green Ext Time (p_c), s	0.1	2.6	0.1	0.3	0.1	2.7	0.5	0.1				

Intersection Summary

HCM 6th Ctrl Delay	20.3
HCM 6th LOS	C

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

7: Millers Run Road & Site Drive A
 Opening Year 2024 With Development AM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	754	446	5	0	4
Future Vol, veh/h	0	754	446	5	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	820	485	5	0	4

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	755
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.8
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	755
HCM Lane V/C Ratio	-	-	-	0.006
HCM Control Delay (s)	-	-	-	9.8
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

7: Millers Run Road & Site Drive A
 Opening Year 2024 With Development PM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	649	807	16	0	17
Future Vol, veh/h	0	649	807	16	0	17
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	2	2
Mvmt Flow	0	705	877	17	0	18

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	559
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.7
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	559
HCM Lane V/C Ratio	-	-	-	0.033
HCM Control Delay (s)	-	-	-	11.7
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

7: Millers Run Road & Site Drive A
 Opening Year 2024 With Development SAT Peak Hour Condition

10/05/2023

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↗
Traffic Vol, veh/h	0	698	652	18	0	15
Future Vol, veh/h	0	698	652	18	0	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	759	709	20	0	16

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	-	0	-	0	365
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	632
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	632
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	10.8
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	632
HCM Lane V/C Ratio	-	-	-	0.026
HCM Control Delay (s)	-	-	-	10.8
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

5: Millers Run Road & Site Drive B
 Opening Year 2024 With Development AM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh 0

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	754	448	12	0	3
Future Vol, veh/h	0	754	448	12	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	820	487	13	0	3

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	750
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.8
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR SBLn1
Capacity (veh/h)	-	-	750
HCM Lane V/C Ratio	-	-	0.004
HCM Control Delay (s)	-	-	9.8
HCM Lane LOS	-	-	A
HCM 95th %tile Q(veh)	-	-	0

5: Millers Run Road & Site Drive B
 Opening Year 2024 With Development PM Peak Hour Condition

10/05/2023

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↓			↑
Traffic Vol, veh/h	0	649	801	33	0	22
Future Vol, veh/h	0	649	801	33	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	0	2
Mvmt Flow	0	705	871	36	0	24

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	553
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.8
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR SBLn1
Capacity (veh/h)	-	-	553
HCM Lane V/C Ratio	-	-	0.043
HCM Control Delay (s)	-	-	11.8
HCM Lane LOS	-	-	B
HCM 95th %tile Q(veh)	-	-	0.1

5: Millers Run Road & Site Drive B
 Opening Year 2024 With Development SAT Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↗
Traffic Vol, veh/h	0	698	652	34	0	18
Future Vol, veh/h	0	698	652	34	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	759	709	37	0	20

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	624
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	624
HCM Lane V/C Ratio	-	-	-	0.031
HCM Control Delay (s)	-	-	-	11
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

10: Newbury Drive & Site Drive C/Plaza Access
 Opening Year 2024 With Development AM Peak Hour Condition

10/05/2023

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↕			↕		
Traffic Vol, veh/h	5	0	18	35	0	11	24	209	59	17	213	7
Future Vol, veh/h	5	0	18	35	0	11	24	209	59	17	213	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	3	2	0	2	5	0	0	2	2
Mvmt Flow	6	0	21	41	0	13	28	246	69	20	251	8

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	474	666	130	503	636	158	259	0	0	315	0	0
Stage 1	295	295	-	337	337	-	-	-	-	-	-	-
Stage 2	179	371	-	166	299	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.56	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.56	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.56	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.53	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	474	379	896	449	394	866	1303	-	-	1257	-	-
Stage 1	689	668	-	648	640	-	-	-	-	-	-	-
Stage 2	805	618	-	817	665	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	451	362	896	423	376	866	1303	-	-	1257	-	-
Mov Cap-2 Maneuver	451	362	-	423	376	-	-	-	-	-	-	-
Stage 1	671	655	-	631	623	-	-	-	-	-	-	-
Stage 2	772	602	-	783	652	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	10.1	13.4	0.7	0.7
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1303	-	-	738	482	1257	-	-
HCM Lane V/C Ratio	0.022	-	-	0.037	0.112	0.016	-	-
HCM Control Delay (s)	7.8	0.1	-	10.1	13.4	7.9	0.1	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.4	0	-	-

10: Newbury Drive & Site Drive C/Plaza Access
 Opening Year 2024 With Development PM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔			↔			↔			↔		
Traffic Vol, veh/h	23	0	57	55	2	28	45	358	62	13	365	19
Future Vol, veh/h	23	0	57	55	2	28	45	358	62	13	365	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	0	2	0	2	2	0	0	2	2
Mvmt Flow	25	0	62	60	2	30	49	389	67	14	397	21

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	730	990	209	748	967	228	418	0	0	456	0	0
Stage 1	436	436	-	521	521	-	-	-	-	-	-	-
Stage 2	294	554	-	227	446	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.5	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.5	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	310	245	797	305	253	781	1138	-	-	1115	-	-
Stage 1	569	578	-	512	530	-	-	-	-	-	-	-
Stage 2	690	512	-	761	572	-	-	-	-	-	-	-
Platoon blocked, %												
Mov Cap-1 Maneuver	279	227	797	266	235	781	1138	-	-	1115	-	-
Mov Cap-2 Maneuver	279	227	-	266	235	-	-	-	-	-	-	-
Stage 1	536	569	-	482	499	-	-	-	-	-	-	-
Stage 2	622	482	-	691	563	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	13.3	19.6	1	0.4
HCM LOS	B	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1138	-	-	520	338	1115	-	-
HCM Lane V/C Ratio	0.043	-	-	0.167	0.273	0.013	-	-
HCM Control Delay (s)	8.3	0.2	-	13.3	19.6	8.3	0.1	-
HCM Lane LOS	A	A	-	B	C	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.6	1.1	0	-	-

10: Newbury Drive & Site Drive C/Plaza Access
 Opening Year 2024 With Development SAT Peak Hour Condition

10/05/2023

Intersection												
Int Delay, s/veh	8.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔			↔			↔				↔	
Traffic Vol, veh/h	26	0	61	105	2	25	54	414	106	20	364	23
Future Vol, veh/h	26	0	61	105	2	25	54	414	106	20	364	23
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	1	2	4	2	0	2	0	1	2
Mvmt Flow	31	0	72	124	2	29	64	487	125	24	428	27

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	863	1230	228	940	1181	306	455	0	0	612	0	0
Stage 1	490	490	-	678	678	-	-	-	-	-	-	-
Stage 2	373	740	-	262	503	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.52	6.54	6.98	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.51	4.02	3.34	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	248	176	775	220	189	684	1102	-	-	977	-	-
Stage 1	529	547	-	411	450	-	-	-	-	-	-	-
Stage 2	620	421	-	723	540	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	213	155	775	181	166	684	1102	-	-	977	-	-
Mov Cap-2 Maneuver	213	155	-	181	166	-	-	-	-	-	-	-
Stage 1	481	529	-	374	410	-	-	-	-	-	-	-
Stage 2	537	383	-	634	522	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB		
HCM Control Delay, s	15.9		59.1		1		0.5		
HCM LOS	C		F						
























Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1102	-	-	433	210	977	-	-
HCM Lane V/C Ratio	0.058	-	-	0.236	0.739	0.024	-	-
HCM Control Delay (s)	8.5	0.3	-	15.9	59.1	8.8	0.1	-
HCM Lane LOS	A	A	-	C	F	A	A	-
HCM 95th %tile Q(veh)	0.2	-	-	0.9	4.9	0.1	-	-

APPENDIX P

Synchro Printouts – Design Year 2029 With Development Condition

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development AM Peak Hour Condition

10/11/2023

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	49	649	90	121	400	228	57	28	113	221	40	23
Future Volume (vph)	49	649	90	121	400	228	57	28	113	221	40	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frnt		0.982				0.850			0.850		0.946	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1692	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1639	3299	0	1778	3278	1613	1841	1700	1540	3416	1692	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13				251			124		18	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	5%	0%	2%	7%	4%	0%	14%	7%	2%	5%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	54	713	99	133	440	251	63	31	124	243	44	25
Shared Lane Traffic (%)												
Lane Group Flow (vph)	54	812	0	133	440	251	63	31	124	243	69	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development AM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	7.8	30.8		11.7	37.8	37.8	7.9	6.8	6.8	11.0	9.3	
Actuated g/C Ratio	0.09	0.36		0.14	0.44	0.44	0.09	0.08	0.08	0.13	0.11	
v/c Ratio	0.36	0.68		0.55	0.30	0.29	0.37	0.23	0.53	0.55	0.35	
Control Delay	48.8	26.1		47.0	17.1	3.2	48.1	47.3	17.5	43.2	37.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	48.8	26.1		47.0	17.1	3.2	48.1	47.3	17.5	43.2	37.0	
LOS	D	C		D	B	A	D	D	B	D	D	
Approach Delay		27.5			17.7			30.6			41.8	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	27	181		67	80	0	32	16	0	62	25	
Queue Length 95th (ft)	78	300		152	137	43	88	53	57	129	80	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	382	2193		415	2175	1154	316	605	628	587	614	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.14	0.37		0.32	0.20	0.22	0.20	0.05	0.20	0.41	0.11	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 85.5
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 26.2
 Intersection Capacity Utilization 55.5%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Ø1 25 s	Ø2 60 s	Ø3 20 s	Ø4 35 s
Ø5 25 s	Ø6 60 s	Ø7 20 s	Ø8 35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	84	528	66	98	740	384	58	37	140	366	53	73
Future Volume (vph)	84	528	66	98	740	384	58	37	140	366	53	73
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.983				0.850			0.850		0.913	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1676	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3387	0	1778	3405	1644	1841	1938	1599	3416	1676	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11				396			144		44	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	2%	3%	2%	0%	0%	3%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	87	544	68	101	763	396	60	38	144	377	55	75
Shared Lane Traffic (%)												
Lane Group Flow (vph)	87	612	0	101	763	396	60	38	144	377	130	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development PM Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	9.5	35.6		10.2	39.6	39.6	7.9	7.5	7.5	14.7	17.5	
Actuated g/C Ratio	0.10	0.38		0.11	0.43	0.43	0.08	0.08	0.08	0.16	0.19	
v/c Ratio	0.49	0.47		0.52	0.53	0.43	0.39	0.24	0.55	0.70	0.37	
Control Delay	53.9	21.9		53.6	21.9	3.5	53.3	49.4	16.9	49.0	32.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	53.9	21.9		53.6	21.9	3.5	53.3	49.4	16.9	49.0	32.7	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		25.9			18.6			31.0			44.8	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	49	130		56	172	0	34	22	0	109	46	
Queue Length 95th (ft)	117	213		131	274	56	89	61	61	#248	127	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	374	2062		380	2069	1154	289	632	619	538	576	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.23	0.30		0.27	0.37	0.34	0.21	0.06	0.23	0.70	0.23	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 93.1
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.70
 Intersection Signal Delay: 26.5
 Intersection Capacity Utilization 59.7%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.























Intersection LOS: C
 ICU Level of Service B

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Q1	Q2	Q3	Q4
25 s	60 s	20 s	35 s
Q5	Q6	Q7	Q8
25 s	60 s	20 s	35 s

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development SAT Peak Hour Condition

10/11/2023

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	99	537	92	99	559	446	74	37	135	411	49	81
Future Volume (vph)	99	537	92	99	559	446	74	37	135	411	49	81
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	12	11	13	12	12	12	12	12	12
Grade (%)		-1%			-1%			-4%			1%	
Storage Length (ft)	225		0	200		350	0		0	375		0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (ft)	75			75			75			75		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00
Ped Bike Factor												
Frt		0.978				0.850			0.850		0.906	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1754	3400	0	1814	3438	1677	1841	1938	1647	3450	1713	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1754	3400	0	1814	3438	1677	1841	1938	1647	3450	1713	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		16				485			147		54	
Link Speed (mph)		40			40			25			25	
Link Distance (ft)		472			644			295			539	
Travel Time (s)		8.0			11.0			8.0			14.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	108	584	100	108	608	485	80	40	147	447	53	88
Shared Lane Traffic (%)												
Lane Group Flow (vph)	108	684	0	108	608	485	80	40	147	447	141	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases						6			8			
Detector Phase	5	2		1	6	6	3	8	8	7	4	
Switch Phase												
Minimum Initial (s)	5.0	15.0		5.0	15.0	15.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	21.0		11.0	21.0	21.0	11.0	11.0	11.0	11.0	11.0	
Total Split (s)	25.0	60.0		25.0	60.0	60.0	20.0	35.0	35.0	20.0	35.0	
Total Split (%)	17.9%	42.9%		17.9%	42.9%	42.9%	14.3%	25.0%	25.0%	14.3%	25.0%	
Maximum Green (s)	19.0	54.0		19.0	54.0	54.0	14.0	29.0	29.0	14.0	29.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?												
Vehicle Extension (s)	2.0	2.1		2.0	2.1	2.1	2.0	2.0	2.0	2.0	2.0	

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development SAT Peak Hour Condition

10/11/2023

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Gap (s)	3.0	6.0		3.0	6.0	6.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	15.0		0.0	15.0	15.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	10.0		0.0	10.0	10.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	Min		None	Min	Min	None	None	None	None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	10.5	30.4		10.3	30.2	30.2	8.9	8.0	8.0	14.9	17.1	
Actuated g/C Ratio	0.12	0.34		0.12	0.34	0.34	0.10	0.09	0.09	0.17	0.19	
v/c Ratio	0.52	0.58		0.51	0.52	0.55	0.44	0.23	0.52	0.78	0.38	
Control Delay	51.1	24.8		50.9	24.5	4.5	51.3	46.7	15.1	49.8	30.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	51.1	24.8		50.9	24.5	4.5	51.3	46.7	15.1	49.8	30.0	
LOS	D	C		D	C	A	D	D	B	D	C	
Approach Delay		28.4			18.8			30.7			45.1	
Approach LOS		C			B			C			D	
Queue Length 50th (ft)	57	147		57	131	0	42	21	0	123	44	
Queue Length 95th (ft)	143	252		143	227	62	114	64	62	#328	134	
Internal Link Dist (ft)		392			564			215			459	
Turn Bay Length (ft)	225			200		350				375		
Base Capacity (vph)	397	2195		410	2212	1252	307	669	665	575	627	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.27	0.31		0.26	0.27	0.39	0.26	0.06	0.22	0.78	0.22	

Intersection Summary

Area Type: Other
 Cycle Length: 140
 Actuated Cycle Length: 89.1
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.78
 Intersection Signal Delay: 28.0
 Intersection Capacity Utilization 59.2%
 Analysis Period (min) 15
 Intersection LOS: C
 ICU Level of Service B

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Q1	Q2	Q3	Q4
25 s	60 s	20 s	35 s
Q5	Q6	Q7	Q8
25 s	60 s	20 s	35 s

APPENDIX Q

HCM Printouts – *Design Year 2029 With Development Condition*

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development AM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	49	649	90	121	400	228	57	28	113	221	40	23
Future Volume (veh/h)	49	649	90	121	400	228	57	28	113	221	40	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1834	1864	1939	1909	1834	1954	2057	1847	1952	1864	1820	1790
Adj Flow Rate, veh/h	54	713	99	133	440	0	63	31	0	243	44	10
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	7	5	0	2	7	4	0	14	7	2	5	7
Cap, veh/h	90	960	133	174	1225		112	64		373	123	28
Arrive On Green	0.05	0.31	0.31	0.10	0.35	0.00	0.06	0.03	0.00	0.11	0.09	0.09
Sat Flow, veh/h	1747	3124	433	1818	3485	1656	1959	1847	1654	3445	1435	326
Grp Volume(v), veh/h	54	404	408	133	440	0	63	31	0	243	0	54
Grp Sat Flow(s),veh/h/ln	1747	1771	1786	1818	1743	1656	1959	1847	1654	1722	0	1761
Q Serve(g_s), s	1.6	10.8	10.8	3.8	5.0	0.0	1.7	0.9	0.0	3.6	0.0	1.5
Cycle Q Clear(g_c), s	1.6	10.8	10.8	3.8	5.0	0.0	1.7	0.9	0.0	3.6	0.0	1.5
Prop In Lane	1.00		0.24	1.00		1.00	1.00		1.00	1.00		0.19
Lane Grp Cap(c), veh/h	90	545	549	174	1225		112	64		373	0	151
V/C Ratio(X)	0.60	0.74	0.74	0.76	0.36		0.56	0.49		0.65	0.00	0.36
Avail Cap(c_a), veh/h	628	1809	1825	654	3561		519	1014		912	0	966
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	16.4	16.4	23.3	12.7	0.0	24.3	25.1	0.0	22.6	0.0	22.8
Incr Delay (d2), s/veh	2.3	0.9	0.9	2.6	0.1	0.0	1.7	2.1	0.0	0.7	0.0	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	3.7	3.8	1.5	1.6	0.0	0.8	0.4	0.0	1.4	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.9	17.4	17.4	25.9	12.8	0.0	25.9	27.2	0.0	23.3	0.0	23.3
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		866			573			94			297	
Approach Delay, s/veh		17.9			15.9			26.3			23.3	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.1	22.3	9.0	10.5	8.7	24.6	11.7	7.8				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+1), s	5.8	12.8	3.7	3.5	3.6	7.0	5.6	2.9				
Green Ext Time (p_c), s	0.1	3.4	0.0	0.2	0.0	2.0	0.3	0.1				

Intersection Summary

HCM 6th Ctrl Delay	18.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development PM Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	84	528	66	98	740	384	58	37	140	366	53	73
Future Volume (veh/h)	84	528	66	98	740	384	58	37	140	366	53	73
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1909	1939	1909	1894	1986	2057	2057	2012	1864	1850	1850
Adj Flow Rate, veh/h	87	544	65	101	763	0	60	38	0	377	55	42
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	2	0	2	3	2	0	0	3	2	3	3
Cap, veh/h	125	949	113	132	1065		108	83		516	131	100
Arrive On Green	0.07	0.29	0.29	0.07	0.30	0.00	0.06	0.04	0.00	0.15	0.13	0.13
Sat Flow, veh/h	1847	3264	389	1818	3599	1683	1959	2057	1705	3445	973	743
Grp Volume(v), veh/h	87	302	307	101	763	0	60	38	0	377	0	97
Grp Sat Flow(s),veh/h/ln	1847	1814	1839	1818	1800	1683	1959	2057	1705	1722	0	1716
Q Serve(g_s), s	2.5	7.6	7.6	2.9	10.2	0.0	1.6	1.0	0.0	5.6	0.0	2.8
Cycle Q Clear(g_c), s	2.5	7.6	7.6	2.9	10.2	0.0	1.6	1.0	0.0	5.6	0.0	2.8
Prop In Lane	1.00		0.21	1.00		1.00	1.00		1.00	1.00		0.43
Lane Grp Cap(c), veh/h	125	527	535	132	1065		108	83		516	0	232
V/C Ratio(X)	0.70	0.57	0.57	0.76	0.72		0.56	0.46		0.73	0.00	0.42
Avail Cap(c_a), veh/h	653	1823	1848	643	3616		510	1110		897	0	926
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	16.2	16.2	24.5	16.9	0.0	24.8	25.2	0.0	21.8	0.0	21.3
Incr Delay (d2), s/veh	2.6	0.4	0.4	3.4	0.4	0.0	1.7	1.5	0.0	0.8	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	2.7	2.7	1.2	3.5	0.0	0.8	0.5	0.0	2.2	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.1	16.7	16.7	27.9	17.3	0.0	26.4	26.7	0.0	22.6	0.0	21.8
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		696			864			98			474	
Approach Delay, s/veh		18.0			18.6			26.5			22.4	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.9	21.6	9.0	13.3	9.6	21.9	14.0	8.2				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	4.9	9.6	3.6	4.8	4.5	12.2	7.6	3.0				
Green Ext Time (p_c), s	0.1	2.4	0.0	0.3	0.1	3.7	0.5	0.1				

Intersection Summary

HCM 6th Ctrl Delay	19.6
HCM 6th LOS	B

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

1: Todd A. Miller Drive/Newbury Drive & Millers Run Road
 Design Year 2029 With Development SAT Peak Hour Condition

10/05/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	99	537	92	99	559	446	74	37	135	411	49	81
Future Volume (veh/h)	99	537	92	99	559	446	74	37	135	411	49	81
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1939	1939	1849	1939	1909	2017	2057	2057	2057	1879	1894	1894
Adj Flow Rate, veh/h	108	584	96	108	608	0	80	40	0	447	53	50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	6	0	2	0	0	0	0	1	0	0
Cap, veh/h	142	869	142	142	995		126	86		585	131	123
Arrive On Green	0.08	0.27	0.27	0.08	0.27	0.00	0.06	0.04	0.00	0.17	0.15	0.15
Sat Flow, veh/h	1847	3169	520	1847	3628	1709	1959	2057	1743	3472	896	846
Grp Volume(v), veh/h	108	339	341	108	608	0	80	40	0	447	0	103
Grp Sat Flow(s),veh/h/ln	1847	1842	1846	1847	1814	1709	1959	2057	1743	1736	0	1742
Q Serve(g_s), s	3.1	9.0	9.0	3.1	8.0	0.0	2.2	1.0	0.0	6.7	0.0	2.9
Cycle Q Clear(g_c), s	3.1	9.0	9.0	3.1	8.0	0.0	2.2	1.0	0.0	6.7	0.0	2.9
Prop In Lane	1.00		0.28	1.00		1.00	1.00		1.00	1.00		0.49
Lane Grp Cap(c), veh/h	142	505	506	142	995		126	86		585	0	254
V/C Ratio(X)	0.76	0.67	0.67	0.76	0.61		0.64	0.47		0.76	0.00	0.41
Avail Cap(c_a), veh/h	641	1818	1822	641	3580		501	1090		888	0	923
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.7	17.7	17.7	24.7	17.3	0.0	25.0	25.6	0.0	21.7	0.0	21.2
Incr Delay (d2), s/veh	3.1	0.7	0.7	3.1	0.3	0.0	2.0	1.5	0.0	0.9	0.0	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	3.3	3.3	1.3	2.8	0.0	1.0	0.5	0.0	2.6	0.0	1.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.8	18.4	18.4	27.8	17.6	0.0	26.9	27.1	0.0	22.6	0.0	21.6
LnGrp LOS	C	B	B	C	B		C	C		C	A	C
Approach Vol, veh/h		788			716			120			550	
Approach Delay, s/veh		19.7			19.1			27.0			22.4	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.2	21.0	9.5	14.0	10.2	21.0	15.2	8.3				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	19.0	54.0	14.0	29.0	19.0	54.0	14.0	29.0				
Max Q Clear Time (g_c+l1), s	5.1	11.0	4.2	4.9	5.1	10.0	8.7	3.0				
Green Ext Time (p_c), s	0.1	2.7	0.1	0.4	0.1	2.8	0.5	0.1				

Intersection Summary

HCM 6th Ctrl Delay	20.6
HCM 6th LOS	C

Notes

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

7: Millers Run Road & Site Drive A
 Design Year 2029 With Development AM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	788	466	5	0	4
Future Vol, veh/h	0	788	466	5	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	857	507	5	0	4

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	743
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.9
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	743
HCM Lane V/C Ratio	-	-	-	0.006
HCM Control Delay (s)	-	-	-	9.9
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

7: Millers Run Road & Site Drive A
 Design Year 2029 With Development PM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↓			↑
Traffic Vol, veh/h	0	678	844	16	0	17
Future Vol, veh/h	0	678	844	16	0	17
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	2	2
Mvmt Flow	0	737	917	17	0	18

Major/Minor

	Major1	Major2	Minor2	
Conflicting Flow All	-	0	-	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	0
Stage 1	0	-	-	0
Stage 2	0	-	-	0
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	542
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach

	EB	WB	SB
HCM Control Delay, s	0	0	11.9
HCM LOS			B

Minor Lane/Major Mvmt

	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	542
HCM Lane V/C Ratio	-	-	-	0.034
HCM Control Delay (s)	-	-	-	11.9
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

7: Millers Run Road & Site Drive A
 Design Year 2029 With Development SAT Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	728	680	18	0	15
Future Vol, veh/h	0	728	680	18	0	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	791	739	20	0	16

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	-
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	618
HCM Lane V/C Ratio	-	-	-	0.026
HCM Control Delay (s)	-	-	-	11
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

5: Millers Run Road & Site Drive B
 Design Year 2029 With Development AM Peak Hour Condition

10/05/2023

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↓			↗
Traffic Vol, veh/h	0	788	468	12	0	3
Future Vol, veh/h	0	788	468	12	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	5	7	2	0	2
Mvmt Flow	0	857	509	13	0	3

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	- 0
Stage 1	-	-	- -
Stage 2	-	-	- -
Critical Hdwy	-	-	- 6.94
Critical Hdwy Stg 1	-	-	- -
Critical Hdwy Stg 2	-	-	- -
Follow-up Hdwy	-	-	- 3.32
Pot Cap-1 Maneuver	0	-	- 0 738
Stage 1	0	-	- 0 -
Stage 2	0	-	- 0 -
Platoon blocked, %	-	-	- -
Mov Cap-1 Maneuver	-	-	- - 738
Mov Cap-2 Maneuver	-	-	- - -
Stage 1	-	-	- - -
Stage 2	-	-	- - -

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.9
HCM LOS			A

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	738
HCM Lane V/C Ratio	-	-	-	0.004
HCM Control Delay (s)	-	-	-	9.9
HCM Lane LOS	-	-	-	A
HCM 95th %tile Q(veh)	-	-	-	0

5: Millers Run Road & Site Drive B
 Design Year 2029 With Development PM Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↗
Traffic Vol, veh/h	0	678	838	33	0	22
Future Vol, veh/h	0	678	838	33	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	3	2	0	2
Mvmt Flow	0	737	911	36	0	24

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	-
Pot Cap-1 Maneuver	0	-	-
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	12
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	537
HCM Lane V/C Ratio	-	-	-	0.045
HCM Control Delay (s)	-	-	-	12
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

5: Millers Run Road & Site Drive B
 Design Year 2029 With Development SAT Peak Hour Condition

10/05/2023

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↓			↗
Traffic Vol, veh/h	0	728	680	34	0	18
Future Vol, veh/h	0	728	680	34	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	-1	1	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	1	2	2	0	2
Mvmt Flow	0	791	739	37	0	20

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	0	-	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	611
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11.1
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	611
HCM Lane V/C Ratio	-	-	-	0.032
HCM Control Delay (s)	-	-	-	11.1
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

10: Newbury Drive & Site Drive C/Plaza Access
 Design Year 2029 With Development AM Peak Hour Condition

10/05/2023

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	5	0	18	35	0	11	24	219	59	17	223	7
Future Vol, veh/h	5	0	18	35	0	11	24	219	59	17	223	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	3	2	0	2	5	0	0	2	2
Mvmt Flow	6	0	21	41	0	13	28	258	69	20	262	8

Major/Minor	Minor2	Minor1	Major1	Major2
Conflicting Flow All	491	689	135	520
Stage 1	306	306	-	349
Stage 2	185	383	-	171
Critical Hdwy	7.54	6.54	6.94	7.56
Critical Hdwy Stg 1	6.54	5.54	-	6.56
Critical Hdwy Stg 2	6.54	5.54	-	6.56
Follow-up Hdwy	3.52	4.02	3.32	3.53
Pot Cap-1 Maneuver	461	367	889	437
Stage 1	679	660	-	638
Stage 2	799	610	-	811
Platoon blocked, %				
Mov Cap-1 Maneuver	438	350	889	412
Mov Cap-2 Maneuver	438	350	-	412
Stage 1	661	647	-	621
Stage 2	766	594	-	777

Approach	EB	WB	NB	SB
HCM Control Delay, s	10.2	13.7	0.7	0.6
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1290	-	-	726	470	1244	-	-
HCM Lane V/C Ratio	0.022	-	-	0.037	0.115	0.016	-	-
HCM Control Delay (s)	7.9	0.1	-	10.2	13.7	7.9	0.1	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.4	0	-	-

10: Newbury Drive & Site Drive C/Plaza Access
 Design Year 2029 With Development PM Peak Hour Condition

10/05/2023

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔			↔			↔			↔		
Traffic Vol, veh/h	23	0	57	55	2	28	45	375	62	13	382	19
Future Vol, veh/h	23	0	57	55	2	28	45	375	62	13	382	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	0	2	0	2	2	0	0	2	2
Mvmt Flow	25	0	62	60	2	30	49	408	67	14	415	21

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	757	1027	218	776	1004	238	436	0	0	475	0	0
Stage 1	454	454	-	540	540	-	-	-	-	-	-	-
Stage 2	303	573	-	236	464	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.5	6.54	6.9	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.5	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.5	4.02	3.3	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	297	233	786	291	240	769	1120	-	-	1098	-	-
Stage 1	555	568	-	499	519	-	-	-	-	-	-	-
Stage 2	681	502	-	752	562	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	267	215	786	253	222	769	1120	-	-	1098	-	-
Mov Cap-2 Maneuver	267	215	-	253	222	-	-	-	-	-	-	-
Stage 1	522	558	-	469	488	-	-	-	-	-	-	-
Stage 2	612	472	-	681	552	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	13.6	20.6	0.9	0.4
HCM LOS	B	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1120	-	-	504	323	1098	-	-
HCM Lane V/C Ratio	0.044	-	-	0.173	0.286	0.013	-	-
HCM Control Delay (s)	8.4	0.2	-	13.6	20.6	8.3	0.1	-
HCM Lane LOS	A	A	-	B	C	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.6	1.2	0	-	-

10: Newbury Drive & Site Drive C/Plaza Access
 Design Year 2029 With Development SAT Peak Hour Condition

10/05/2023

Intersection												
Int Delay, s/veh	9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	26	0	61	105	2	25	54	432	106	20	380	23
Future Vol, veh/h	26	0	61	105	2	25	54	432	106	20	380	23
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	-1	-	-	1	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	2	2	2	1	2	4	2	0	2	0	1	2
Mvmt Flow	31	0	72	124	2	29	64	508	125	24	447	27

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	892	1270	237	971	1221	317	474	0	0	633	0	0
Stage 1	509	509	-	699	699	-	-	-	-	-	-	-
Stage 2	383	761	-	272	522	-	-	-	-	-	-	-
Critical Hdwy	7.54	6.54	6.94	7.52	6.54	6.98	4.14	-	-	4.1	-	-
Critical Hdwy Stg 1	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.54	5.54	-	6.52	5.54	-	-	-	-	-	-	-
Follow-up Hdwy	3.52	4.02	3.32	3.51	4.02	3.34	2.22	-	-	2.2	-	-
Pot Cap-1 Maneuver	237	167	764	209	179	673	1084	-	-	960	-	-
Stage 1	515	536	-	399	440	-	-	-	-	-	-	-
Stage 2	611	412	-	713	529	-	-	-	-	-	-	-
Platoon blocked, %												
Mov Cap-1 Maneuver	203	146	764	171	157	673	1084	-	-	960	-	-
Mov Cap-2 Maneuver	203	146	-	171	157	-	-	-	-	-	-	-
Stage 1	467	518	-	362	399	-	-	-	-	-	-	-
Stage 2	527	374	-	624	511	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	16.4	67.5	1	0.5
HCM LOS	C	F		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1084	-	-	418	199	960	-	-
HCM Lane V/C Ratio	0.059	-	-	0.245	0.78	0.025	-	-
HCM Control Delay (s)	8.5	0.3	-	16.4	67.5	8.8	0.1	-
HCM Lane LOS	A	A	-	C	F	A	A	-
HCM 95th %tile Q(veh)	0.2	-	-	0.9	5.4	0.1	-	-

APPENDIX R

Radar Speed Study



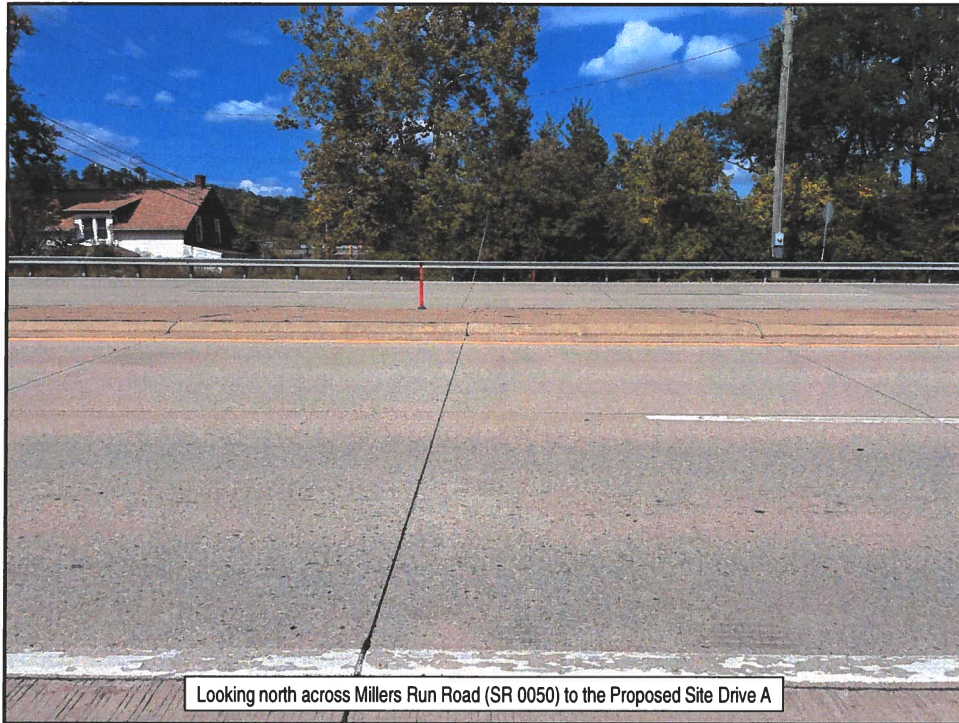
Weather: Clouds & Sun/ 70's
Study By: J.K. (Spot Radar Speed)
Road: Newbury Dr. NB/SB approaches
Day: Wednesday, May 20, 2023 (3:30 pm)

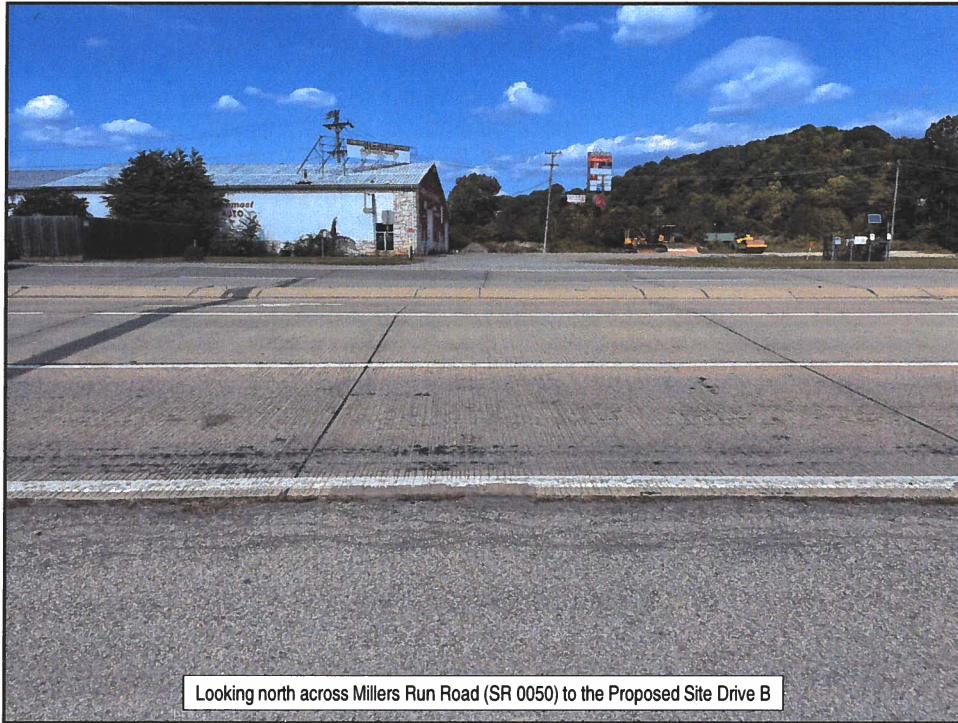
#	SB app	NB app
1	27	22
2	23	22
3	27	17
4	30	20
5	23	23
6	24	22
7	34	22
8	26	17
9	27	26
10	25	22
11	28	24
12	26	21
13	23	25
14	27	24
15	28	17
16	30	20
17	21	24
18	20	19
19	25	22
20	23	21
21	30	27
22	23	18
23	24	27
24	28	21
25	23	23
26		

Class	Vehicle Count	Average Speed	85 Percentile
SB app	25	26	28
NB app	25	22	24
Summary	50	24	27

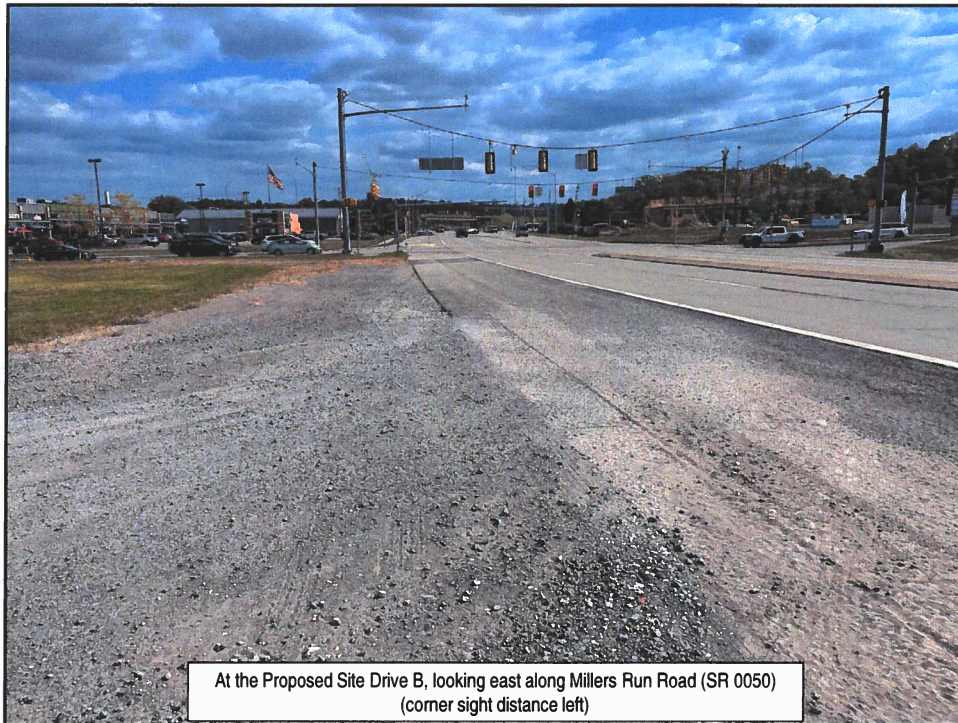
APPENDIX S

Sight Distance Photo Log

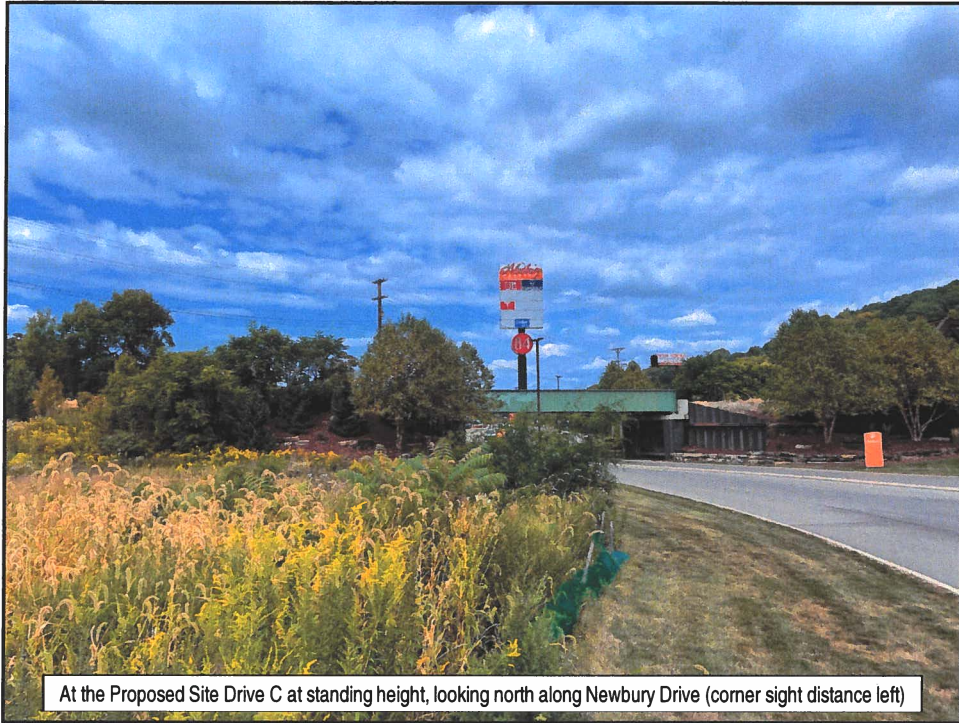




Looking north across Millers Run Road (SR 0050) to the Proposed Site Drive B



At the Proposed Site Drive B, looking east along Millers Run Road (SR 0050)
(corner sight distance left)



At the Proposed Site Drive C at standing height, looking north along Newbury Drive (corner sight distance left)



At the Proposed Site Drive C, looking south along Newbury Drive (corner sight distance right)



Forward view of a northbound, left turning vehicle from Newbury Drive into the Proposed Site Drive C (stopping sight distance ahead)



Rear view of a northbound, left turning vehicle from Newbury Drive into the Proposed Site Drive C (stopping sight distance behind)

APPENDIX T

SimTraffic Printouts – *Design Year 2029 Without Development Condition*

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	61	207	195	132	141	86	16	97	82	59	146	167
Average Queue (ft)	21	127	96	66	68	33	1	39	26	7	51	86
95th Queue (ft)	50	194	171	113	125	74	11	74	64	37	115	142
Link Distance (ft)		1063	1063		591	591		249	249	249		451
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0										
Queuing Penalty (veh)		0										

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	118
Average Queue (ft)	47
95th Queue (ft)	91
Link Distance (ft)	451
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Newbury Drive & Plaza Access

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	56	4	48
Average Queue (ft)	18	0	6
95th Queue (ft)	39	3	30
Link Distance (ft)	284	451	396
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 0

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	86	187	157	138	234	182	51	106	88	80	171	201
Average Queue (ft)	36	111	67	66	129	80	5	44	31	10	85	119
95th Queue (ft)	73	170	135	121	202	159	32	87	69	48	154	181
Link Distance (ft)		1063	1063		591	591		249	249	249		451
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0			1							
Queuing Penalty (veh)		0			1							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	151
Average Queue (ft)	61
95th Queue (ft)	116
Link Distance (ft)	451
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Newbury Drive & Plaza Access

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	89	66
Average Queue (ft)	28	8
95th Queue (ft)	60	37
Link Distance (ft)	284	396
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 1

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	90	189	173	125	201	156	73	110	104	65	177	208
Average Queue (ft)	41	109	83	57	110	57	11	50	31	5	87	120
95th Queue (ft)	79	169	147	104	178	124	49	91	72	34	155	180
Link Distance (ft)		1063	1063		591	591		249	249	249		451
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)					0							
Queuing Penalty (veh)					0							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	141
Average Queue (ft)	65
95th Queue (ft)	116
Link Distance (ft)	451
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Newbury Drive & Plaza Access

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	115	22	78
Average Queue (ft)	48	1	13
95th Queue (ft)	92	9	51
Link Distance (ft)	284	451	396
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 0

APPENDIX U

SimTraffic Printouts – *Design Year 2029 With Development Condition*

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	108	214	209	151	163	140	49	84	85	38	120	144
Average Queue (ft)	37	127	103	69	72	39	2	37	28	4	50	84
95th Queue (ft)	81	195	172	126	133	95	25	72	69	29	110	134
Link Distance (ft)		399	399		593	593		249	249	249		454
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0		0	0							
Queuing Penalty (veh)		0		0	0							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	100
Average Queue (ft)	43
95th Queue (ft)	81
Link Distance (ft)	454
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Millers Run Road & Site Drive B

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 7: Millers Run Road & Site Drive A

Movement

Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 10: Newbury Drive & Site Drive C/Plaza Access

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LT	LT
Maximum Queue (ft)	13	54	31	38
Average Queue (ft)	7	23	5	4
95th Queue (ft)	17	49	24	20
Link Distance (ft)	210	298	454	363
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	110	185	171	137	229	195	82	116	77	56	189	206
Average Queue (ft)	53	107	85	62	141	95	11	44	33	9	102	135
95th Queue (ft)	99	163	148	112	217	178	52	86	67	43	171	198
Link Distance (ft)		399	399		593	593		249	249	249		454
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)					1							
Queuing Penalty (veh)					1							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	171
Average Queue (ft)	66
95th Queue (ft)	124
Link Distance (ft)	454
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Millers Run Road & Site Drive B

Movement	SB
Directions Served	R
Maximum Queue (ft)	16
Average Queue (ft)	1
95th Queue (ft)	8
Link Distance (ft)	223
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 7: Millers Run Road & Site Drive A

Movement	SB
Directions Served	R
Maximum Queue (ft)	13
Average Queue (ft)	0
95th Queue (ft)	7
Link Distance (ft)	232
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: Newbury Drive & Site Drive C/Plaza Access

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	LT	TR	LT	TR
Maximum Queue (ft)	51	89	52	4	47	7
Average Queue (ft)	18	35	16	0	4	0
95th Queue (ft)	39	68	44	3	23	3
Link Distance (ft)	210	298	454	454	363	363
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Network Summary

Network wide Queuing Penalty: 1

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	T	TR	L	T	T	R	L	T	R	L	L
Maximum Queue (ft)	137	206	182	129	206	159	89	103	82	64	197	230
Average Queue (ft)	62	104	80	55	115	76	14	49	32	9	107	134
95th Queue (ft)	109	163	142	103	188	146	58	91	70	44	176	202
Link Distance (ft)		399	399		593	593		249	249	249		454
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225			200			350				375	
Storage Blk Time (%)		0			0							
Queuing Penalty (veh)		0			0							

Intersection: 1: Todd A. Miller Drive/Newbury Drive & Millers Run Road

Movement	SB
Directions Served	TR
Maximum Queue (ft)	143
Average Queue (ft)	62
95th Queue (ft)	118
Link Distance (ft)	454
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Millers Run Road & Site Drive B

Movement	SB
Directions Served	R
Maximum Queue (ft)	17
Average Queue (ft)	1
95th Queue (ft)	9
Link Distance (ft)	223
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 7: Millers Run Road & Site Drive A

Movement	SB
Directions Served	R
Maximum Queue (ft)	7
Average Queue (ft)	0
95th Queue (ft)	5
Link Distance (ft)	232
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 10: Newbury Drive & Site Drive C/Plaza Access

Movement	EB	WB	NB	NB	SB	SB
Directions Served	LTR	LTR	LT	TR	LT	TR
Maximum Queue (ft)	68	204	56	9	65	17
Average Queue (ft)	21	66	20	0	9	1
95th Queue (ft)	47	143	51	5	37	6
Link Distance (ft)	210	298	454	454	363	363
Upstream Blk Time (%)		0				
Queuing Penalty (veh)		0				
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Network Summary

Network wide Queuing Penalty: 0

**CRASH DATA
APPENDIX**

for the proposed

**RETAIL
DEVELOPMENT**

South Fayette Township, Allegheny County, Pennsylvania

October 17, 2023

CRASH DATA APPENDIX

for the proposed

RETAIL DEVELOPMENT

South Fayette Township, Allegheny County, PA

October 17, 2023

Prepared for: **Cozza Enterprises, LLC**
PO Box 453
Carnegie, PA 15106

Prepared by: **David E. Wooster and Associates, LLC**
Two East Crafton Avenue
Pittsburgh, PA 15205

Project Engineer(s): **Jesse Nelson & Suleiman A. Swai, P.E.**

Supervising Engineer: **Joshua A. Haydo, P.E., PTOE**

Confidential – Traffic Engineering and Safety Study

This document is the property of the Commonwealth of Pennsylvania, Department of Transportation. The data and information contained herein are part of a traffic engineering and safety study. This safety study is only provided to those official agencies or persons who have responsibility in the highway transportation system and may only be used by such agencies or persons for traffic safety-related planning or research. The document and information are confidential pursuant to 75 Pa. C.S. §3754 and 23 U.S.C. §409 and may not be published, reproduced, released or discussed without the written permission of the Pennsylvania Department of Transportation.

**Crash Data Appendix
Proposed Retail Development
South Fayette Township, Allegheny County, Pennsylvania**

1.0 PROJECT DESCRIPTION

The proposed project is located on the northwestern corner of the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive in South Fayette Township, Allegheny County, Pennsylvania. The development is proposed to consist of ~45,126-square foot of retail space.

The study area for this project includes two (2) existing intersections:

- Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive
- Newbury Drive with Plaza Access

2.0 OBTAINING CRASH DATA

Copies of crash data summaries for the five (5) most recent calendar years were obtained from the Pennsylvania Department of Transportation (PennDOT) Pennsylvania Crash Information Tool (PCIT) for the existing study intersections.

As the crash data is property of PennDOT, a summary of the identified crashes and the corresponding crash reports have been provided in this separately-bound appendix.

3.0 CRASH DATA SUMMARY

3.1 Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive

At the intersection of Millers Run Road (SR 0050) with Newbury Drive / Todd A. Miller Drive, eight (8) reportable crashes occurred between January 1, 2018 and December 31, 2022. Of the crashes, three (3) occurred in 2020, two (2) occurred in 2021, and three (3) occurred in 2022. Of the crashes four (4) were rear-end collisions, two (2) were opposite direction side-swipe collisions, one (1) involved a motorist striking a curb, and one (1) involved a motorist striking a traffic island/pole. None of the crashes involved fatalities.

3.2 Newbury Drive with Plaza Access

PCIT does not contain crash information for the intersection of Newbury Drive with Plaza Access.

4.0 SUMMARY / CONCLUSIONS

Based on a review of the crash data, five or more crashes of types susceptible to correction by traffic control or geometric improvements were not reported within a twelve-month period at any of the study intersections.

Copies of the crash data provided by PennDOT have been included in the Enclosures section of this appendix.

PENNDOT CRASH DATA

Pennsylvania Crash Information Tool

Millers Run Road (SR 0050) with Newbury Drive

Date Range: 01/01/2018 to 12/31/2022

USER ID / QUERY ID:
b-sswai / 0320231006133



MONTH OF YEAR								DAY OF WEEK					
	JAN	FEB	APR	MAY	JUN	AUG	NOV	TOTAL	SUN	MON	TUE	FRI	TOTAL
CRASHES	1	1	1	1	1	1	2	8	1	4	2	1	8
PCT	13%	13%	13%	13%	13%	13%	25%	100%	13%	50%	25%	13%	100%

HOUR OF DAY									
	05	07	08	13	16	17	18	19	TOTAL
CRASHES	1	1	1	1	1	1	1	1	8
PCT	13%	13%	13%	13%	13%	13%	13%	13%	100%

YEAR	CRASHES	PCT
2020	3	38%
2021	2	25%
2022	3	38%
TOTAL	8	100%

COLLISION TYPE		
	CRASHES	PCT
REAR END	4	50%
HIT FIX OBJ	2	25%
OPP DIR SS	2	25%
TOTAL	8	100%

CRASH SEVERITY LEVEL		
	CRASHES	PCT
POSSIBLE INJURY	1	13%
UNK IF INJURED	1	13%
PDO	6	75%
TOTAL	8	100%

SEVERITY COUNT	
	PERSONS
FATALITIES	0
SUSPECTED SERIOUS	0
SUSPECTED MINOR	0
POSSIBLE INJURY	1
UNK SEVERITY	0
UNK IF INJURED	2

DRIVER ACTIONS		
	ACTIONS	PCT
NO CONTRIBUTING ACTION	7	39%
DRIVER WAS DISTRACTED	5	28%
FAILURE TO RESPOND TCD	1	6%
IMPROPER/CARELESS TURN	1	6%
OTHER IMPROPER DRIVING	1	6%
RUNNING RED LIGHT	1	6%
SUDDEN SLOWING/STOP	1	6%
TAILGATING	1	6%
TOTAL	18	100%

VEHICLE TYPE		
	VEHICLES	PCT
AUTOMOBILE	6	38%
SUV	4	25%
SMALL TRUCK	3	19%
VAN	2	13%
BUS	1	6%
TOTAL	16	100%

ROAD CONDITION		
	CRASHES	PCT
DRY	7	88%
WET	1	13%
TOTAL	8	100%

ILLUMINATION		
	CRASHES	PCT
DAYLIGHT	6	75%
STREET LIGHTS	2	25%
TOTAL	8	100%

WEATHER		
	CRASHES	PCT
CLEAR	7	88%
RAIN	1	13%
TOTAL	8	100%

ENVIR/ROADWAY FACTORS		
	FACTORS	PCT
NONE	7	88%
OTHER WEATHER COND	1	13%
TOTAL	8	100%

IMPORTANT: This traffic engineering and safety study is confidential pursuant to 75 Pa. C.S. §3754 and 23 U.S.C. §407 and may not be disclosed or used in litigation without written permission from PennDOT.

NOTES:1 Injury Severity Disclaimer

Please note that beginning January 1, 2016, PennDOT adopted the Federal standard for collecting injury severity data. The field descriptions and definitions changed from the state standard that had been in use for decades. This resulted in a substantial shift in severity levels. Therefore, comparison of the "Suspected Serious Injury", "Suspected Minor Injury" and "Possible Injury" categories will not be consistent for crashes taking place before versus after the adoption of the new standard.

REPORT PARAMETERS:

Query ID: 0320231006133

User ID: b-sswai

Title: Millers Run Road (SR 0050)

Date Range: 01/01/2018 to 12/31/2022

Filter Characteristics:

Selected Shapes : NO NAME RD x MILLERS RUN RD,NO NAME RD x MILLERS RUN RD - Buffer (250 feet)

This report counts the number of crashes.



Date Range: 01/01/2018 to 12/31/2022*

CRASH SEVERITY LEVEL BY YEAR

	2020 CRASHES	2021 CRASHES	2022 CRASHES	ALL YEARS CRASHES
POSSIBLE INJURY	1	0	0	1
UNKNOWN IF INJURED	0	0	1	1
PROPERTY DMG ONLY	2	2	2	6
TOTAL	3	2	3	8

CRASH DESCRIPTION TYPES BY YEAR

	2020 CRASHES	2021 CRASHES	2022 CRASHES	ALL YEARS CRASHES
HIT FIXED OBJECT	0	1	1	2
OPP DIRECTION SIDESWIPE	2	0	0	2
REAR END	1	1	2	4
TOTAL	3	2	3	8

PERSON INJURY SUMMARY BY YEAR

	2020 PERSONS	2021 PERSONS	2022 PERSONS	ALL YEARS PERSONS
FATALITIES	0	0	0	0
SUSPECTED SERIOUS INJURIES	0	0	0	0
SUSPECTED MINOR INJURIES	0	0	0	0
POSSIBLE INJURIES	1	0	0	1
UNKNOWN SEVERITY	0	0	0	0
UNKNOWN IF INJURED	0	0	2	2

* **PLEASE NOTE:** Years which do not appear in the report contain zero crashes for this request.

* Complete records of reportable crashes are available in PCIT for the following years: 2003 - 2022

* Crash information for 2023 is incomplete at the time of this printing. As such, data for 2023 is not included in this report.

IMPORTANT: The information contained in this document is drawn from raw data and should not be interpreted as representing an engineering judgement or determination made by the Department of Transportation as to the type and severity of accidents noted herein.

Print Date: 10/06/2023

Pennsylvania Crash Information Tool

Print Date: 10/06/2023

PCIT - PUBLIC REQUEST / PRESS INQUIRY REPORT (01-07)

NOTES:

1 Injury Severity Disclaimer

Please note that beginning January 1, 2016, PennDOT adopted the Federal standard for collecting injury severity data. The field descriptions and definitions changed from the state standard that had been in use for decades. This resulted in a substantial shift in severity levels. Therefore, comparison of the "Suspected Serious Injury", "Suspected Minor Injury" and "Possible Injury" categories will not be consistent for crashes taking place before versus after the adoption of the new standard.

REPORT PARAMETERS:

Query ID: 0320231006134

User ID: b-sswai

Title: Millers Run Road (SR 0050)

Date Range : 01/01/2018 to 12/31/2022

Selected Shapes : NO NAME RD x MILLERS RUN RD,NO NAME RD x MILLERS RUN RD - Buffer (250 feet)

Filter Characteristics:

This report counts the number of crashes.

Pennsylvania Crash Information Tool

Millers Run Road (SR 0050) with Newbury Drive

Sorted by Crash Date

Date Range: 01/01/2018 to 12/31/2022

USER ID / QUERY ID:

b-sswai / 0320231006135



CRN	CO	DATE	DAY	TIME	LIGHTING	ROAD SURF	WEATHER	FAT	INJ	PED	VEH	MAX SEVERITY
1	<u>2020004800</u>	02	01/13/2020	MON	08:08	DAYLIGHT	DRY CLEAR	0	0	0	2	PROP DMG ONLY OPP DIR SIDESW
ENV RDWY FACTORS: NONE 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR VEH: 1 BUS TRAVELING EAST IN RIGHT OF TRAFFICWAY TURNING RIGHT VEH EVENTS: HIT UNIT 02 DVR ACTIONS: OTHER IMPROPER DRIV ACTIONS VEH: 2 VAN TRAVELING EAST IN RIGHT LANE GOING STRAIGHT VEH EVENTS: STRUCK BY UNIT 01 DVR ACTIONS: NO CONTRIBUTING ACTION												
2	<u>2020097116</u>	02	11/13/2020	FRI	18:02	STREET LT	DRY CLEAR	0	0	0	2	PROP DMG ONLY REAR-END
ENV RDWY FACTORS: NONE 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR VEH: 1 SMALL TRUCK TRAVELING WEST IN LEFT LANE GOING STRAIGHT VEH EVENTS: HIT UNIT 02 DVR ACTIONS: TAILGATING DRIVER WAS DISTRACTED VEH: 2 SMALL TRUCK TRAVELING WEST IN LEFT LANE GOING STRAIGHT VEH EVENTS: STRUCK BY UNIT 01 DVR ACTIONS: NO CONTRIBUTING ACTION												
3	<u>2020098335</u>	02	11/17/2020	TUE	07:40	DAYLIGHT	WET RAIN	0	1	0	2	POSSIBLE INJURY OPP DIR SIDESW
ENV RDWY FACTORS: OTHER WEATHER CONDITIONS 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR VEH: 1 SUV TRAVELING WEST IN ONCOMING TRAFFIC LANE TURNING LEFT VEH EVENTS: HIT UNIT 02 DVR ACTIONS: NO CONTRIBUTING ACTION VEH: 2 SUV TRAVELING EAST IN LEFT LANE GOING STRAIGHT VEH EVENTS: STRUCK BY UNIT 01 DVR ACTIONS: RUNNING RED LIGHT												
4	<u>2021041821</u>	02	05/09/2021	SUN	05:01	STREET LT	DRY CLEAR	0	0	0	1	PROP DMG ONLY HIT FIXED OBJ
ENV RDWY FACTORS: NONE 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR VEH: 1 AUTOMOBILE TRAVELING EAST IN RIGHT LANE TURNING RIGHT VEH EVENTS: HIT CURB DVR ACTIONS: IMPROPER/CARELESS TURN												
5	<u>2021059885</u>	02	06/28/2021	MON	13:06	DAYLIGHT	DRY CLEAR	0	0	0	2	PROP DMG ONLY REAR-END
ENV RDWY FACTORS: NONE T-INT 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR VEH: 1 AUTOMOBILE TRAVELING WEST IN RIGHT TURN LANE TURNING RIGHT VEH EVENTS: HIT UNIT 02 DVR ACTIONS: DRIVER WAS DISTRACTED VEH: 2 AUTOMOBILE TRAVELING WEST IN RIGHT TURN LANE TURNING RIGHT VEH EVENTS: STRUCK BY UNIT 01 DVR ACTIONS: NO CONTRIBUTING ACTION												

Pennsylvania Crash Information Tool

Millers Run Road (SR 0050) with Newbury Drive

Sorted by Crash Date

Date Range: 01/01/2018 to 12/31/2022

USER ID / QUERY ID:

b-sswai / 0320231006135



CRN	CO	DATE	DAY	TIME	LIGHTING	ROAD SURF	WEATHER	FAT	INJ	PED	VEH	MAX SEVERITY
6	<u>2022015432</u>	02 02/07/2022	MON	16:15	DAYLIGHT	DRY	CLEAR	0	0	0	1	PROP DMG ONLY
ENV RDWY FACTORS: NONE 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR HIT FIXED OBJ VEH: 1 SMALL TRUCK TRAVELING WEST IN RIGHT LANE GOING STRAIGHT VEH EVENTS: HIT TRAFFIC ISLAND / CHANNELIZATOTHER POST, POLE, OR SUPPORT DVR ACTIONS: DRIVER WAS DISTRACTED												
7	<u>2022032948</u>	02 04/04/2022	MON	19:20	DAYLIGHT	DRY	CLEAR	0	0	0	2	UNK IF INJURED
ENV RDWY FACTORS: NONE 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR REAR-END VEH: 1 SUV TRAVELING EAST IN RIGHT LANE GOING STRAIGHT VEH EVENTS: HIT UNIT 02 DVR ACTIONS: DRIVER WAS DISTRACTED FAILURE TO RESPOND TO TCD VEH: 2 VAN TRAVELING EAST IN RIGHT LANE SLOWING OR STOPPING IN LANE VEH EVENTS: STRUCK BY UNIT 01 DVR ACTIONS: SUDDEN SLOWING / STOPPING												
8	<u>2022079619</u>	02 08/30/2022	TUE	17:40	DAYLIGHT	DRY	CLEAR	0	0	0	4	PROP DMG ONLY
ENV RDWY FACTORS: NONE 4WAY 0050/0090/1242 0050/0091/1312 NEWBURY DR TODD A MILLER DR REAR-END VEH: 1 AUTOMOBILE TRAVELING EAST IN OTHER FWD MOVING LANE GOING STRAIGHT VEH EVENTS: HIT UNIT 02 DVR ACTIONS: DRIVER WAS DISTRACTED VEH: 2 AUTOMOBILE TRAVELING EAST IN OTHER FWD MOVING LANE STOPPED IN TRAFFIC LANE VEH EVENTS: HIT UNIT 03 DVR ACTIONS: NO CONTRIBUTING ACTION VEH: 3 AUTOMOBILE TRAVELING EAST IN OTHER FWD MOVING LANE STOPPED IN TRAFFIC LANE VEH EVENTS: HIT UNIT 04 DVR ACTIONS: NO CONTRIBUTING ACTION VEH: 4 SUV TRAVELING EAST IN OTHER FWD MOVING LANE STOPPED IN TRAFFIC LANE VEH EVENTS: STRUCK BY UNIT 03 DVR ACTIONS: NO CONTRIBUTING ACTION												

Pennsylvania Crash Information Tool

Millers Run Road (SR 0050) with Newbury Drive

Sorted by Crash Date

NOTES:

- 1 **Injury Severity Disclaimer**
Please note that beginning January 1, 2016, PennDOT adopted the Federal standard for collecting injury severity data. The field descriptions and definitions changed from the state standard that had been in use for decades. This resulted in a substantial shift in severity levels. Therefore, comparison of the "Suspected Serious Injury", "Suspected Minor Injury" and "Possible Injury" categories will not be consistent for crashes taking place before versus after the adoption of the new standard.

REPORT PARAMETERS:

Query ID: 0320231006135

User ID: b-sswai

Title: Millers Run Road (SR 0050)

Date Range: 01/01/2018 to 12/31/2022

Selected Shapes: NO NAME RD x MILLERS RUN RD,NO NAME RD x MILLERS RUN RD - Buffer (250 feet)

Filter Characteristics:

This report counts the number of crashes.

January 14, 2022
C-12199-0020

Cozza Enterprises, LLC
P.O. Box 453
Carnegie, PA 15106

Attn: Craig Cozza

Subject: Geotechnical Engineering Review
Proposed Building Addition
180 Millers Run Road, South Fayette Township
Allegheny County, Pennsylvania

As authorized, we have completed our review of subsurface conditions at the above-referenced site. This report describes the findings of the review and presents recommendations for the design and construction of foundations to support the proposed building addition.

SITE LOCATION AND CONDITIONS

The subject site consists of Lot 256-L-2 as reported on the Allegheny County Real Estate website, and is situated north of Millers Run Road, north of its intersection with Newbury Drive, in South Fayette Township, Allegheny County, Pennsylvania. It is mostly covered with stone and vegetation around an existing single-story building. The ground surface in the area investigated generally plateaued at about 830 feet, as as estimated from Google Earth. The elevations referenced appear to be based on North American Vertical Datum of 1988 (NAVD88).

GEOLOGIC RECONNAISSANCE

Geologically, the site is located on the east flank of the south-trending Carnegie Syncline (a trough-like fold in the bedrock strata) about 0.4 miles from its axis. Surficial bedrock strata at the site dip, and groundwater in the bedrock migrates, gently south on a grade of about 1.2 percent (1.2 ft in vertical rise in 100 feet horizontal distance).

The Pennsylvania Department of Environmental Resources, Mineral Resource Report 89, Coal Resources of Allegheny County, Pennsylvania, Part 1, dated 1986, indicates that the base of the Pittsburgh Coal seam horizon - a significant marker bed in this locale - lies at about elev 800, or at least 30 feet below the site, and has been deep mined south of Millers Run Road and the subject site. In addition, mining maps available on the Pennsylvania Mine Map Portal (minemaps.psu.edu/) of the U.S. Steel National Mine No. 1 (attached), indicate the subject site is north of the old alignment of Millers Run Road and the mine workings. These resources do not indicate any other coal seams below the site. Therefore, mine subsidence is not a factor for this project.

Bedrock at the site represents the Casselman Formation, Conemaugh Group, Pennsylvanian System, consisting of sandstones and shales, with some limestone and coal seams. This formation is overlain by alluvial soil deposited by Millers Run and Chartiers Creek.

AERIAL AND USGS QUADRANGLE RECONNAISSANCE

Based on the review of historical aerials dated back to 1949 and topographic maps dated back to 1904, the subject site has existed at the current surface elevation since 1904. A structure was first built by the late 1940s, as 1940 and 1944 updates to the 1904 quadrangle illustrate no structure on the north side of Millers Run until that time. "Scott's Ponds", which are the surface ponds indicated on the U.S. Steel mine mapping were backfilled starting in the late 1940s. This is notable, as the ponds are a consistent physical barrier between the mine limits and the subject site. USGS mapping from 1904 and 1953 are attached.

DISCUSSION AND RECOMMENDATIONS

Based on our understanding, the project will consist of the construction of an addition to the existing building on the property which will require foundation and floor slab construction. No cut or filling for slopes will be performed.

FOUNDATION BEARING CAPACITY AND SETTLEMENT CONSIDERATIONS

Based on our review of the site, there may exist thick variable layers of imported fill and alluvial soil. These materials may be of variable density and compressibility, which can negatively impact the performance of the proposed foundations and floor slab. Therefore, prior to foundation and floor slab construction, at least four test pits should be performed within the proposed footprint of the building addition. The test pits should extend to a depth of 10 feet or bedrock, whichever first occurs. The test pits should be observed and logged by a licensed professional geotechnical engineer or their representative. The engineer should compare and contrast the test pit information and the considerations provided below. Should the soil and rock conditions observed not support the considerations provided, then better informed recommendations should be prepared and the report revised consistent with those recommendations.

In general, to prepare for shallow foundation and floor slab construction, all topsoil, vegetation, soft to medium soils, and other deleterious materials should be removed from the subgrade areas. The areas shall consist of the width of the footing plus 3-ft wide strips inside and outside the footing to facilitated proofrolling by a ride-on vibratory roller compactor.

The exposed subgrades should be proof-rolled with a minimum of ten passes with a vibratory roller compactor, such as a Catapillar CS56, or equal. Soft or loose zones delineated by the proof-rolling should be undercut to competent material or to an additional depth of one foot, whichever first occurs. Should the additionally undercut subgrade remain unsuitable, it should be stabilized with suitable, inert (non-carbonaceous, non-pyritiferous, non-expansive, and non-slag) American Association of State Transportation Officials (AASHTO) No. 1 stone. The vacated volume can then be backfilled to final subgrade level with suitable, inert, on- or off-site borrow that is compacted as described below, in the Fill/Backfill Considerations section.

Fill/Backfill Considerations

It is recommended that borrow soil used as backfill be either placed and compacted as soon as possible to limit its exposure to rainfall events, or protected with plastic tarps if the construction activity requires multiple days.

Cohesive fill should be compacted to a minimum of 95 percent of the maximum modified proctor dry density as determined by ASTM International Test Designation: D1557-12, at water contents within three percent ($\pm 3\%$) of the optimum water content established by that test.

Granular fill should be compacted to at least 70 percent relative density as determined by ASTM International Test Designations: D4253- and 4254-16.

QUALITY VERIFICATION/CONSIDERATIONS

It should be noted that this is a cursory review of the project conditions and test borings or pits should be excavated prior to construction. It is essential that all test pits, foundation construction, floor slab subgrade preparation, and fill/backfill operations be monitored on a full-time basis by our personnel to verify that the recommended bearing horizons/materials and fill/backfill compaction requirements are consistently implemented. All recommendations presented herein are contingent upon such field verification.

This report has been prepared using cursory review methods conforming to commonly accepted local geotechnical engineering practices. All recommendations and/or conclusions herein pertain only to this specific project and should not be used or interpreted by others for modifications to this project, unless reviewed and approved by us, or for other projects or sites. Even within the project context, subsurface conditions can only be determined by boring or test pits, and actual conditions between/beyond borings or test pits may vary. Due in part to such variability in subsurface conditions, the implementation of the recommended measures must be informed and validated by the recommended test pits and inspected by our personnel to confirm that the subsurface conditions encountered during construction are consistent with the borings or test pits and our engineering analysis, and to verify that the subgrades, backfills, and all other geomaterials used are behaving as anticipated. Some conditions or material/subgrade behavior and/or performance may require modifications to our recommendations, which can typically only be determined "on-the-spot" during full-time inspections by one of our soil technicians, under the direct supervision of our professional (licensed) geotechnical engineers.

Not only could the interpretation and field inspection of our recommendations by others result in a structure that does not perform as intended, but inspection by individuals not qualified and/or

not under the direct supervision of a professional geotechnical engineer could result in structure failures. We therefore will not be responsible nor professionally liable for the performance and/or suitability of any structures affected by geotechnical elements of the project inspected by others. The selected inspection agency must take full responsibility for proper implementation and performance of the project geotechnical recommendations for this site.

We sincerely appreciate the opportunity to be of service to you on this project. Should you have any questions regarding our findings or recommendations, please do not hesitate to contact us.



Respectfully submitted,

THE GATEWAY ENGINEERS, INC.

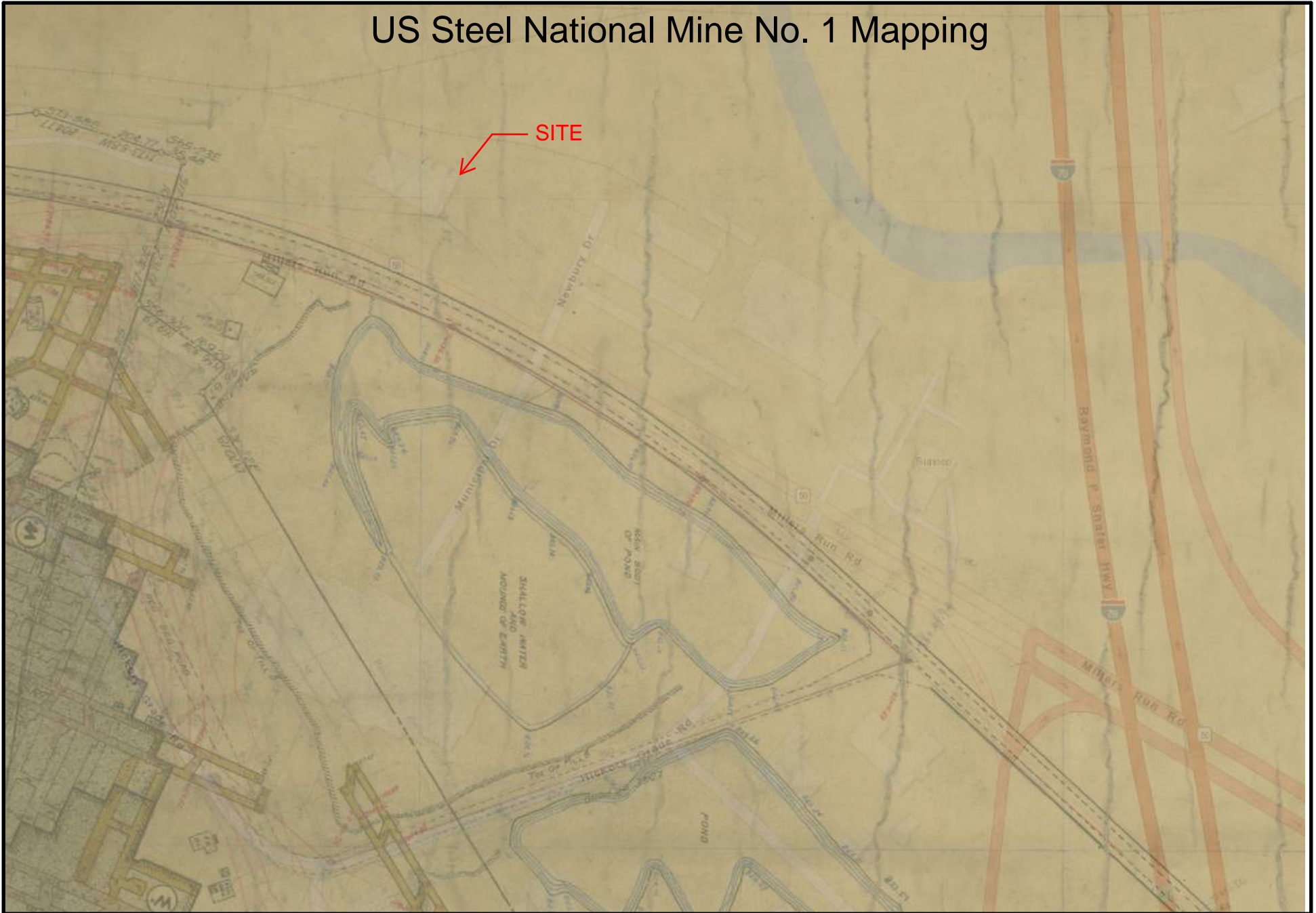
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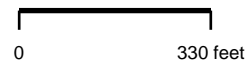
Nathaniel S. Hayes, P.E.
Project Manager

IUPASG_00431 Close Up

US Steel National Mine No. 1 Mapping



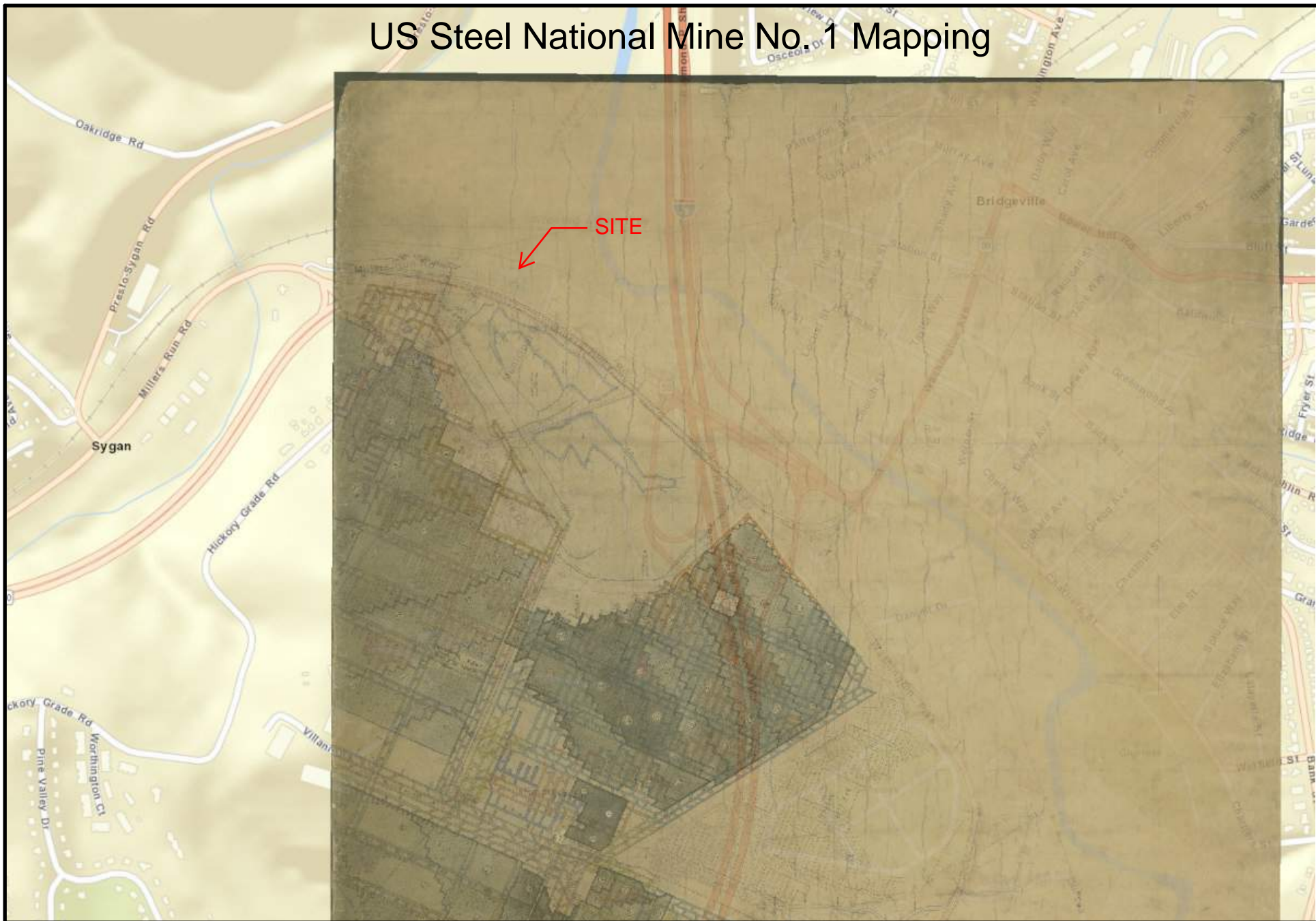
The Department cannot verify the accuracy or completeness of this information or alignment of images.



Created on: 01/12/22

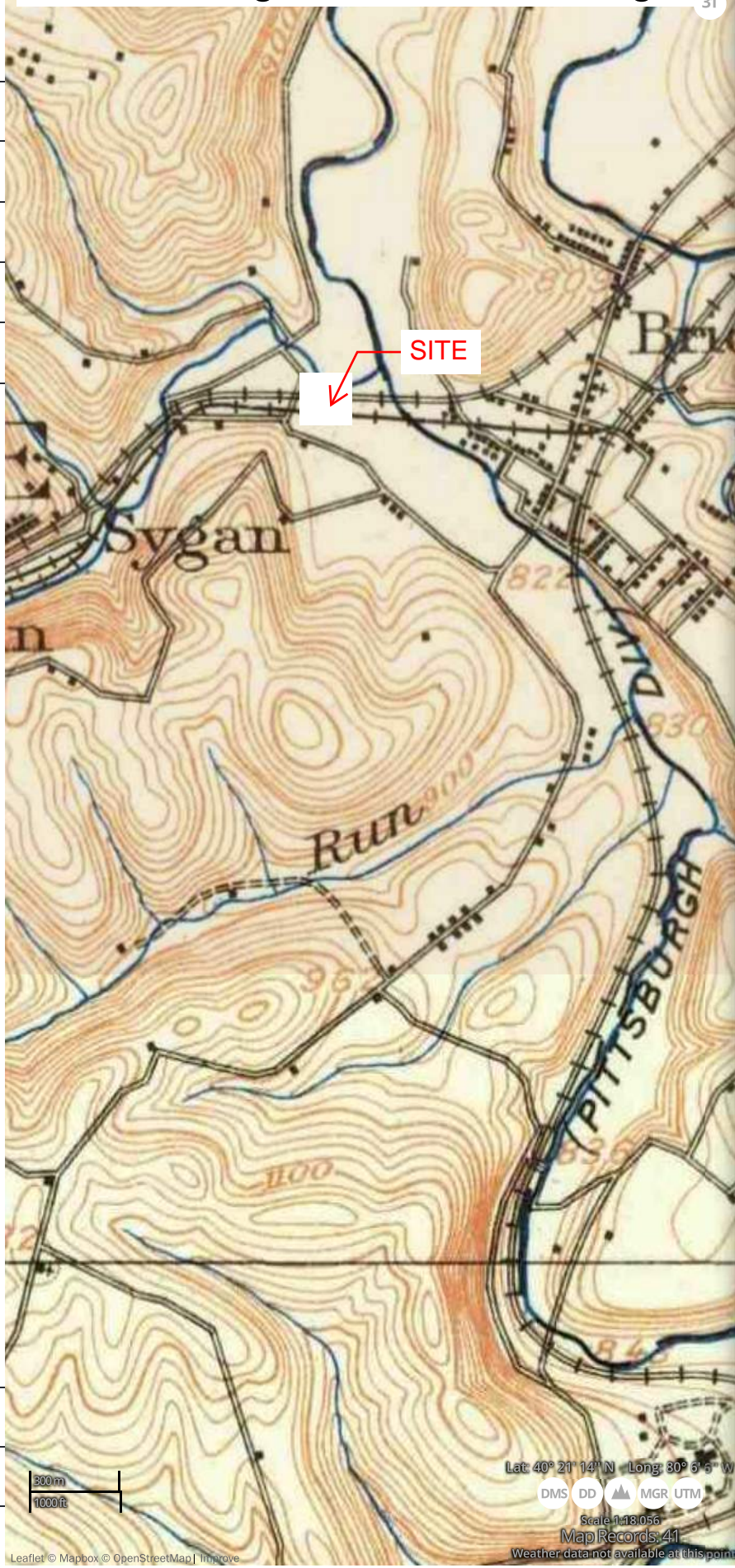


US Steel National Mine No. 1 Mapping



1904 Carnegie USGS Quadrangle

31



Location | Map Name

Search by location

1880 2021

All 250K 100K 63K 48K 24K | HTMC UST All

31 maps here | Scale: All, Date: 1880-2021, Series: All
Elevation @ 40.359, -80.123 is 831 ft. (253 m)

Filter records Name Date Scale State

Carnegie, PA

1904 (HTMC, 1954 ed.) Scale 1:62500

JPEG (3 MB) GeoTiff (9 MB)

KMZ (4 MB) GeoPDF (10 MB)

HIDE INFO ZOOM PAN PIN FIX

MAP TRANSPARENCY

- Carnegie, PA
1906 (HTMC, 1906 ed.) Scale 1:62500
- Carnegie, PA
1906 (HTMC, 1913 ed.) Scale 1:62500
- Carnegie, PA
1906 (HTMC, 1920 ed.) Scale 1:62500
- Carnegie, PA
1906 (HTMC, 1927 ed.) Scale 1:62500
- Carnegie, PA
1906 (HTMC, 1935 ed.) Scale 1:62500
- Carnegie, PA
1906 (HTMC, 1940 ed.) Scale 1:62500
- Carnegie, PA
1906 (HTMC, 1944 ed.) Scale 1:62500
- Carnegie, PA
1908 (HTMC, 1910 ed.) Scale 1:62500
- Canton, OH

Lat: 40° 21' 14" N Long: 80° 6' 6" W

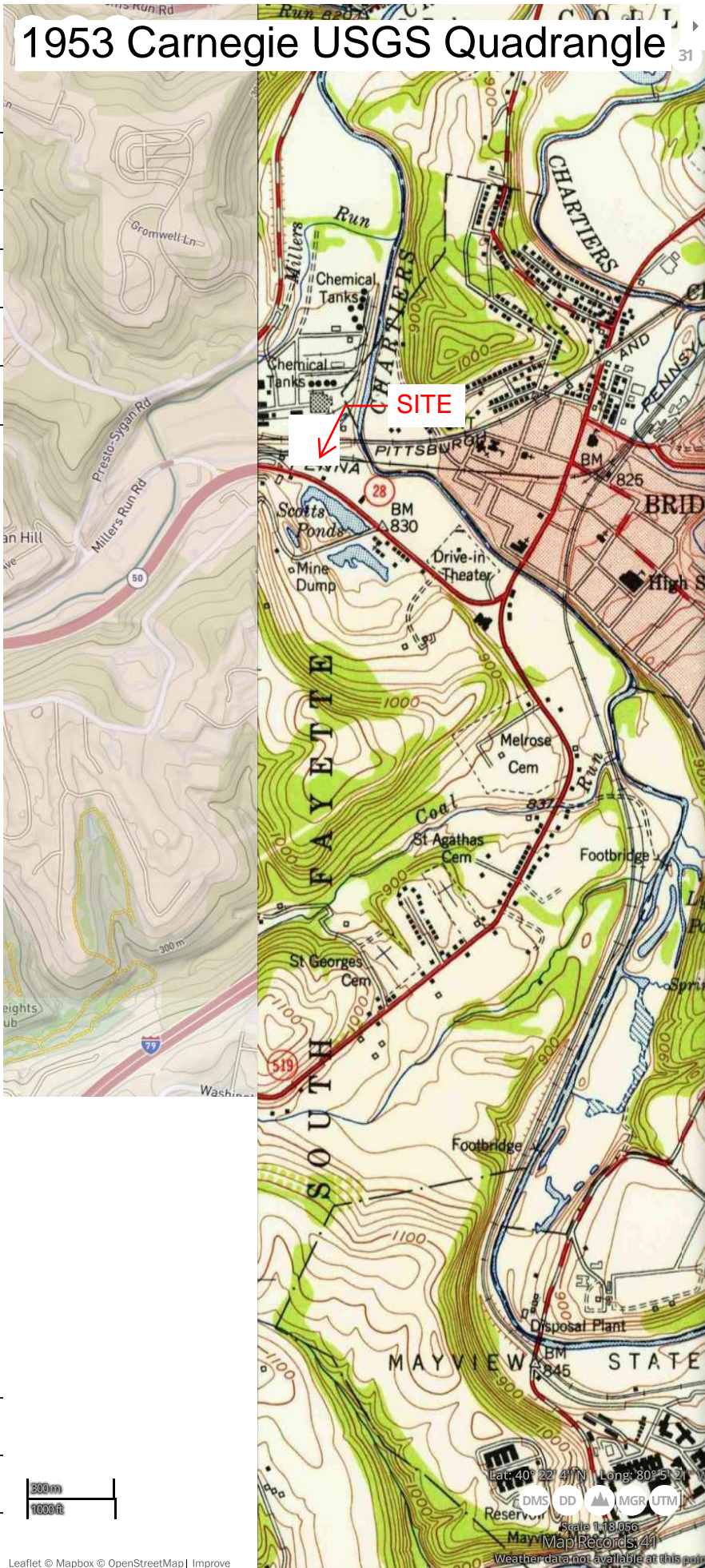
DMS DD MGR UTM

Scale: 1:13,056
MapRecords: 41

Weather data not available at this point



1953 Carnegie USGS Quadrangle



Location | Map Name

Search by location

1880 2021

All 250K 100K 63K 48K 24K | HTMC UST All

31 maps here | Scale: All, Date: 1880-2021, Series: All
Elevation @ 40.359, -80.123 is 831 ft. (253 m)

Filter records Name Date Scale State

- Bridgeville, PA
1953 (HTMC, 1955 ed.) Scale 1:24000
 JPEG (4 MB) GeoTiff (14 MB)
 KMZ (4 MB) GeoPDF (14 MB)
 HIDE INFO ZOOM PAN PIN FIX
 MAP TRANSPARENCY
- Canton, OH
1957 (HTMC, 1971 ed.) Scale 1:250000
- Canton, OH
1957 (HTMC, 1978 ed.) Scale 1:250000
- Canton, OH
1957 (HTMC, 1978 ed.) Scale 1:250000
- Bridgeville, PA
1960 (HTMC, 1961 ed.) Scale 1:24000
- Bridgeville, PA
1960 (HTMC, 1966 ed.) Scale 1:24000
- Bridgeville, PA
1960 (HTMC, 1971 ed.) Scale 1:24000
- Bridgeville, PA
1960 (HTMC, 1979 ed.) Scale 1:24000
- Bridgeville, PA
1960 (HTMC, 1984 ed.) Scale 1:24000

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**PAG-02
AUTHORIZATION TO DISCHARGE UNDER THE
NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES)
GENERAL PERMIT FOR DISCHARGES OF
STORMWATER ASSOCIATED WITH CONSTRUCTION ACTIVITIES**

NPDES PERMIT NO: PAC021240

In compliance with the provisions of the Clean Water Act, 33 U.S.C.A. §§ 1251--1387 and the Clean Streams Law, as amended, 35 P.S. §§ 691.1--691.1001,

**Craig Cozza
CE-SF, LP
295 Myoma Road
Mars, PA 16046**

(permittee) is authorized to discharge from a project site known as **Lafayette 180**, located in **South Fayette Township, Allegheny County** to **Chartiers Creek** in accordance with the effluent limitations, monitoring and reporting requirements, best management practices (BMPs), stormwater control measures (SCMs) and other conditions set forth in Parts A, B, and C herein.

APPROVAL OF COVERAGE TO DISCHARGE UNDER THIS GENERAL NPDES PERMIT IS *AUTHORIZED BEGINNING ON JANUARY 12, 2026* AND WILL *EXPIRE ON DECEMBER 7, 2029*. WHEN THE GENERAL PERMIT IS RENEWED, REISSUED OR MODIFIED, THE FACILITY OR ACTIVITY COVERED BY THE APPROVAL FOR COVERAGE MUST COMPLY WITH THE FINAL RENEWED, REISSUED OR MODIFIED GENERAL PERMIT.

Coverage and authorization to discharge under the PAG-02 NPDES General Permit ("General Permit") are subject to the following conditions:

1. The permittee's Notice of Intent (NOI), Erosion and Sediment Control (E&S) Plan, and Post-Construction Stormwater Management (PCSM) Plan are incorporated into this approval of coverage.
2. If there is a conflict between the requirements in the NOI or its supporting documents and the terms and conditions of the General Permit, the permittee must comply with the terms and conditions of the General Permit.
3. The permittee's failure to comply with the terms, conditions, and effluent limitations of the General Permit is grounds for the Department of Environmental Protection (DEP) and/or a delegated county conservation district (CCD) to take an enforcement action, and/or to terminate or revoke coverage under this General Permit.

Coverage under the PAG-02 General Permit is authorized by:

Matt Gordon

**Matt Gordon
Regulatory Team Director
Allegheny County Conservation District**



GATEWAY

On Call. On Time. On Target.

C-12199-0025

October 2023

Cozza Commercial Building

South Fayette Township
Allegheny County, Pennsylvania

PREPARED FOR
Cozza Enterprises, LLC
P.O. Box 453
Carnegie, PA 15106

SUBMITTED BY

Adam Greathouse
Permitting Specialist

The Gateway Engineers, Inc.
100 McMorris Road
Pittsburgh, PA 15205
412.275.3793 PHONE
412.921.9960 FAX

www.gatewayengineers.com
agreathouse@gatewayengineers.com



A FULL-SERVICE CIVIL ENGINEERING FIRM

WETLAND DELINEATION AND STREAM IDENTIFICATION REPORT

ENVIRONMENTAL

Project Name: Cozza Commercial Building
Client: Cozza Enterprises, LLC
Project Number: C-12199-0025
Prepared By: Adam Greathouse

The Gateway Engineers, Inc. (Gateway), on behalf of Cozza Enterprises, LLC (Cozza), conducted an environmental investigation to identify wetlands and streams for a proposed commercial development in South Fayette Township, Allegheny County, Pennsylvania; hereafter, referred to as the ‘study area’ (Attachment 1 – Site Location Maps). The investigation was conducted as part of the planning stage of the project to identify any wetlands or streams that occur within the study area of the proposed re-route.

The wetland delineation was conducted by Gateway on October 4, 2023, in accordance with the procedures provided in the U.S. Army Corps of Engineers (USACE) *Corps of Engineers Wetland Delineation Manual (1987)*¹ and the USACE’s *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Eastern Mountains and Piedmont Region (Version 2.0)*². The study area included an approximate five (5)-acre area that encompassed the proposed development. The existing land use of the study area is comprised of existing residential area, a graded commercial pad with some presence of herbaceous vegetation and a narrow and forested strip that included a dense scrub/shrub undergrowth consisting of invasive shrubs. The investigation of the study area identified no (0) wetlands and no (0) streams. For verification of a completed delineation, three (3) test pit data forms (TP-1 – TP-3) can be found in Attachment 4 – Data Forms.

The study area is located within the Upper Ohio watershed (HUC 05030101), which drains 1,950 square miles and the PA Water Plan identified watershed 20F – Ohio River. Drainage from the study area flows into Chartiers Creek – Warm Water Fishes (WWF)³. Chartiers Creek flows north-northeast before flowing into the Ohio River (WWF) in McKees Rocks, Pennsylvania.

Zero (0) National Wetland Inventory (NWI)⁴ wetlands were indicated within close proximity to the study area boundaries (Attachment 2 – NWI Map). As a result, no impacts to NWI wetlands are anticipated for the proposed project.

The Soil Survey(s) of Allegheny County⁵, in conjunction with PA DEP’s “Limitations of Pennsylvania Soils Pertaining to Earthmoving Projects,” indicate that no (0) soil series with hydric inclusions occur within the study area (Attachment 3 - USDA/NRCS Soil Map).

The following items are attached to provide further information: Attachment 1 – Site Location Maps; Attachment 2 – NWI Map; Attachment 3 – USDA/NRCS Soil Map; Attachment 4 – Data Forms; Attachment 5 – Personnel Résumés.

¹U.S. Army Corps of Engineers. 1987. *Corps of Engineers Wetland Delineation Manual*. Technical Report Y-87-1.

²U.S. Army Corps of Engineers. April 2012. *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Eastern Mountains and Piedmont Region (Version 2.0)*. EDR/EL TR-12-9.

³ WWF – Warm Water Fishes, as classified by PA Code Title 25, Chapter 93.

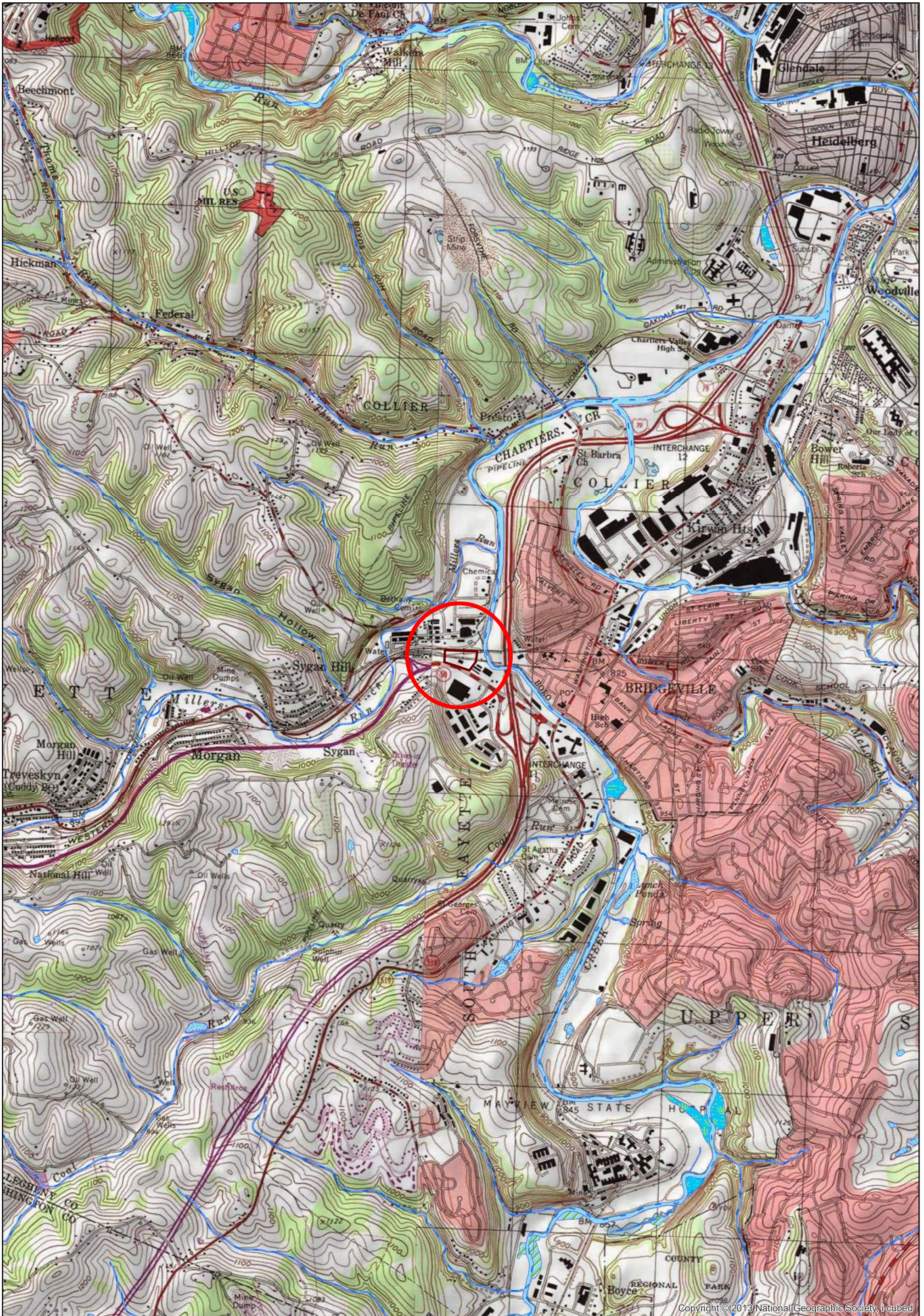
⁴United States fish and Wildlife Service. 2008. *National Wetlands Inventory for Pennsylvania*. Pennsylvania Spatial Data Access. www.pasda.psu.edu

⁵United States Department of Agriculture, Natural Resources Conservation Service. 2008. *Soil Survey Geographic Database for Allegheny County, Pennsylvania*. <http://SoilDataMart.nrcs.usda.gov>. Accessed October 2023.

Cozza Enterprises, LLC
Cozza Commercial Building
South Fayette Township, Allegheny County, PA

ATTACHMENT 1

SITE LOCATION MAPS



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Figure 1 - USGS Site Location Map



100 McMorris Road Pittsburgh, PA 15205
 Phone: 856-634-9284 - Fax 412-921-9960
 http://www.gatewayengineers.com

- National Wetland Inventory
- Freshwater Emergent Wetland
 - Freshwater Forested/Shrub Wetlands
 - Freshwater Pond
 - River/Lake
 - Other
 - Study Area
 - Chapter 93 Streams



1 inch = 2,000 feet

Date: October 12, 2023

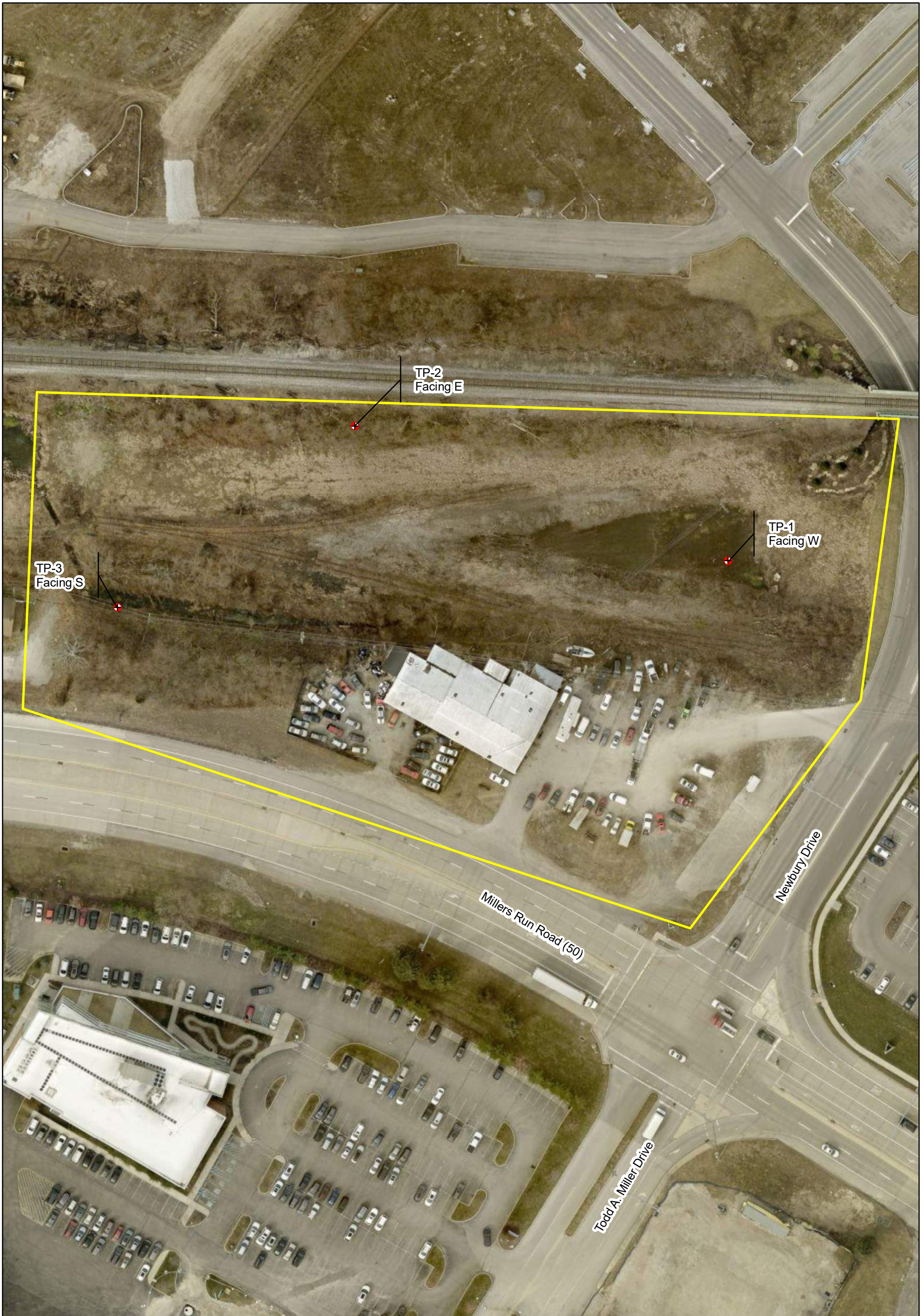


Figure 2 - Aerial Site Location Map

GPS Streams	GPS Data	National Wetland Inventory	
Ephemeral Stream	GPS Wetlands	Freshwater Emergent Wetland	Study Area
Intermittent Stream	Upland Data Point	Freshwater Forested/Shrub Wetlands	Chapter 93 Streams
Perennial Stream	Wetland Data Point	Freshwater Pond	Culvert
	Test Pit	River/Lake	
		Other	



100 McMorris Road Pittsburgh, PA 15205
 Phone: 855-634-9284 - Fax 412-921-9960
<http://www.gatewayengineers.com>

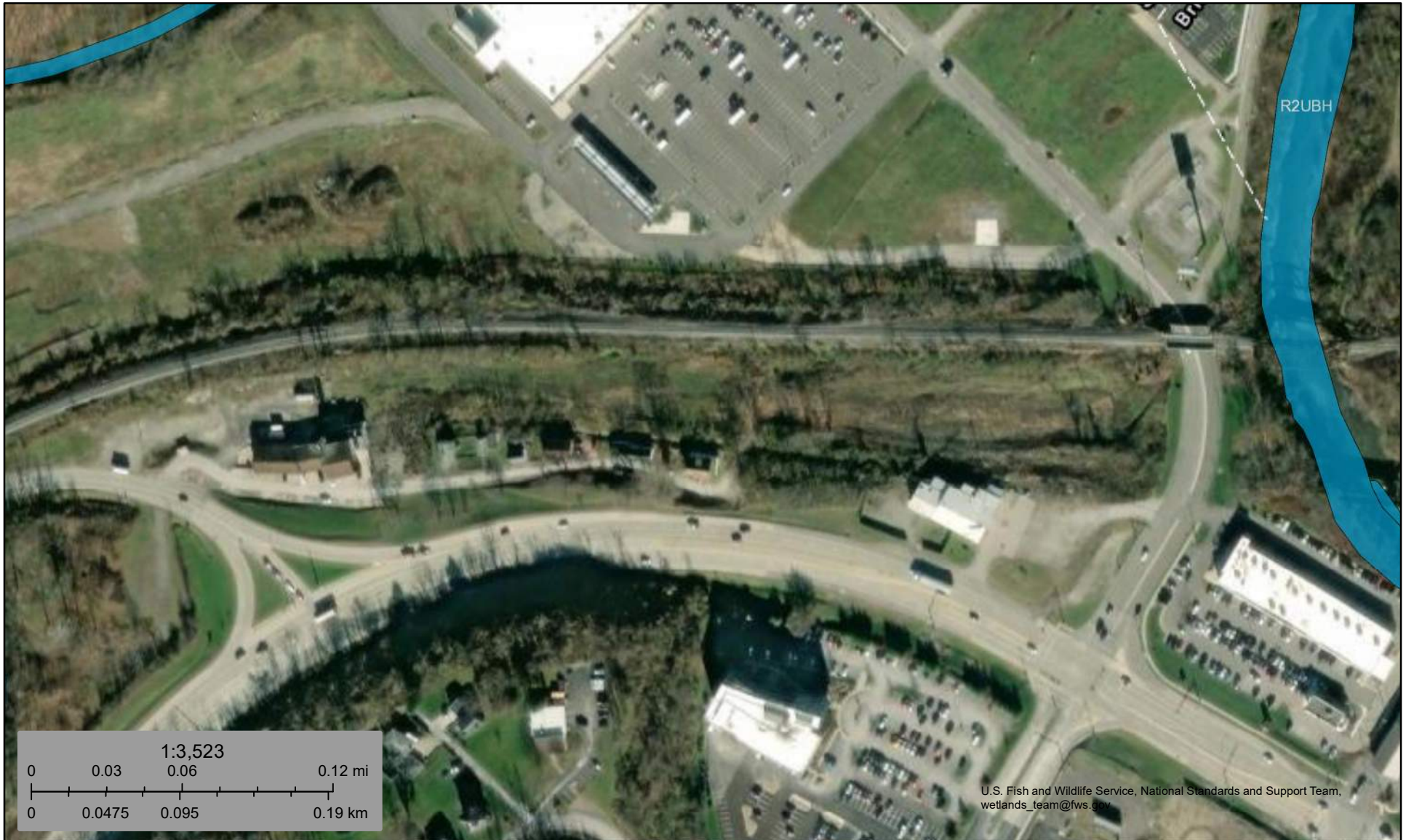
1 inch = 75 feet



Date: October 12, 2023








Cozza Enterprises, LLC
Cozza Commercial Building
South Fayette Township, Allegheny County, PA

ATTACHMENT 2
NWI MAP



October 4, 2023

Wetlands

- | | | |
|--|---|--|
|  Estuarine and Marine Deepwater |  Freshwater Emergent Wetland |  Lake |
|  Estuarine and Marine Wetland |  Freshwater Forested/Shrub Wetland |  Other |
| |  Freshwater Pond |  Riverine |

This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the Wetlands Mapper web site.

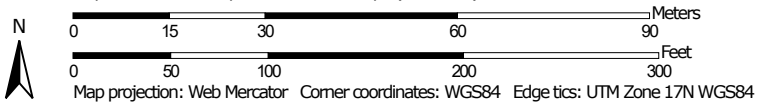
Cozza Enterprises, LLC
Cozza Commercial Building
South Fayette Township, Allegheny County, PA

ATTACHMENT 3
USDA/NRCS SOIL MAP

Soil Map—Allegheny County, Pennsylvania
(Cozza Commercial Building)




Map Scale: 1:1,180 if printed on A landscape (11" x 8.5") sheet.



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Allegheny County, Pennsylvania

Survey Area Data: Version 19, Sep 4, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 19, 2021—Sep 19, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
UB	Urban land	5.1	100.0%
Totals for Area of Interest		5.1	100.0%

Cozza Enterprises, LLC
Cozza Commercial Building
South Fayette Township, Allegheny County, PA

ATTACHMENT 4

DATA FORMS

Project/Site: Cozza Commercial Building City/County: Bridgeville, Allegheny Sampling Date: 10/4/2023
 Applicant/Owner: Cozza Enerprises, LLC State: PA Sampling Point: TP-1
 Investigator(s): ALG Section, Township, Range: South Fayette Twp
 Landform (hillside, terrace, etc.): Toe Slope Local relief (concave, convex, none): concave Slope (%): 0
 Subregion (LRR or MLRA): LRR N Lat: 40.358815 Long: -80.121804 Datum: NAD83
 Soil Map Unit Name: UB - Urban Land NWI classification: N/A
 Are climatic / hydrologic conditions on the site typical for this time of year? Yes X No (If no, explain in Remarks.)
 Are Vegetation Y, Soil Y, or Hydrology N significantly disturbed? Are "Normal Circumstances" present? Yes No X
 Are Vegetation N, Soil N, or Hydrology N naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present? Yes <u>X</u> No <u> </u> Hydric Soil Present? Yes <u> </u> No <u>X</u> Wetland Hydrology Present? Yes <u> </u> No <u>X</u>	Is the Sampled Area within a Wetland? Yes <u> </u> No <u>X</u>
Remarks: TP-1 located amongst a graded site; some vegetation has grown in; however, the soil consists of fill dirt	

HYDROLOGY

Wetland Hydrology Indicators: <u>Primary Indicators (minimum of one is required; check all that apply)</u> <input type="checkbox"/> Surface Water (A1) <input type="checkbox"/> True Aquatic Plants (B14) <input type="checkbox"/> High Water Table (A2) <input type="checkbox"/> Hydrogen Sulfide Odor (C1) <input type="checkbox"/> Saturation (A3) <input type="checkbox"/> Oxidized Rhizospheres on Living Roots (C3) <input type="checkbox"/> Water Marks (B1) <input type="checkbox"/> Presence of Reduced Iron (C4) <input type="checkbox"/> Sediment Deposits (B2) <input type="checkbox"/> Recent Iron Reduction in Tilled Soils (C6) <input type="checkbox"/> Drift Deposits (B3) <input type="checkbox"/> Thin Muck Surface (C7) <input type="checkbox"/> Algal Mat or Crust (B4) <input type="checkbox"/> Other (Explain in Remarks) <input type="checkbox"/> Iron Deposits (B5) <input type="checkbox"/> Inundation Visible on Aerial Imagery (B7) <input type="checkbox"/> Water-Stained Leaves (B9) <input type="checkbox"/> Aquatic Fauna (B13)	<u>Secondary Indicators (minimum of two required)</u> <input checked="" type="checkbox"/> Surface Soil Cracks (B6) <input type="checkbox"/> Sparsely Vegetated Concave Surface (B8) <input type="checkbox"/> Drainage Patterns (B10) <input type="checkbox"/> Moss Trim Lines (B16) <input type="checkbox"/> Dry-Season Water Table (C2) <input type="checkbox"/> Crayfish Burrows (C8) <input type="checkbox"/> Saturation Visible on Aerial Imagery (C9) <input type="checkbox"/> Stunted or Stressed Plants (D1) <input type="checkbox"/> Geomorphic Position (D2) <input type="checkbox"/> Shallow Aquitard (D3) <input type="checkbox"/> Microtopographic Relief (D4) <input type="checkbox"/> FAC-Neutral Test (D5)
--	---

Field Observations: Surface Water Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> Water Table Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> Saturation Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> (includes capillary fringe)	Wetland Hydrology Present? Yes <u> </u> No <u>X</u>
--	---

Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
 surface soil cracks observed; no other signs of hydrology present. Site is graded to towards a small depression where soil cracks were observed.

VEGETATION (Five Strata) – Use scientific names of plants.

Sampling Point: TP-1

Tree Stratum (Plot size: <u>30</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. _____	_____	_____	_____
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
=Total Cover			
50% of total cover: _____		20% of total cover: _____	

Sapling Stratum (Plot size: <u>15</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. _____	_____	_____	_____
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
=Total Cover			
50% of total cover: _____		20% of total cover: _____	

Shrub Stratum (Plot size: <u>15</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. _____	_____	_____	_____
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
=Total Cover			
50% of total cover: _____		20% of total cover: _____	

Herb Stratum (Plot size: <u>5</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Setaria faberi</u>	<u>20</u>	<u>Yes</u>	<u>UPL</u>
2. <u>Echinochloa crus-galli</u>	<u>15</u>	<u>Yes</u>	<u>FAC</u>
3. <u>Panicum capillare</u>	<u>10</u>	<u>Yes</u>	<u>FAC</u>
4. <u>Phleum pratense</u>	<u>5</u>	<u>No</u>	<u>FACU</u>
5. _____	_____	_____	_____
6. _____	_____	_____	_____
7. _____	_____	_____	_____
8. _____	_____	_____	_____
9. _____	_____	_____	_____
10. _____	_____	_____	_____
11. _____	_____	_____	_____
50 =Total Cover			
50% of total cover: <u>25</u>		20% of total cover: <u>10</u>	

Woody Vine Stratum (Plot size: <u>30</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. _____	_____	_____	_____
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
=Total Cover			
50% of total cover: _____		20% of total cover: _____	

Dominance Test worksheet:

Number of Dominant Species That Are OBL, FACW, or FAC: 2 (A)

Total Number of Dominant Species Across All Strata: 3 (B)

Percent of Dominant Species That Are OBL, FACW, or FAC: 66.7% (A/B)

Prevalence Index worksheet:

Total % Cover of:	Multiply by:
OBL species <u>0</u>	x 1 = <u>0</u>
FACW species <u>0</u>	x 2 = <u>0</u>
FAC species <u>25</u>	x 3 = <u>75</u>
FACU species <u>5</u>	x 4 = <u>20</u>
UPL species <u>20</u>	x 5 = <u>100</u>
Column Totals: <u>50</u> (A)	<u>195</u> (B)
Prevalence Index = B/A = <u>3.90</u>	

- Hydrophytic Vegetation Indicators:**
- 1 - Rapid Test for Hydrophytic Vegetation
 - 2 - Dominance Test is >50%
 - 3 - Prevalence Index is ≤3.0¹
 - 4 - Morphological Adaptations¹ (Provide supporting data in Remarks or on a separate sheet)
 - Problematic Hydrophytic Vegetation¹ (Explain)
- ¹Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Definitions of Five Vegetation Strata:

Tree – Woody plants, excluding woody vines, approximately 20 ft (6 m) or more in height and 3 in. (7.6 cm) or larger in diameter at breast height (DBH).

Sapling – Woody plants, excluding woody vines, approximately 20 ft (6 m) or more in height and less than 3 in. (7.6 cm) DBH.

Shrub - Woody Plants, excluding woody vines, approximately 3 to 20 ft (1 to 6 m) in height.

Herb – All herbaceous (non-woody) plants, including herbaceous vines, regardless of size, and woody plants, except woody vines, less than approximately 3 ft (1 m) in height.

Woody Vine – All woody vines, regardless of height.

Hydrophytic Vegetation Present?

Yes	<input checked="" type="checkbox"/>	No	<input type="checkbox"/>
-----	-------------------------------------	----	--------------------------

Remarks: (Include photo numbers here or on a separate sheet.)
 Lots of open space between vegetation; the ground is not 100% covered.

SOIL

Sampling Point: TP-1

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type ¹	Loc ²		
0-9	10YR 2/2	100					Loamy/Clayey	Fill dirt

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, MS=Masked Sand Grains.

²Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators:

- Histosol (A1)
- Histic Epipedon (A2)
- Black Histic (A3)
- Hydrogen Sulfide (A4)
- Stratified Layers (A5)
- 2 cm Muck (A10) (**LRR N**)
- Depleted Below Dark Surface (A11)
- Thick Dark Surface (A12)
- Sandy Mucky Mineral (S1)
- Sandy Gleyed Matrix (S4)
- Sandy Redox (S5)
- Stripped Matrix (S6)
- Dark Surface (S7)

- Polyvalue Below Surface (S8) (**MLRA 147, 148**)
- Thin Dark Surface (S9) (**MLRA 147, 148**)
- Loamy Mucky Mineral (F1) (**MLRA 136**)
- Loamy Gleyed Matrix (F2)
- Depleted Matrix (F3)
- Redox Dark Surface (F6)
- Depleted Dark Surface (F7)
- Redox Depressions (F8)
- Iron-Manganese Masses (F12) (**LRR N, MLRA 136**)
- Umbric Surface (F13) (**MLRA 122, 136**)
- Piedmont Floodplain Soils (F19) (**MLRA 148**)
- Red Parent Material (F21) (**MLRA 127, 147, 148**)

Indicators for Problematic Hydric Soils³:

- 2 cm Muck (A10) (**MLRA 147**)
- Coast Prairie Redox (A16) (**MLRA 147, 148**)
- Piedmont Floodplain Soils (F19) (**MLRA 136, 147**)
- Red Parent Material (F21) (**outside MLRA 127, 147, 148**)
- Very Shallow Dark Surface (F22)
- Other (Explain in Remarks)

³Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if observed):

Type: _____ Rock _____
 Depth (inches): _____ 9 _____

Hydric Soil Present? Yes _____ No X

Remarks:

Area has been previously graded; much of the soil is disturbed and there's not a lot of vegetation present.



TP-1 Facing West

Project/Site: Cozza Commercial Building City/County: Bridgeville, Allegheny Sampling Date: 10/4/2023
 Applicant/Owner: Cozza Enerprises, LLC State: PA Sampling Point: TP-2
 Investigator(s): ALG Section, Township, Range: South Fayette Twp
 Landform (hillside, terrace, etc.): Toe Slope Local relief (concave, convex, none): concave Slope (%): 0
 Subregion (LRR or MLRA): LRR N Lat: 40.359107 Long: -80.122952 Datum: NAD83
 Soil Map Unit Name: UB - Urban Land NWI classification: N/A
 Are climatic / hydrologic conditions on the site typical for this time of year? Yes X No (If no, explain in Remarks.)
 Are Vegetation Y, Soil Y, or Hydrology N significantly disturbed? Are "Normal Circumstances" present? Yes No X
 Are Vegetation N, Soil N, or Hydrology N naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present? Yes <u> </u> No <u>X</u> Hydric Soil Present? Yes <u> </u> No <u>X</u> Wetland Hydrology Present? Yes <u> </u> No <u>X</u>	Is the Sampled Area within a Wetland? Yes <u> </u> No <u>X</u>
Remarks: TP-2 located amongst a graded site; some vegetation has grown in; however, the soil consists of fill dirt	

HYDROLOGY

Wetland Hydrology Indicators: <u>Primary Indicators (minimum of one is required; check all that apply)</u> <input type="checkbox"/> Surface Water (A1) <input type="checkbox"/> True Aquatic Plants (B14) <input type="checkbox"/> High Water Table (A2) <input type="checkbox"/> Hydrogen Sulfide Odor (C1) <input type="checkbox"/> Saturation (A3) <input type="checkbox"/> Oxidized Rhizospheres on Living Roots (C3) <input type="checkbox"/> Water Marks (B1) <input type="checkbox"/> Presence of Reduced Iron (C4) <input type="checkbox"/> Sediment Deposits (B2) <input type="checkbox"/> Recent Iron Reduction in Tilled Soils (C6) <input type="checkbox"/> Drift Deposits (B3) <input type="checkbox"/> Thin Muck Surface (C7) <input type="checkbox"/> Algal Mat or Crust (B4) <input type="checkbox"/> Other (Explain in Remarks) <input type="checkbox"/> Iron Deposits (B5) <input type="checkbox"/> Inundation Visible on Aerial Imagery (B7) <input type="checkbox"/> Water-Stained Leaves (B9) <input type="checkbox"/> Aquatic Fauna (B13)	<u>Secondary Indicators (minimum of two required)</u> <input checked="" type="checkbox"/> Surface Soil Cracks (B6) <input type="checkbox"/> Sparsely Vegetated Concave Surface (B8) <input type="checkbox"/> Drainage Patterns (B10) <input type="checkbox"/> Moss Trim Lines (B16) <input type="checkbox"/> Dry-Season Water Table (C2) <input type="checkbox"/> Crayfish Burrows (C8) <input type="checkbox"/> Saturation Visible on Aerial Imagery (C9) <input type="checkbox"/> Stunted or Stressed Plants (D1) <input type="checkbox"/> Geomorphic Position (D2) <input type="checkbox"/> Shallow Aquitard (D3) <input type="checkbox"/> Microtopographic Relief (D4) <input type="checkbox"/> FAC-Neutral Test (D5)
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Field Observations: Surface Water Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> Water Table Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> Saturation Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> (includes capillary fringe)	Wetland Hydrology Present? Yes <u> </u> No <u>X</u>
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Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
 surface soil cracks observed; no other signs of hydrology present. Site is graded to towards a small depression where soil cracks were observed.

VEGETATION (Five Strata) – Use scientific names of plants.

Sampling Point: TP-2

Tree Stratum (Plot size: <u>30</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Prunus serotina</u>	<u>15</u>	<u>Yes</u>	<u>FACU</u>
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
<u>15</u> =Total Cover			
50% of total cover: <u>8</u>		20% of total cover: <u>3</u>	

Sapling Stratum (Plot size: <u>15</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Robinia pseudoacacia</u>	<u>10</u>	<u>Yes</u>	<u>FACU</u>
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
<u>10</u> =Total Cover			
50% of total cover: <u>5</u>		20% of total cover: <u>2</u>	

Shrub Stratum (Plot size: <u>15</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. _____	_____	_____	_____
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
_____ =Total Cover			
50% of total cover: _____		20% of total cover: _____	

Herb Stratum (Plot size: <u>5</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Cyperus odoratus</u>	<u>45</u>	<u>Yes</u>	<u>FACW</u>
2. <u>Setaria faberi</u>	<u>5</u>	<u>No</u>	<u>UPL</u>
3. <u>Panicum capillare</u>	<u>10</u>	<u>No</u>	<u>FAC</u>
4. <u>Erechtites hieraciifolius</u>	<u>5</u>	<u>No</u>	<u>FACU</u>
5. _____	_____	_____	_____
6. _____	_____	_____	_____
7. _____	_____	_____	_____
8. _____	_____	_____	_____
9. _____	_____	_____	_____
10. _____	_____	_____	_____
11. _____	_____	_____	_____
<u>65</u> =Total Cover			
50% of total cover: <u>33</u>		20% of total cover: <u>13</u>	

Woody Vine Stratum (Plot size: <u>30</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Lonicera japonica</u>	<u>10</u>	<u>Yes</u>	<u>FACU</u>
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
<u>10</u> =Total Cover			
50% of total cover: <u>5</u>		20% of total cover: <u>2</u>	

Dominance Test worksheet:

Number of Dominant Species That Are OBL, FACW, or FAC: 1 (A)

Total Number of Dominant Species Across All Strata: 4 (B)

Percent of Dominant Species That Are OBL, FACW, or FAC: 25.0% (A/B)

Prevalence Index worksheet:

Total % Cover of:	Multiply by:
OBL species <u>0</u>	x 1 = <u>0</u>
FACW species <u>45</u>	x 2 = <u>90</u>
FAC species <u>10</u>	x 3 = <u>30</u>
FACU species <u>40</u>	x 4 = <u>160</u>
UPL species <u>5</u>	x 5 = <u>25</u>
Column Totals: <u>100</u> (A)	<u>305</u> (B)
Prevalence Index = B/A = <u>3.05</u>	

- Hydrophytic Vegetation Indicators:**
- 1 - Rapid Test for Hydrophytic Vegetation
 - 2 - Dominance Test is >50%
 - 3 - Prevalence Index is ≤3.0¹
 - 4 - Morphological Adaptations¹ (Provide supporting data in Remarks or on a separate sheet)
 - Problematic Hydrophytic Vegetation¹ (Explain)
- ¹Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Definitions of Five Vegetation Strata:

Tree – Woody plants, excluding woody vines, approximately 20 ft (6 m) or more in height and 3 in. (7.6 cm) or larger in diameter at breast height (DBH).

Sapling – Woody plants, excluding woody vines, approximately 20 ft (6 m) or more in height and less than 3 in. (7.6 cm) DBH.

Shrub - Woody Plants, excluding woody vines, approximately 3 to 20 ft (1 to 6 m) in height.

Herb – All herbaceous (non-woody) plants, including herbaceous vines, regardless of size, and woody plants, except woody vines, less than approximately 3 ft (1 m) in height.

Woody Vine – All woody vines, regardless of height.

Hydrophytic Vegetation Present?	Yes	No
	<u> </u>	<u>X</u>

Remarks: (Include photo numbers here or on a separate sheet.)

SOIL

Sampling Point: TP-2

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type ¹	Loc ²		
0-6	10YR 4/6	100					Loamy/Clayey	Fill dirt

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, MS=Masked Sand Grains.

²Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators:

- Histosol (A1)
- Histic Epipedon (A2)
- Black Histic (A3)
- Hydrogen Sulfide (A4)
- Stratified Layers (A5)
- 2 cm Muck (A10) (**LRR N**)
- Depleted Below Dark Surface (A11)
- Thick Dark Surface (A12)
- Sandy Mucky Mineral (S1)
- Sandy Gleyed Matrix (S4)
- Sandy Redox (S5)
- Stripped Matrix (S6)
- Dark Surface (S7)

- Polyvalue Below Surface (S8) (**MLRA 147, 148**)
- Thin Dark Surface (S9) (**MLRA 147, 148**)
- Loamy Mucky Mineral (F1) (**MLRA 136**)
- Loamy Gleyed Matrix (F2)
- Depleted Matrix (F3)
- Redox Dark Surface (F6)
- Depleted Dark Surface (F7)
- Redox Depressions (F8)
- Iron-Manganese Masses (F12) (**LRR N, MLRA 136**)
- Umbric Surface (F13) (**MLRA 122, 136**)
- Piedmont Floodplain Soils (F19) (**MLRA 148**)
- Red Parent Material (F21) (**MLRA 127, 147, 148**)

Indicators for Problematic Hydric Soils³:

- 2 cm Muck (A10) (**MLRA 147**)
- Coast Prairie Redox (A16) (**MLRA 147, 148**)
- Piedmont Floodplain Soils (F19) (**MLRA 136, 147**)
- Red Parent Material (F21) (**outside MLRA 127, 147, 148**)
- Very Shallow Dark Surface (F22)
- Other (Explain in Remarks)

³Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if observed):

Type: _____ Rock _____
 Depth (inches): _____ 6 _____

Hydric Soil Present? Yes _____ No X

Remarks:

Area has been filled and graded. The commercial pad drains towards depression at the base of the slope below the railroad.



TP-2 Facing East

Project/Site: Cozza Commercial Building City/County: Bridgeville, Allegheny Sampling Date: 10/4/2023
 Applicant/Owner: Cozza Enerprises, LLC State: PA Sampling Point: TP-3
 Investigator(s): ALG Section, Township, Range: South Fayette Twp
 Landform (hillside, terrace, etc.): Hill Slope Local relief (concave, convex, none): concave Slope (%): 0
 Subregion (LRR or MLRA): LRR N Lat: 40.358667 Long: -80.123662 Datum: NAD83
 Soil Map Unit Name: UB - Urban Land NWI classification: N/A
 Are climatic / hydrologic conditions on the site typical for this time of year? Yes X No (If no, explain in Remarks.)
 Are Vegetation Y, Soil Y, or Hydrology N significantly disturbed? Are "Normal Circumstances" present? Yes No X
 Are Vegetation N, Soil N, or Hydrology N naturally problematic? (If needed, explain any answers in Remarks.)

SUMMARY OF FINDINGS – Attach site map showing sampling point locations, transects, important features, etc.

Hydrophytic Vegetation Present? Yes <u> </u> No <u>X</u> Hydric Soil Present? Yes <u> </u> No <u>X</u> Wetland Hydrology Present? Yes <u> </u> No <u>X</u>	Is the Sampled Area within a Wetland? Yes <u> </u> No <u>X</u>	
Remarks: TP-3		

HYDROLOGY

Wetland Hydrology Indicators: <u>Primary Indicators (minimum of one is required; check all that apply)</u> <input type="checkbox"/> Surface Water (A1) <input type="checkbox"/> True Aquatic Plants (B14) <input type="checkbox"/> High Water Table (A2) <input type="checkbox"/> Hydrogen Sulfide Odor (C1) <input type="checkbox"/> Saturation (A3) <input type="checkbox"/> Oxidized Rhizospheres on Living Roots (C3) <input type="checkbox"/> Water Marks (B1) <input type="checkbox"/> Presence of Reduced Iron (C4) <input type="checkbox"/> Sediment Deposits (B2) <input type="checkbox"/> Recent Iron Reduction in Tilled Soils (C6) <input type="checkbox"/> Drift Deposits (B3) <input type="checkbox"/> Thin Muck Surface (C7) <input type="checkbox"/> Algal Mat or Crust (B4) <input type="checkbox"/> Other (Explain in Remarks) <input type="checkbox"/> Iron Deposits (B5) <input type="checkbox"/> Inundation Visible on Aerial Imagery (B7) <input type="checkbox"/> Water-Stained Leaves (B9) <input type="checkbox"/> Aquatic Fauna (B13)	<u>Secondary Indicators (minimum of two required)</u> <input type="checkbox"/> Surface Soil Cracks (B6) <input type="checkbox"/> Sparsely Vegetated Concave Surface (B8) <input type="checkbox"/> Drainage Patterns (B10) <input type="checkbox"/> Moss Trim Lines (B16) <input type="checkbox"/> Dry-Season Water Table (C2) <input type="checkbox"/> Crayfish Burrows (C8) <input type="checkbox"/> Saturation Visible on Aerial Imagery (C9) <input type="checkbox"/> Stunted or Stressed Plants (D1) <input type="checkbox"/> Geomorphic Position (D2) <input type="checkbox"/> Shallow Aquitard (D3) <input type="checkbox"/> Microtopographic Relief (D4) <input type="checkbox"/> FAC-Neutral Test (D5)
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Field Observations: Surface Water Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> Water Table Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> Saturation Present? Yes <u> </u> No <u>X</u> Depth (inches): <u> </u> (includes capillary fringe)	Wetland Hydrology Present? Yes <u> </u> No <u>X</u>
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Describe Recorded Data (stream gauge, monitoring well, aerial photos, previous inspections), if available:

Remarks:
 surface soil cracks observed; no other signs of hydrology present. Site is graded to towards a small depression where soil cracks were observed.

VEGETATION (Five Strata) – Use scientific names of plants.

Sampling Point: TP-3

Tree Stratum (Plot size: <u>30</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. _____	_____	_____	_____
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
_____ =Total Cover			
50% of total cover: _____		20% of total cover: _____	

Sapling Stratum (Plot size: <u>15</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Ailanthus altissima</u>	<u>15</u>	<u>Yes</u>	<u>FACU</u>
2. <u>Robinia pseudoacacia</u>	<u>15</u>	<u>Yes</u>	<u>FACU</u>
3. <u>Acer negundo</u>	<u>15</u>	<u>Yes</u>	<u>FAC</u>
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
_____ =Total Cover			
50% of total cover: <u>23</u>		20% of total cover: <u>9</u>	

Shrub Stratum (Plot size: <u>15</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Lonicera maackii</u>	<u>20</u>	<u>Yes</u>	<u>UPL</u>
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
6. _____	_____	_____	_____
_____ =Total Cover			
50% of total cover: <u>10</u>		20% of total cover: <u>4</u>	

Herb Stratum (Plot size: <u>5</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Cyperus odoratus</u>	<u>10</u>	<u>Yes</u>	<u>FACW</u>
2. <u>Setaria faberi</u>	<u>10</u>	<u>Yes</u>	<u>UPL</u>
3. <u>Panicum capillare</u>	<u>10</u>	<u>Yes</u>	<u>FAC</u>
4. <u>Hackelia virginiana</u>	<u>5</u>	<u>No</u>	<u>FACU</u>
5. <u>Verbesina alternifolia</u>	<u>5</u>	<u>No</u>	<u>FAC</u>
6. <u>Toxicodendron radicans</u>	<u>5</u>	<u>No</u>	<u>FAC</u>
7. _____	_____	_____	_____
8. _____	_____	_____	_____
9. _____	_____	_____	_____
10. _____	_____	_____	_____
11. _____	_____	_____	_____
_____ =Total Cover			
50% of total cover: <u>23</u>		20% of total cover: <u>9</u>	

Woody Vine Stratum (Plot size: <u>30</u>)	Absolute % Cover	Dominant Species?	Indicator Status
1. <u>Lonicera japonica</u>	<u>15</u>	<u>Yes</u>	<u>FACU</u>
2. _____	_____	_____	_____
3. _____	_____	_____	_____
4. _____	_____	_____	_____
5. _____	_____	_____	_____
_____ =Total Cover			
50% of total cover: <u>8</u>		20% of total cover: <u>3</u>	

Dominance Test worksheet:

Number of Dominant Species That Are OBL, FACW, or FAC: 3 (A)

Total Number of Dominant Species Across All Strata: 8 (B)

Percent of Dominant Species That Are OBL, FACW, or FAC: 37.5% (A/B)

Prevalence Index worksheet:

Total % Cover of:	Multiply by:
OBL species <u>0</u>	x 1 = <u>0</u>
FACW species <u>10</u>	x 2 = <u>20</u>
FAC species <u>35</u>	x 3 = <u>105</u>
FACU species <u>50</u>	x 4 = <u>200</u>
UPL species <u>30</u>	x 5 = <u>150</u>
Column Totals: <u>125</u> (A)	<u>475</u> (B)
Prevalence Index = B/A = <u>3.80</u>	

- Hydrophytic Vegetation Indicators:**
- 1 - Rapid Test for Hydrophytic Vegetation
 - 2 - Dominance Test is >50%
 - 3 - Prevalence Index is ≤3.0¹
 - 4 - Morphological Adaptations¹ (Provide supporting data in Remarks or on a separate sheet)
 - Problematic Hydrophytic Vegetation¹ (Explain)
- ¹Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.

Definitions of Five Vegetation Strata:

Tree – Woody plants, excluding woody vines, approximately 20 ft (6 m) or more in height and 3 in. (7.6 cm) or larger in diameter at breast height (DBH).

Sapling – Woody plants, excluding woody vines, approximately 20 ft (6 m) or more in height and less than 3 in. (7.6 cm) DBH.

Shrub - Woody Plants, excluding woody vines, approximately 3 to 20 ft (1 to 6 m) in height.

Herb – All herbaceous (non-woody) plants, including herbaceous vines, regardless of size, and woody plants, except woody vines, less than approximately 3 ft (1 m) in height.

Woody Vine – All woody vines, regardless of height.

Hydrophytic Vegetation Present?

Yes	<u> </u>	No	<u>X</u>
-----	-----------	----	----------

Remarks: (Include photo numbers here or on a separate sheet.)

SOIL

Sampling Point: TP-3

Profile Description: (Describe to the depth needed to document the indicator or confirm the absence of indicators.)

Depth (inches)	Matrix		Redox Features				Texture	Remarks
	Color (moist)	%	Color (moist)	%	Type ¹	Loc ²		
0-6	10YR 4/4	100					Loamy/Clayey	

¹Type: C=Concentration, D=Depletion, RM=Reduced Matrix, MS=Masked Sand Grains.

²Location: PL=Pore Lining, M=Matrix.

Hydric Soil Indicators:

- Histosol (A1)
- Histic Epipedon (A2)
- Black Histic (A3)
- Hydrogen Sulfide (A4)
- Stratified Layers (A5)
- 2 cm Muck (A10) (**LRR N**)
- Depleted Below Dark Surface (A11)
- Thick Dark Surface (A12)
- Sandy Mucky Mineral (S1)
- Sandy Gleyed Matrix (S4)
- Sandy Redox (S5)
- Stripped Matrix (S6)
- Dark Surface (S7)

- Polyvalue Below Surface (S8) (**MLRA 147, 148**)
- Thin Dark Surface (S9) (**MLRA 147, 148**)
- Loamy Mucky Mineral (F1) (**MLRA 136**)
- Loamy Gleyed Matrix (F2)
- Depleted Matrix (F3)
- Redox Dark Surface (F6)
- Depleted Dark Surface (F7)
- Redox Depressions (F8)
- Iron-Manganese Masses (F12) (**LRR N, MLRA 136**)
- Umbric Surface (F13) (**MLRA 122, 136**)
- Piedmont Floodplain Soils (F19) (**MLRA 148**)
- Red Parent Material (F21) (**MLRA 127, 147, 148**)

Indicators for Problematic Hydric Soils³:

- 2 cm Muck (A10) (**MLRA 147**)
- Coast Prairie Redox (A16) (**MLRA 147, 148**)
- Piedmont Floodplain Soils (F19) (**MLRA 136, 147**)
- Red Parent Material (F21) (**outside MLRA 127, 147, 148**)
- Very Shallow Dark Surface (F22)
- Other (Explain in Remarks)

³Indicators of hydrophytic vegetation and wetland hydrology must be present, unless disturbed or problematic.

Restrictive Layer (if observed):

Type: _____ Rock _____
 Depth (inches): _____ 6 _____

Hydric Soil Present? Yes _____ No X

Remarks:



TP-3 Facing South

Cozza Enterprises, LLC
Cozza Commercial Building
South Fayette Township, Allegheny County, PA

ATTACHMENT 5
PERSONNEL RESUME/S



Project Team Resume

Adam L. Greathouse

Permitting Specialist

Years of Experience

15 Years conducting stream delineations, 6 years conducting wetland delineations

Education

Pennsylvania State University – University Park, Pennsylvania

B.S. in Wildlife and Fisheries Science, 2007

Pennsylvania State University – DuBois, Pennsylvania

Associate in Wildlife Technology, 2005

Registrations / Certifications

N/A

Affiliations

NA

Training

36-Hour Wetland Delineation Training, Gailey Environmental LLC, 2020

Primary Headwater Stream Assessment Training, EnviroScience, Inc., 2022

Endangered Species Act Consultation, Swamp School, LLC, 2023

Memberships

N/A

General Qualifications

Mr. Greathouse joined the Gateway Engineers, Inc as a full time employee in August, 2023, after working as an environmental scientist for two other multi-disciplined environmental engineering firms since 2008. He has experience performing the following tasks: freshwater wetland identification, delineation and functional assessment analysis; preparation of PA and OH state and federal permits; stream classification and delineation, aquatic macroinvertebrate identification to family; backpack electrofishing with fish identification; and client relations.

REPRESENTATIVE PROJECTS

Erosion and Sedimentation Projects

Erosion and Sedimentation Control Inspections – Client: Multiple

- Responsibilities included: Walking the project area assessing the E&S controls, coordinating with the project manager/contractor; compiled inspection reports and worked with clients to address concerns.

Wetland Delineation and Stream Identification Projects

Stream Restoration PRP Design – Client: Various Municipalities

- Responsibilities included: Conduct detailed wetland delineation and stream identification studies within a stream corridor; compile the associated environmental reports and compile the associated permits for submission to the PADEP and/or the county conservation district.

Residential Land Development – Client: Maronda Homes LLC, PA/OH/KY

- Responsibilities included: Conduct detailed wetland delineation and stream identification studies at multiple locations; compile the associated environmental reports and compile the associated permits for submission to the PADEP and/or the county conservation district.

Commercial Development for GetGo/WetGo – Client: Giant Eagle, Inc., PA/OH

- Responsibilities included: Conduct detailed wetland delineation and stream identification studies at multiple locations; compile the associated environmental reports and compile the associated permits for submission to the PADEP and/or the county conservation district.

Waterline Expansion Project – New Sewickley Township Municipal Authority, PA

- Responsibilities included: Conduct detailed wetland delineation and stream identification studies along a 3-mile line; compile the associated environmental reports and compile the associated permits for submission to the PADEP and/or the county conservation district.

Underground Mining Areas – Client: Multiple Mining Clients, Greene County, PA

- Responsibilities included: Conduct pre- and post-mining stream and wetland assessments; complete macroinvertebrate sampling; compile the associated reports and data forms for submission to PADEP; and compile applicable permitting for submission to PADEP.

Endangered Species Surveys

Indiana Bat Tree Evaluation – Client: Alpha Natural Resources

- Responsibilities included: complete tree surveys within plot areas identified in GIS; measure and identify all trees greater than 3” DBH; and compile data forms per plot area.



Peoples Natural Gas Company LLC
375 North Shore Drive
Pittsburgh, PA 15212

August 10, 2023

Joseph Chirumbolo
Utilities Specialist, Safety Tier 2
The Gateway Engineers, Inc.
100 McMorris Road
Pittsburgh, PA 15205

Subject: Letter of Gas Availability for **“Lafayette 180” – located at 180 Millers Run Road in Bridgeville, PA.**

Dear Joseph:

Thank you for your interest in the Peoples Natural Gas Company. We have reviewed your request and have determined that natural gas service is available to serve the “proposed” project known as **“Lafayette 180” – located at 180 Millers Run Road in Bridgeville, PA.**

Please be aware that any time we extend our mainline or increase our capacity, a possibility exists that there may be a cost to you, the requesting party. The determination of cost will be made once a gas application and formal site plans have been submitted.

At a high level we would be able to provide these units service, but we cannot guarantee this until we have processed a formal application.

To submit a gas application, you will need to:

- Please provide the following items to complete your application for gas service:
 - Service Address
 - Approval Letter Name/Address
 - Billing Name/Address
 - Gas equipment and load information (BTUs)
 - Pressure Requirements
 - AutoCAD file of Site Plans (.dwg file)

If you have any questions about submitting your gas application, please call us at (866) 654-4660.

Sincerely,

Peoples Natural Gas Company



August 10, 2023

Mr. Joseph Chirumbolo
Gateway Engineers, Inc.
100 McMorris Rd
Pittsburgh, PA 15205

RE: LaFayette 180 – Cozza Development

Dear Mr. Chirumbolo

This letter is to inform you of the availability of natural gas service from Columbia Gas of Pennsylvania, Inc. (CPA) for the above-referenced project. CPA has existing natural gas facilities along Route 50 in Bridgeville, PA. Please be advised that while gas service is located close to the site, this letter should not be interpreted as a commitment to serve until a more thorough engineering review has been completed. Once CAD development drawings and connected load information are received, our engineering group will begin the design of a gas system to serve this development.

Please keep me updated on the timing and construction progress for this project so we may coordinate the installation of the natural gas facilities to meet your development timeline. I look forward to working with you to assist in securing natural gas service to this new building.

Best regards,

Jc Danhires

Jc Danhires.
Sr. Lead Development Manager
724-610-8378
JDanhires@NiSource.com



Joseph E. Chirumbolo
Utilities Specialist, Safety Tier 2
100 McMorris Road,
Pittsburgh, PA 15205

Attn: Joseph E. Chirumbolo

RE: Will Serve request for availability of Comcast communication services.

Property Address: Development located at 150 Millers Run Road, Bridgeville, PA 15017
Date of Issue: August 09, 2023.

Dear Joseph,


In response to your request for service, we have determined that based on our initial investigation, Comcast Cable Communications Management LLC has the ability to construct and install certain wires, cables, and other equipment over, under, across and along the property located at 150 Millers Run Road, Bridgeville, PA 15017.

A preliminary plan if available, may be provided with this "Will Serve" letter. Notwithstanding the determination that Comcast may provide services at the Property, this letter does not represent any binding agreement for service. Additionally, this letter is non-transferrable and expires one hundred and eighty days from the issue date

If you have any questions or need more information, feel free to contact us.

*Jodi Alberta D.X.P.
Market Development & Expansion
Business Development Organization
Keystone Regional Office
15 Summit Park Drive
Pittsburgh, PA 15275
724.554.4270 Cell*



Authorized by  _____
7018182702564021

Name: Jodi Alberta
Market Development & Expansion

Gateway Engineers

Joseph Chirumbolo

Mr. Chirumbolo:

Please be advised that Verizon has existing facilities near Lafayette 180, Cuddy, PA Commercial Development and will provide service to this location.

If you have any questions, I can be reached at 724 229-0695.

Sincerely

Keith Fowler

Verizon Outside Plant Engineer



MUNICIPAL AUTHORITY
TOWNSHIP OF SOUTH FAYETTE

C. Kenneth Chambon, Chairman
Charles L. Rothermel, Vice-Chairman
Robert Zedreck, Secretary
Glenn Davis, Treasurer
John Alan Kosky, Assistant Secretary/Treasurer

900 Presto Sygan Road
Bridgeville PA 15017
412-257-5100 phone
412-257-5125 fax
sewerbilling@sftwp.com
www.matsf.net

September 5, 2023

Shannon L. McCullough
100 McMorris Road
Pittsburgh PA, 15205

Subject: LaFayette 180

Dear Shannon:

I am in receipt of your request dated September 5, 2023, requesting service availability for the old Foremost Auto property. Please accept this correspondence confirming the availability of sanitary sewer service. The plans do not show how the building will connect but sanitary sewer is available in the front and rear of the proposed building.

Don't hesitate to contact me should you have any questions in this regard.

Sincerely,

**THE MUNICIPAL AUTHORITY OF
THE TOWNSHIP OF SOUTH FAYETTE**

Nick Goettman
Operations Manger



Joseph Chirumbolo of Gateway Engineers
Mailing Address: 100 McMorris Road Pittsburgh, PA 15205
Date: October 17, 2023

Point of Service Description: Lafayette 180 Newbury Drive Cuddy, PA 15031

Dear Requestor:

In response to your recent request for service on the subject property, the following information is provided regarding the process for obtaining electric service from West Penn Power ("WPP").

It has been determined that the subject property is within WPP's service territory. Electric service will be provided in accordance with WPP's Tariff for Service which is on file with and approved by the Pennsylvania Board of Public Utilities. Completion of your request for service is contingent upon your obtaining all required approvals from appropriate authorities and agencies, including, but not limited to, permits for construction. It will be your responsibility to provide WPP with complete electric load information and site plans for review, approvals, and engineering, etc. well in advance of construction of the proposed facility.

The Applicant/Customer is responsible for compliance with all Federal, State and Local Codes. This includes but is not limited to the National Electric Code and the National Electric Safety Code. WPP reserves the right to refuse connection to customer premises that are not in compliance with applicable Codes.

The exact method of service to the point of delivery will be determined subject to the above-requested information and approval by our Regional Engineering Process. This may require the Applicant/Customer to pay for a prorated portion of the cost to upgrade system facilities or add other necessary equipment to the electrical infrastructure to meet your requirements.

Thank you for your inquiry. If you wish to proceed with this project, you'll need to apply for service by calling 1-800-686-0021. We look forward to working with you.

Reference Number: PA-WPP-2023 05 17-202305170587

THIS DEED

MADE the 14th day of January, 2022

BETWEEN

C. Hackett Holdings, LLC, a Pennsylvania limited liability company
(hereinafter called "Grantor")

AND

CE-S.F. One, LP, a Pennsylvania limited partnership
(hereinafter called "Grantee")

WITNESSETH, that the said Grantor in consideration of One Million Five Hundred Sixty-Nine Thousand Two Hundred Fifty and no/100 Dollars (\$1,569,250.00), paid to the Grantor by the Grantee, receipt of which is hereby acknowledged, does grant, bargain, sell, and convey to the said Grantee, its successors and assigns, all of the Grantor's right, title, and interest in and to the following property:

ALL THAT CERTAIN lot or tract of land situate in the Township of South Fayette, County of Allegheny and Commonwealth of Pennsylvania, being known as Lot No. 2, as shown on a certain plan entitled Schneider Plan, recorded in the Department of Real Estate Office of Allegheny County, Pennsylvania in Plan Book Volume 130, Page 133.

AND

ALL THAT CERTAIN lot or piece of ground situate in the Township of South Fayette, County of Allegheny and Commonwealth of Pennsylvania, bounded and described as follows:

BEGINNING at a point on the Northeasterly side of State Highway L.R. 545 (also known as Traffic Route 28) at the corner of land now or late of Humble Oil and Refining Co.; thence along the Northeasterly side of said State Highway, Northwestwardly by the arc of a circle curving to the left, having a radius of 3367.10 feet, an arc distance of 297.75 feet to a point on line of land now or late of Anelita Ferri and Angelina Mals; thence by said land North 19 degrees, 05 minutes, 35 seconds East, 102.41 feet to a point on line of land now or late of Humble Oil and Refining Company; thence by said land the following two courses and distances; South 58 degrees, 59 minutes, 55 seconds East 310.50 feet to a point; thence South 25 degrees, 57 minutes, 05 seconds West 55.77 feet to the Northeasterly side of said State Highway at the place of beginning.

SUBJECT TO the condemnation of a portion of the subject property by the Commonwealth of Pennsylvania, Department of Transportation of right of way for Legislative Route 1138 of the Court of Common Pleas of Allegheny County, Pennsylvania, at No. 1717 October Term, 1971.

ALSO DESCRIBED AS all that certain lot or parcel of land situate in the Township of South Fayette, County of Allegheny, Commonwealth of Pennsylvania, being a portion of Block and Lot No. 256-L-2, more particularly bounded and described as follows:

Beginning at a point on the northerly right of way line of Miller Run Road, S.R. 0050, variable width, said point being at the southeast corner of property now or formerly Anthoni Mals Peterson (Tax Parcel 256-L-1); thence along the dividing line of property now or formerly Anthoni Mals Peterson and property herein described, North 20°43'08" East, 29.76' to a point on the former southerly line of Lot 2 of the Schneider Plan, recorded in Plan Book Volume 130, Page 133; thence along the former southerly line of Lot 2 of the Schneider Plan and through property now or formerly C. Hackett Holdings, LLC, (Tax Parcel 256-L-2), South 57°22'22" East, 225.98' to a point on the northerly right of way line of Miller Run Road, S.R. 0050, variable width; thence along the northerly right of way line of Miller Run Road, S.R. 0050, by an arc of a circle deflecting to the left in a northwestwardly direction, having a radius of 1245.92', an arc distance of 222.05' (chord bearing and distance, North 64°55'04" West, 221.76') to a point at the place of beginning.

Bearings based on First Revision to the Kosky Plan of Lots, recorded in Plan Book Volume 274, Page 44.

Contains 2,559 Sq. Ft. or 0.0587 Acres

THE ABOVE DESCRIBED PROPERTY TOGETHER BEING BLOCK AND LOT 256-L-2.

TOGETHER with and subject to all rights, duties and obligations set forth in the certain Easement Agreement, dated December 14, 2015, and recorded January 13, 2016, in Plan Book Volume 16256, page 110, being an Access Drive Easement.

BEING the same property which Richard Schneider and Jane Schneider, husband and wife, by Corrective Deed dated January 10, 2022 and recorded on January __, 2022 in the Department of Real Estate of Allegheny County, Pennsylvania, in Deed Book Volume _____, Page _____ granted and conveyed C. Hackett Holdings, LLC.

UNDER AND SUBJECT TO coal and mining rights and all rights and privileges incident to the mining of coal heretofore conveyed, excepted, or reserved by instruments of record; the right of surface, lateral, or subjacent support; or any surface subsidence; oil and gas and minerals and all rights incident to the extraction or development of oil and gas or minerals heretofore conveyed, leased, excepted, or reserved by instruments of record; and all easements, rights of way, and restrictions as contained in prior instruments of record and/or as installed or located on the premises and all other matters of record appearing prior hereto.

With the appurtenances thereto: **TO HAVE AND TO HOLD** the same to and for the use of the said Grantee, its successors and assigns forever, and the Grantor for its successors and assigns hereby covenants and agrees that it will **SPECIALLY** warrant title to the property hereby conveyed.

NOTICE: THIS DOCUMENT MAY NOT/DOES NOT SELL, CONVEY, TRANSFER, INCLUDE, OR INSURE THE TITLE TO THE COAL AND RIGHT OF SUPPORT UNDERNEATH THE SURFACE LAND DESCRIBED OR REFERRED TO HEREIN, AND THE OWNER OR OWNERS OF SUCH COAL MAY HAVE/HAVE THE COMPLETE LEGAL RIGHT TO REMOVE ALL OF SUCH COAL AND, IN THAT CONNECTION, DAMAGE MAY RESULT TO THE SURFACE OF THE LAND AND ANY HOUSE, BUILDING, OR OTHER STRUCTURE ON OR IN SUCH LAND. THE INCLUSION OF THIS NOTICE DOES NOT ENLARGE, RESTRICT, OR MODIFY ANY LEGAL RIGHTS OR ESTATES OTHERWISE CREATED, TRANSFERRED, EXCEPTED, OR RESERVED BY THIS INSTRUMENT. [This notice is set forth in the manner provided in Section 1 of the Act of July 17, 1957, P.L. 984, as amended, and is not intended as notice of unrecorded instruments, if any.]

WITNESS the hand and seal of the said Grantor.

WITNESS:

C. Hackett Holdings, LLC

[Signature]

By [Signature]
Charles Hackett, Member

Commonwealth of Pennsylvania)
County of Allegheny) ss:

On this, the 14th day of January, 2022, before me, a Notary Public, the undersigned officer, personally appeared Charles Hackett, who acknowledged himself to be the Member of C. Hackett Holdings, LLC, a Pennsylvania limited liability company, and that he as such Member, being authorized to do so, executed the foregoing deed for the purposes therein contained by signing the name of the corporation by himself as such Member.

IN WITNESS WHEREOF, I have hereunto set my hand and official seal.

[Signature]
Notary Public

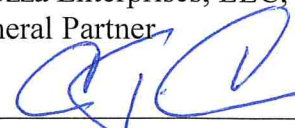
My commission expires:

Commonwealth of Pennsylvania - Notary Seal
Thomas H. Ayoub III, Notary Public
Allegheny County
My commission expires February 21, 2025
Commission number 1004840
Member, Pennsylvania Association of Notaries

NOTICE: THE UNDERSIGNED, AS EVIDENCED BY THE SIGNATURE[S] TO THIS NOTICE AND THE ACCEPTANCE AND RECORDING OF THIS DEED, IS/ARE FULLY COGNIZANT OF THE FACT THAT THE UNDERSIGNED MAY NOT BE OBTAINING THE RIGHT OF PROTECTION AGAINST SUBSIDENCE, AS TO THE PROPERTY HEREIN CONVEYED, RESULTING FROM COAL MINING OPERATIONS AND THAT THE PURCHASED PROPERTY, HEREIN CONVEYED, MAY BE PROTECTED FROM DAMAGE DUE TO MINE SUBSIDENCE BY A PRIVATE CONTRACT WITH THE OWNERS OF THE ECONOMIC INTEREST IN THE COAL. THIS NOTICE IS INSERTED HEREIN TO COMPLY WITH THE BITUMINOUS MINE SUBSIDENCE AND LAND CONSERVATION ACT OF 1966, AS AMENDED 1980, OCT. 10, P.L. 874, NO. 156, § 1.

WITNESS:



CE-S.F. One, LP
By: Cozza Enterprises, LLC,
its General Partner
By: 
Craig J. Cozza, Managing Member

CERTIFICATE OF RESIDENCE

I hereby certify that (1) FOR THE PURPOSE OF DELIVERY OF TAX STATEMENTS ONLY, the precise residence of the Grantee is P.O. Box 453, Carnegie, PA 15106,

and (2) FOR ALL OTHER PURPOSES (including delivery of assessment change notices) the precise residence of Grantee is P.O. Box 453, Carnegie, PA 15106.

Witness the due execution hereof this 14th day of January, 2022



Grantee/Agent for Grantee

AFTER RECORDING, PLEASE RETURN TO:

Pioneer Land Settlement, Inc.
710 Fifth Ave. – Suite 2000
Pittsburgh, PA 15219

PURCHASE AND SALE AGREEMENT

THIS PURCHASE AND SALE AGREEMENT ("Agreement") by and between PETERSON ANTHONI MALS ("Seller") and CE-South Fayette, LP or related assigns, a limited liability company ("Purchaser") is made and entered into as of the last date this Agreement is executed by Seller or Purchaser (the "Effective Date").

Recitals:

WHEREAS, Seller is the owner of a certain property identified as Millers Run Rd in the City of Bridgeville, County of 946 South Fayette, Commonwealth of Pennsylvania identified as Block and Lot Parcel ID 0256-L-00001-0000-00, as such property is more fully described at Deed Book Volume 10562, page 390 (the "Property"); and

WHEREAS, Seller desires to sell and Purchaser desires to purchase the Property, pursuant to the terms, provisions, and conditions herein.

NOW, THEREFORE, intending to be legally bound the parties hereto agree as follows:

1. PURCHASE AND SALE OF PROPERTY.

Seller agrees to sell to Purchaser, and Purchaser agrees to purchase from Seller, subject to the terms and conditions of this Agreement, the Property. The Property shall be conveyed together with all privileges, rights, easements and appurtenances belonging to such land, and all right, title and interest (if any) of Seller in and to any streets, alleys, passages, and other rights-of-way or appurtenances included in, adjacent to or used in connection with such land, and all right, title and interest (if any) of Seller in all mineral and development rights appurtenant to such land and with all of the rights and privileges attributable to ownership of the Property.

2. PURCHASE PRICE AND DEPOSIT.

2.1 The purchase price for the Property shall be [REDACTED] (the "Purchase Price").

2.2 Within three (5) days of the Effective Date, Purchaser shall deposit with Pioneer Land Settlement, Inc. (hereinafter "Title Company") as escrow agent, a deposit in the amount of [REDACTED] (the "Deposit") to be held in a non-interest bearing account.

2.3 At Closing, the Deposit shall be applied to the Purchase Price.

3. TITLE.

3.1 Title to the Property shall be conveyed to Purchaser at Closing in fee simple by general Warranty Deed, in a form and substance satisfactory to Purchaser's counsel.

3.2 Purchaser shall obtain a Commitment for Title Insurance from the Title Company, committing to insure upon the payment of a requisite premium at standard rates that Purchaser shall own good and indefeasible fee simple title to the Property, subject only to the

Permitted Exceptions, as defined herein.

3.3 The term "Permitted Exceptions", as used herein, shall mean (i) the lien of real estate taxes not yet due and payable, (ii) all matters revealed in the Title Commitment obtained by Purchaser and approved by Purchaser, (iii) all existing building, zoning and other city, state, county or federal laws, codes and regulations affecting the Property, (iv) any existing general utility easements serving the Property, provided such existing utility easements would not materially interfere with Purchaser's intended use of the Property as determined by Purchaser in its sole discretion, and (v) any title exception created directly by any act or omission of Purchaser or its representatives, agents, employees or invitees.

3.4 Notwithstanding anything to the contrary in this agreement, Seller shall pay all costs of clearing title.

4. DUE DILIGENCE PERIOD.

4.1 Purchaser, at Purchaser's sole expense, shall have the right for a period of [REDACTED] days from the Effective Date (the "Due Diligence Period"), at any time, to perform any due diligence at its sole cost and expense that it deems proper, including but not limited to, survey and title review, environmental review, structural review, roof evaluation, electrical and plumbing review, and zoning review. Purchaser may elect, at its sole discretion, during the Due Diligence Period, to terminate this Agreement for any reason (or for no reason whatsoever) and receive the prompt refund of the Deposit. Purchaser shall elect to terminate this Agreement by providing written notice delivered to Seller prior to the expiration of the Due Diligence Period notifying Seller that Purchaser is terminating this Agreement. In the absence of such notice, this Agreement shall remain in full force and effect.

4.2 Seller shall provide to Purchaser, within five (5) days after the Effective date of this Agreement, to the extent such are available to Seller, a copy of all plans, drawings, and blueprints pertaining to the Property, any existing title insurance policies covering the Property, a copy of any site plans and/or surveys for the Property, and a copy of any environmental reports.

4.3 The Purchaser shall have one (1) successive option to extend the term of the Due Diligence Period for periods of thirty (30) days each. To exercise an option to extend the term of the Due Diligence Period, Purchaser must notify Seller in writing to be received by Seller on or before the end of the Due Diligence Period, as extended, pursuant to the notice provisions set forth below in this Agreement.

5. REPRESENTATIONS AND WARRANTIES OF SELLER.

Seller hereby represents and warrants the following to the Purchaser as of the date Seller signs this Agreement and as of the Closing:

5.1 Seller is the record owner in fee simple of the Property, and the Property will be on the Closing date free and clear of all liens and encumbrances except for Permitted Exceptions, as defined herein.

5.2 Seller possesses all requisite power and authority to enter into and perform this Agreement and to carry out the transactions contemplated herein. The execution and delivery by Seller of this Agreement and the performance and consummation by Seller of the transaction

contemplated by this Agreement have been duly and validly authorized by all requisite and necessary company and other internal action on the part of Seller.

5.3 No suit, action, arbitration, or legal, administrative, or other proceedings, including but not limited to condemnation proceeding, is pending or has been threatened against the Property or against the Seller with respect to the Property.

5.4 No bankruptcy, insolvency, rearrangement, or similar action or proceedings, whether voluntary or involuntary, is pending or threatened against Seller, or any partner of Seller and Seller has no intention of filing or commencing any such action or proceeding.

5.5 There are no existing or pending contracts of sale, leases, options to purchase, or rights of first refusal (or the like) with respect to the Property.

5.6 Seller is not a "foreign person" as defined in the Foreign Investment in Real Property Tax Act of 1980, as amended.

5.7 The Property is not subject to any protest or appeal proceedings related to real property taxes.

5.8 Seller has not received any written notice indicating that the Property is in violation, or that with the giving of notice or the passage of time would be in violation, of any applicable law, enactment, statute, code, ordinance, rule, regulation, judgment, writ, injunction, authorization, covenant, condition, restriction or agreement, or other direction or requirement of any governmental authority.

5.9 Neither Seller nor any affiliate or agent or contractor of Seller has disposed of or otherwise released any Hazardous Substances on the Property. To the best of Seller's knowledge, there are no Hazardous Substances present on the Property. Seller further warrants that until termination of this Agreement or delivery of possession of the Property to Purchaser, neither Seller nor any agent of Seller will cause or permit any Hazardous Substance to be disposed of or released or present on, over, beneath, in or upon the Property or to exist on or within any portion of the Property. "Hazardous Substances" shall mean asbestos (including asbestos in friable form), polychlorinated biphenyls, petroleum products, any flammable explosives, radioactive materials, hazardous materials, hazardous wastes, hazardous or toxic substances or related materials as defined in the Comprehensive Environmental Response, Compensation and Liability Act of 1980, as amended (42 U.S.C. §9601, et seq.), the Hazardous Materials Transportation Act, as amended, (49 U.S.C. §1801, et seq.), the Resource Conservation and Recovery Act, as amended (42 U.S.C. §6901, et seq.), the Toxic Substances Control Act, as amended (15 U.S.C. §2601, et seq.), any Environmental Laws. "Environmental Laws" means any federal, state or local statutes, laws, regulations, rules, decrees, orders, judgments, stipulations, ordinances, policies or common law related to the protection of human health and the environment or the use, handling, treatment, storage, disposal, release, remediation or transportation, or exposure of persons to, Hazardous Substances.

5.10 The representations and warranties of this Section 5 shall survive Closing.

6. CLOSING.

6.1 The consummation of the contemplated transaction (the "Closing") shall be held at the offices of Pioneer Land Settlement, Inc., in Pittsburgh, Pennsylvania, not later than thirty (30) days after expiration of the Due Diligence Period, including any extensions, time being of the essence. The exact date and time of the Closing shall be designated by mutual agreement of the Seller and Purchaser upon notice to Seller of not less than five (5) days. The Title Company shall be responsible at the Closing for preparing the settlement statement, causing all documents to be recorded, disbursing all closing proceeds, and otherwise conducting settlement.

6.2 The following apportionments shall be made between the parties at the Closing:

(a) Real estate taxes, personal property taxes, special assessments, if any, on the basis of the fiscal or calendar period for which assessed.

(b) Water and sewer service charges and charges for gas, electricity, telephone and all other public utilities. If there are meters measuring the consumption of water, gas or electric current, Seller shall, not more than one day prior to the Closing date, if possible, cause such meters to be read, and shall pay all utility bills for which Seller is liable upon receipt of statements therefor. Purchaser shall be responsible for causing such utilities and services to be changed to its name and shall be liable for and shall pay all utility bills for services rendered after the Closing.

(c) All other charges and fees customarily prorated and adjusted in similar transactions in Pennsylvania.

6.3 At the Closing, Seller shall deliver to Purchaser, the following:

(a) A recordable Warranty Deed as required by Section 3.1 of this Agreement, conveying the Property in fee simple to Purchaser.

(b) A certificate, dated as of the Closing date, to establish that Seller is not a foreign person for the purposes of the Foreign Investment in Real Property Tax Act.

(c) Exclusive physical possession of the Property in its "AS IS" condition with all personal property removed, together with all books and records in Seller's possession or control and all keys.

(d) Such customary owner's title affidavits and gap indemnities as may be required by the Title Company in order to issue the title policy subject only to the Permitted Exceptions and without exception for parties in possession, mechanics' or materialmen's liens, unrecorded easements or matters first appearing of record after the effective date of the most recent Title Commitment but prior to the conveyance of the Property to Purchaser.

(e) If Seller is a business entity other than an individual, a Pennsylvania Good Standing Certificate, copies of the organizational documents for the Seller, and resolutions of Seller approving this Agreement and the transaction contemplated hereby and authorizing the execution and delivery of this Agreement, the completion of the transaction contemplated hereby and the execution and delivery of all documents required to be executed and delivered by Seller.

(f) Such other documents, instruments and affidavits as may be reasonably requested by Purchaser or the Title Company to effectuate the transaction contemplated by this Agreement and to induce the Title Company to insure title to the Property as described herein.

6.4 At the Closing, Purchaser shall deliver to Seller, the following:

(a) The balance of the Purchase Price, less the Deposit and subject to the prorations and credits set forth herein, payable in certified funds or by Federal Reserve Bank wire transfer to the Title Company on or prior to Closing.

(b) Such other documents, instruments and affidavits as may be reasonably requested by Seller or the Title Company to effectuate the transaction contemplated by this Agreement and to induce the Title Company to insure title to the Property as described herein.

6.5 Purchaser shall pay the costs and expenses associated with the following: (i) all costs of Purchaser's due diligence, including fees due its consultants and attorneys, (ii) all lenders' fees related to any financing to be obtained by Purchaser, (iii) all recording and filing charges in connection with the instruments by which Seller conveys the Property, (iv) all premiums and charges of the Title Company for the Title Commitment and the Owner's (and any mortgagee's) Title Policy (including endorsements), (v) the cost of the Survey, (vi) one-half of the transfer taxes, documentary stamp taxes and similar charges, if any, applicable to the transfer of the Property to Purchaser. The obligations of the Purchaser under this Section 6.5 shall survive the Closing (and not be merged therein) or any earlier termination of this Agreement.

6.6 Seller shall pay the costs and expenses associated with the following: (i) all fees due its attorneys and consultants, (ii) all reasonable costs incurred in connection with causing the Title Company to remove any title objections required to be removed or otherwise cured by Seller, (iii) all costs incurred in connection with the satisfaction of monetary liens on the Property, including any costs related to recording of any satisfaction or termination documents, and (iv) one-half of the transfer taxes, documentary stamp taxes and similar charges, if any, applicable to the transfer of the Property to Purchaser, and (v) a customary and reasonable settlement fee. The obligations of the Seller under this Section 6.6 shall survive the Closing (and not be merged therein) or any earlier termination of this Agreement.

7. RISKS OF LOSS; MAINTENANCE OF PROPERTY.

Risk of loss of the Property shall remain upon the Seller until Closing and delivery of possession to Purchaser. Seller shall maintain the Property in as good condition as it is now, except for ordinary wear and tear, until delivery of the same to Purchaser. Seller shall maintain such fire and casualty insurance as it has in force at this time. Purchaser understands that Purchaser may have an insurable interest in the Property upon the signing of this Agreement and, in order to protect Purchaser's own interest in the Property, Purchaser may retain or place in force adequate fire and casualty insurance with extended coverage on the Property as of the Effective Date of this Agreement.

8. EMINENT DOMAIN; CASUALTY.

After the Effective Date, in the event Seller receives any notice of any condemnation proceedings, or other proceedings in the nature of eminent domain, or if any part

of the Property is damaged or destroyed by casualty, Seller will forthwith notify Purchaser of same, and Purchaser shall have the option to: (i) proceed under this Agreement and obtain by assignment or otherwise all damages to which the owner of the Property may be entitled pursuant to the Pennsylvania Eminent Domain Code, or under any insurance policy of Seller, as applicable; or (ii) void this Agreement whereupon no party shall have any further duty or liability to the other. Notwithstanding the foregoing, if the Property is damaged by fire or casualty, and such damage can be repaired or reconstructed prior to the Closing in a good and workmanlike manner to the reasonable satisfaction of Purchaser, the Purchaser shall not have the right to terminate the Agreement.

9. REMEDIES.

9.1 In the event Seller materially fails to perform or breaches any of its representations, warranties or covenants to be performed by Seller under this Agreement, or Seller materially misrepresents any fact or circumstance, Purchaser shall be entitled (a) to enforce specific performance of this Agreement; (b) to bring suit for all damages suffered by reason of such failure and all of Purchaser's costs and expenses, including reasonable attorneys' fees; or (c) to terminate this agreement and have the Deposit and any Additional Deposit returned to Purchaser. Each remedy under this Section 9.1 may be cumulative and not exclusive.

9.2 If Purchaser defaults in its performance of any term, covenant, condition, or obligation under this Agreement, including the obligation of Purchaser to purchase the Property if all conditions precedent to such obligations have been satisfied, Seller shall be entitled to receive as complete liquidated damages the Deposit and any Additional Deposit as liquidated damages. The parties acknowledge that the Deposit and any Additional Deposit represents a reasonable effort to ascertain the damages to Seller in the event of a Purchaser default, which damages are difficult or impossible to quantify. Seller waives all other remedies.

9.3 A failure by either party to perform any act required by it under this Agreement, other than the requirement to close if all conditions have been met, shall not be deemed a default under this Agreement until such party has received written notice from the other party setting forth the alleged failure, and such failure has not been cured within five (5) days of receipt of such notice.

10. BROKERAGE COMMISSION.

Purchaser and Seller acknowledge that no brokerage commission is payable in connection with this transaction. Each of the parties hereto agrees to indemnify and hold the other harmless from claims made by any other broker, attorney or finder claiming through such party for a commission, fee or compensation in connection with this Agreement or the sale of the Property hereunder. The provisions of this Section 10 shall survive Closing.

11. ASSIGNMENT.

11.1 Neither party shall assign or transfer or permit the assignment or transfer of its rights or obligations under this Agreement without the prior written consent of the other, any such assignment or transfer without such prior consent being hereby declared to be null and void; provided, however, that Purchaser shall have the right to assign this Agreement to an Affiliate, whose direct or indirect ownership is at least 51% of the ownership of the Purchaser,

upon written notice to Seller no later than two (2) days prior to the Closing date, and such assignee(s) shall assume Purchaser's rights and obligations under this Agreement.

11.2 In the event either party consents to an assignment of this Agreement by the other for which consent is required, no further assignment shall be made without another written consent from the consenting party, unless the assignment may otherwise be made without consent under this Agreement. An assignment by either Seller or Purchaser of its interest in this Agreement shall not relieve Seller or Purchaser, as the case may be, from its obligations, but this Agreement shall then inure to the benefit of, and be binding on, the assignee's successors, heirs, legal representatives and assigns.

11.3 If Seller or Purchaser reasonably determine that an assignment of this Agreement may be subject to the imposition of realty transfer tax or other applicable taxes, then the parties shall terminate this Agreement effective prior to Closing. In the event of such termination, the parties hereby agree that Seller and Purchaser (or Purchaser's assignee) shall enter into a new purchase agreement immediately following the termination of this Agreement, which shall contain the same terms and conditions as this Agreement, except as otherwise agreed by the parties in advance. In addition to the foregoing, the parties hereby acknowledge and agree that any termination of this Agreement as contemplated by this Section 11.3 shall not constitute a default under this Agreement or result in the disbursement of any portion of the Deposit and any Additional Deposit, and, upon such termination, the Deposit and Any Additional Deposit shall be treated as if they were delivered to Purchaser and repaid to the Title Company. The parties shall execute and deliver such additional documents, instruments and certificates as may be reasonably requested by either party to evidence the transactions described in this Section 11.3.

12. GENERAL PROVISIONS.

12.1 The terms and conditions of this Agreement shall be binding upon and inure to the benefit of the parties hereto and their respective heirs, successors, assigns, and legal representatives.

12.2 Notices and other communications required by this Agreement shall be in writing and (i) delivered by hand with receipt; (ii) sent by recognized overnight delivery service; (iii) sent by certified or registered mail, postage prepaid, with return receipt requested; or (iv) by electronic mail with a confirmation copy sent by another method permitted under this Section. All notices shall be addressed as follows:

If to the Seller:	PETERSON ANTHONI MALS 754 Windows Road, Smicksburg, PA 16256
If to the Purchaser:	Cozza Enterprises LLC 295 Myoma Rd Mars, PA 16046
With Copy To:	Thomas H. Ayoob III, Esquire Thomas H. Ayoob III & Associates, LLC 710 Fifth Avenue, Suite 2000

Pittsburgh, PA 15219
e-mail: tom@pioneerls.com

Notices shall be deemed to be effective upon receipt or refusal of the addressee to accept delivery.

12.3. Whenever used herein, unless expressly provided otherwise, the term "days" shall mean consecutive calendar days, except that if the expiration of any time period measured in days occurs on a Saturday, Sunday, legal holiday, such expiration shall automatically be extended to the next business day.

12.4 This Agreement constitutes the entire agreement between the parties concerning the Property and supersedes all prior agreements or undertakings.

12.5 This Agreement may not be modified except by the written agreement of the parties.

12.6 In the event any one or more of the provisions contained in this Agreement are held to be invalid, illegal, or unenforceable in any respect, such invalidity, illegality, or unenforceability will not affect any other provisions hereof, and this Agreement shall be construed as if such invalid, illegal, or unenforceable provision had not been contained herein.

12.7 The parties acknowledge that each party and its counsel of choice if so desired has had an opportunity to review and revise this Agreement and that the normal rule of construction to the effect that any ambiguities are to be resolved against the drafting party shall not be employed in the interpretation of this Agreement or any amendment or modification hereof or any of the closing documents delivered by Seller or Purchaser hereunder.

12.8 Any paragraph headings or captions contained in this Agreement shall be for convenience of reference only and shall not affect the construction or interpretation of any provisions of this Agreement.

12.9 This Agreement shall be governed by and construed in accordance with the laws of the Commonwealth of Pennsylvania.

12.10 The parties hereby agree to indemnify and defend the Title Company in its role as escrow agent from any and all suits, actions or claims if the Title Company in its role as escrow agent acts in good faith on the written notice and direction of the parties delivered in accordance with the terms hereof.

12.11 The individuals executing this Agreement represent and warrant that they have full authority and/or have been duly authorized by their respective parties to do so on behalf of such parties.

12.12 This Agreement may be executed in separate counterparts, none of which need contain the signatures of all parties, each of which shall be deemed to be an original, and all of which taken together constitute one and the same instrument. It shall not be necessary in making proof of this Agreement to produce or account for more than the number of counterparts containing the respective signatures of, or on behalf of, all of the parties hereto. The exchange of executed copies of this Agreement by electronic mail, portable document format (.pdf) or other

electronic transmission method will constitute effective execution and delivery of this Agreement as to the parties for all purposes, and electronic signatures of the parties shall be deemed to be their original signatures for all purposes.

12.13 NOTICE--THIS DOCUMENT MAY NOT SELL, CONVEY, TRANSFER, INCLUDE OR INSURE THE TITLE TO THE COAL AND RIGHT OF SUPPORT UNDERNEATH THE SURFACE LAND DESCRIBED OR REFERRED TO HEREIN, AND THE OWNER OR OWNERS OF SUCH COAL MAY HAVE THE COMPLETE LEGAL RIGHT TO REMOVE ALL OF SUCH COAL, AND, IN THAT CONNECTION, DAMAGE MAY RESULT TO THE SURFACE OF THE LAND AND ANY HOUSE, BUILDING OR OTHER STRUCTURE ON OR IN SUCH LAND, THE INCLUSION OF THIS NOTICE DOES NOT ENLARGE OR RESTRICT OR MODIFY ANY LEGAL RIGHTS OR ESTATES OTHERWISE CREATED, TRANSFERRED, EXCEPTED OR RESERVED BY THIS INSTRUMENT. (This notice is set forth in the manner provided in Section 1 of the Act of July 17, 1957, P.L. 984, as amended, and is not intended as notice of unrecorded instruments, if any.

IN WITNESS WHEREOF, the parties hereto have executed this Agreement on the dates written below.

Date: 6/23/23

SELLER:

Anthony Mels Peterson

Date: 6/20/23

PURCHASER:

LJC QJ@